Safety Analysis Technical Report

Final Environmental Impact Statement

US-95 Thorncreek Road to Moscow Project No. DHP-NH-4110(156);Key No 09294



Blank Sample of Form Current Access Purchase Determination Idaho Transportation Department

- 1. Complete all blanks as indicated.
- 2. Refer to "Right-of-Way Memo: Access Control, Board Policy 4005, and IDAPA Rule 39.03.42" for further information.
- 3. Indicate the units under "Limits" as either Milepost (MP) or Station (Sta.)
- 4. Provide justification for the proposed limits of Access Purchase. Attach Traffic Impact Study as needed.
- 5. Attach an 8 1/2" x 11" Vicinity Map showing the limits of purchased access if the limits are not readily understood (e.g., an interchange or major intersection).
- 6. Complete the ITD 0606 and send a copy to Headquarters Right-of-Way in concurrence with the Right-of-Way plans.

Key Number	Project Number	Loc	ation				District
Route Number	Highway Access Ty	rpe (see IDAPA 39.03.42)	Design Year	ADT	DHV	Desigr	Speed

Access Purchased

Limits	Justification for Purchase

Remarks

Recommended By			
District Traffic Engineer			Date
District R/W Property Mgr			Date
District PDE			Date
Approved By			
District Engineer			Date
Distribution: Original - District Project File	Copies - HQ ROW	HQ Traffic	DCM

Types of Access Spacing

Effective December 12, 2012, the Idaho Board approved <u>Board Policy 4005</u> - Management of Department-Owned Property, which incorporated the recently revised <u>IDAPA Rule 39.03.42</u> - Rules Governing Right-of-Way Encroachments on State Rights-of-Way.

Under the revised <u>IDAPA Rule 39.03.42</u>, all routes on the State Highway System are classified into one of six tiers; Interstate, Freeway, Expressway, Statewide, Regional, and District. Segments of highway are further classified according to whether they are rural, transitional, urban high-speed, or urban low-speed. A map of these classifications is attached to this memo and is available at: <u>September 2012 Access Map.</u>

Any existing access removed during a highway project shall be documented on the right-of-way documents and the "As Constructed" plans.

To maintain system capacity, safety and efficiency, maximize signal progression, and minimize delays to the traveling public, all approaches and signals shall be spaced in accordance with ITD standards. Variances to the spacing standards shall be handled as follows (from IDAPA 39.03.42):

The District Engineer shall have the authority to approve a decrease in the minimum access spacing distances set forth in Table 1, provided that the basis for any exception is justified and documented. The basis for the exception may include overriding economic opportunity considerations. For any exception that would result in a decrease in access spacing of more than ten percent (10%) of the distances set forth in Table 1, a Traffic Impact Study will be required in order to determine whether auxiliary lanes or other appropriate mitigation must be included in the permit's conditions. (10-1-12)T

A copy of this required documentation shall be available in the Project File.

Idaho Administrative Code	IDAPA 39.03.42 - Rules Governing Highway Right-of-Way
Idaho Transportation Department	Encroachments on State Rights-of-Way

Highway Type	Area Type	Signalized Road Spacing	Public Road Spacing (A)	Driveway Distance Upstream from Public Road Intersection (B)	Driveway Distance Downstream from Unsignalized Public Road Intersection (C)	Distance Between Unsignalized Accesses Other Than Public Roads (D)
Interstate	All	Accessible only by interchanges (ramps) and requires approval by the Board and Federal Highway Administration				
Freeway	All	Accessible only by interchanges (ramps)				
Expressway	All	Accessible only at locations specified by the Department				
	Rural	5,280 ft.	5,280 ft.	1,000 ft.	650 ft.	650 ft.
Statewide Deute	Transitional	5,280 ft.	2,640 ft.	760 ft.	500 ft.	500 ft.
Statewide Route	Urban >35 mph	2,640 ft.	1,320 ft.	790 ft.	500 ft.	500 ft.
	Urban <35 mph	2,640 ft.	1,320 ft.	790 ft.	250 ft.**	250 ft.**
	Rural	5,280 ft.	2,640 ft.	1,000 ft.	650 ft.	650 ft.
Decienal Doute	Transitional	2,640 ft.	1,320 ft.	690 ft.	360 ft.**	360 ft.**
Regional Route	Urban >35 mph	2,640 ft.	660 ft.	660 ft.	360 ft.**	360 ft.**
	Urban <35 mph	2,640 ft.	660 ft.	660 ft.	250 ft.**	250 ft.**
District Doute	Rural	2,640 ft.	1,320 ft.	760 ft.	500 ft.	500 ft.
	Transitional	2,640 ft.	660 ft.	660 ft.	360 ft.**	360 ft.**
District Route	Urban >35 mph	1,320 ft.	660 ft.	660 ft.	360 ft.**	360 ft.**
	Urban <35 mph	1,320 ft.	660 ft.	660 ft.	250 ft.**	250 ft.**

 Table 1 – Access Spacing*

*Distances in table are minimums based on optimal operational and safety conditions such as adequate sight distance and level grade. Definitions of spacing designated by (A), (B), (C), and (D) are represented on Figure 1.

**Where the public road intersection or private access intersection is signalized, the distances in the table are for driveways restricted to right-in/right-out movements only. For unrestricted driveways the minimum distance shall be 500 feet from a signalized intersection.

REGULAR MEETING OF THE IDAHO TRANSPORTATION BOARD

January 16, 2014

The Idaho Transportation Board met at 7:30 AM, on Thursday, January 16, 2014, at the Idaho Transportation Department in Boise, Idaho. The following principals were present: Jerry Whitehead, Chairman Jim Coleman, Vice Chairman – District 1 Janice B. Vassar, Member – District 2 Julie DeLorenzo, Member – District 3 Jim Kempton, Member – District 4 Lee Gagner, Member – District 6 Brian W. Ness, Director Scott Stokes, Chief Deputy Larry Allen, Lead Deputy Attorney General Sue S. Higgins, Executive Assistant and Secretary to the Board

<u>Executive Session on Personnel and Legal Issues</u>. Member Vassar made a motion to meet in executive session at 7:30 AM to discuss personnel and legal issues as authorized in Idaho Code Section 67-2345(a), (c), and (f). Member DeLorenzo seconded the motion and it passed 5-0 by individual roll call vote.

Discussions were held on personnel and legal matters.

The Board came out of executive session at 10:45 AM.

<u>New Business</u>. Information Technology Administrator Shannon Barnes requested approval of two agreements with 3M Corporation. The License Agreement is for software to update components such as titles and registrations for the Division of Motor Vehicles' modernization project. The Service Agreement is for training and consulting related to the software.

Member Vassar made a motion, seconded by Vice Chairman Coleman, and passed unopposed to approve the following resolution:

RES. NO. WHEREAS, in December 2010 the Idaho Transportation Board selected 3M

ITB14-01 Corporation to modernize the Idaho Transportation Department Division of Motor Vehicles' (DMV) computer system; and

WHEREAS, the contract was signed and work commenced in May 2011; and

WHEREAS, after 18 months of support from 3M, ITD decided it could independently complete the project and implement the software provided by 3M; and

WHEREAS, the Division of Purchasing has reviewed a License Agreement for ITD to use the 3M software in its DMV modernization program and a Service

Agreement under which 3M would install the software and train ITD in its use; and

WHEREAS, the Division of Purchasing has delegated to ITD the authority to enter into the License Agreement and the Service Agreement.

NOW THEREFORE BE IT RESOLVED, that the Board approves the License Agreement presented to it; and

BE IT FURTHER RESOLVED, that the Board approves the Service Agreement presented to it.

<u>Board Minutes</u>. Member Gagner made a motion to approve the minutes of the regular Board meeting held on December 11, 2013 as submitted. Member Vassar seconded the motion and it passed unopposed.

<u>Board Meeting Dates</u>. The following meeting dates and locations were scheduled: February 19-20, 2014 – Boise March 19-20, 2014 – Boise April 15-16, 2014 – District 2 (tentative location)

<u>Consent Items</u>. Vice Chairman Coleman made a motion, seconded by Member Gagner, and passed unopposed, to approve the following resolution:

RES. NO. WHEREAS, consent calendar items are to be routine, non-controversial, self-ITB14-02 explanatory items that can be approved in one motion; and

WHEREAS, Idaho Transportation Board members have the prerogative to remove items from the consent calendar for questions or discussion.

NOW THEREFORE BE IT RESOLVED, that the Board approves the addition of the Railroad-ITD mitigation strategies to FY14 of the program, transit program changes requested by Valley Regional Transit and Community Planning Association of Idaho to FY14, and the designation of an expressway – US-95, Lewiston to Thorncreek Road and proposed Thorncreek Road to Moscow.

1) Addition of the Railroad-ITD Mitigation Strategies to FY14 of the Program. The Department received a \$25,000 Strategic Highway Research Program User Incentive grant from the Federal Highway Administration. The grant is for ITD and Idaho's major rail partners to work cooperatively to delineate responsibilities and identify policy and programmatic changes to expedite project delivery for construction projects involving highways at railroad crossings. The grant will be used to hire a consultant to assist with establishing best practices for partnering with the railroad companies. The goal is to create a standardized institutional agreement to increase collaboration, reduce project delays, and streamline resources. Staff requested the addition of the Railroad-ITD Mitigation Strategies project to FY14 for \$25,000 and to update the Idaho Transportation Investment Program (ITIP) accordingly.

2) Transit Program Changes Requested by Valley Regional Transit (VRT) and Community Planning Association of Southwest Idaho (COMPASS). On behalf of VRT, COMPASS requested increasing funding for vehicle lease or purchase for the fixed line and demand response services, support for equipment and maintenance, and demand response services in the Nampa Urbanized Area for FY13. For the Boise Urbanized Area, it requested increases for mobility management administration and implementation in FY13; and demand response services, transit planning efforts, and mobility management administration implementation in FY14. Additionally, reductions in funding are being requested in the Nampa Urbanized Area for preventative maintenance support for fixed route and demand response service in FY13 and for demand response services in FY14. The COMPASS Regional Transportation Improvement Program has been modified to reflect these changes, as shown as Exhibit #436, which is made a part hereof with like effect. Staff requested modifying the Transit Program and amending the FY13-17 Statewide Transportation Improvement Program.

3) Designate Expressway – US-95, Lewiston to Thorncreek Road and Proposed Thorncreek Road to Moscow. Per IDAPA 39.03.42, the Board has authority to designate a highway as an "expressway" for the purpose of access control. Staff requested the designation of US-95 milepost 312.675 to 323.360 (segment code 001540); US-95 milepost 323.360 to 330.407 and milepost 330.407 to 337.668 (segment code 001539) and the proposed US-95 Thorncreek to Moscow project, milepost 337.668 to 344.004 as expressways.

<u>Informational Items</u>. 1) Monthly Financial Statements. Net obligations through December totaled \$103.3 million. Of those obligations, \$100.4 million were for activities programmed for 2014; although the Program estimated those activities at \$102.7 million. December's obligations exceeded the three-year average of \$83.1 million for the same period.

Through November, federal aid to the State Highway Fund totaled \$167.3 million yearto-date, or about 23% more than the same time period last year. Revenue from the Highway Distribution Account was 2.3% below projections. Staff will monitor the revenue to determine if adjustments are required. It was noted, however, that December's revenue improved with yearto-date revenue short of projections by .9%. Miscellaneous revenue and transfers in from the elimination of the ethanol exemption was \$1 million over the estimate. Expenditures for personnel costs were 4.4% less than budgeted through November. This is due to a continued effort to evaluate the workforce. Total expenditures, including encumbrances, for operations reflected a 1.7% positive variance. Capital equipment had a 5.6% negative variance, which is a timing difference in allotments versus contracts issued.

Aviation fuel tax revenue through November was 21% over projections. Staff does not believe the trend will continue. Miscellaneous revenue to the State Aeronautics Fund was 21% higher than projected. Overall expenditures were less than budgeted.

2) Non-Construction Professional Service Contracts Issued by Business and Support Management (BSM). From November 23 to December 30, the BSM Section processed three new professional service agreements and renewed one. The total activity equaled \$147,240.

3) FY15 Program Update – Funding Level Assumptions and Highlights. For the FY15 ITIP Update, funding assumptions are \$276 million from federal sources annually from FY15 through FY19. The projected state levels are \$12.5 million for FY15; \$10.2 million for FY16; \$11 million for FY17; and \$6 million for FY18 and FY19 each year. The process and schedule to update the Program was also outlined.

4) Status of FY15 Appropriation Request. The Department's FY15 appropriation request has been adjusted based on the Governor's budget recommendation. The main revisions are the elimination of the 1% change in employee compensation, totaling almost \$1 million; a fee increase of \$25,300 for the Idaho Technology Authority; and an additional \$65,900 for contract construction.

5) Contract Awards. Keys #12305 and #12304 – US-95, Plummer Creek Bridge, Milepost 395 and Plummer Creek Bridge, Milepost 394, District 1. Low bidder: Cook and Sons Construction - \$1,039,386.

Keys #12299 and #12300 – US-95, Kootenai River Bridge, Bonners Ferry and Burlington Northern Santa Fe Railroad and Arizona Street Bridge, Bonners Ferry, District 1. Low bidder: Braun-Jensen, Inc. - \$2,112,000.

Key #12329 – US-12 and US-95, Memorial, Spalding and Big Canyon Creek Bridges, District 2. Low bidder: The Truesdell Corporation - \$214,214.

Keys #12342 and #12396 – I-84, FY14 District 3 Pavement Striping and FY14 District 4 Pavement Striping. Low bidder: Interstate Barricades - \$557,750.

Key #12344 – SH-52, Union Pacific Railroad Overpass Rehabilitation, Payette, District 3. Low bidder: Braun-Jensen, Inc. - \$1,286,000.

Key #12407 – US-30, Snake River Gridley Bridge, District 4. Low bidder: Braun-Jensen, Inc. \$1,388,000.

Key #13065 – FY15 District 4 Districtwide Sealcoat. Low bidder: Intermountain Slurry Seal, Inc. - \$3,028,000.

Key #12401 – SH-50, Intersection 3800 East Road, Twin Falls County, District 4. Low bidder: Staker & Parson Companies dba Idaho Sand & Gravel Company - \$194,987.

Key #12454 – SH-28 and SH-33, FY15 District 6 Guardrail Upgrades. Low bidder: D L Beck Inc. - \$483,022.

Key #11675 – US-20, Island Park Lodge to Montana State Line, District 6. Low bidder: H-K Contractors, Inc. - \$2,715,808.

Key #12467 – US-20, South Rexburg Interchange #332 to South Fork Teton River Bridge, District 6. Low bidder: H-K Contractors, Inc. - \$3,043,588.

6) Professional Services Agreements and Term Agreement Work Task Report. From November 25 through December 27, 19 new professional services agreements and work tasks were processed, totaling \$778,400. Two supplemental agreements to existing agreements were processed during this period in the amount of \$37,330.

7) Annual Report on Outdoor Advertising Sign Status. At the close of federal FY13, 5 illegal and 190 non-conforming signs remained throughout the state. Illegal signs do not comply with state and federal law and are to be removed. Non-conforming signs complied with law at one time, but due to a change in conditions or rules, lost their conforming status. Non-conforming signs are allowed to remain in place but cannot be improved.

8) Administrative and Legal Settlements of Right-of-Way Acquisitions. From July 1 through December 31, 2013, the Right-of-Way Section processed 45 parcels. There were ten administrative settlements and five legal settlements during this time frame.

Legislative Report. Government Affairs Manager (GAM) Mollie McCarty reported on various legislative meetings and presentations scheduled. Overall, the Department's rules are proceeding well through the germane committees. Staff is monitoring and analyzing some non-ITD bills. It is continuing to work with the sponsors on the proposed 24/7 Sobriety and Drug Monitoring Program Act. She also mentioned that Member Gagner's term expires on January 31; however, the Governor is re-appointing him to another six-year term.

Chairman Whitehead thanked GAM McCarty for the report.

<u>Director's Report</u>. Director Ness also summarized some of the legislative activities. The Transportation Coalition has scheduled a series of presentations to the germane committees. Staff will present information on the condition of the state's bridges and highway safety. He recognized employees for their outstanding customer service and mentioned other recognitions ITD received. Director Ness also said he is changing the format of his monthly report. The Executive Team members will report on activities and accomplishments in their respective area.

The Director's entire report can be viewed at <u>http://itd.idaho.gov/Board/report.htm</u>.

Some of the Chief Executive Officers' highlights follow. At the federal level, efforts are underway on the next surface transportation act, as Moving Ahead for Progress in the 21st Century expires this fall. The final GARVEE bond sale generated a lot of buyers, resulting in an interest rate of 3.86%. About \$320 million in construction projects are on the shelf, ready to bid. The US-95, Thorncreek to Moscow project is proceeding well. The Record of Decision is expected soon, which will be followed by a 30-day advertisement period in the Federal Register. The Human Resource initiatives focus on recruitment; talent management, such as career paths and succession planning; developing employees through efforts such as leadership development and coaching and mentoring; and monitoring success by tracking the turnover rate and employee engagement. The Division of Administration is working on establishing a better team culture, engaging employees, and collaborating with other divisions. It wants hassle-free results, reports that are easy to understand, and to hire and develop the right people for each position. Chief Operations Officer Jim Carpenter also announced some personnel changes. Pat Lightfield, Assistant District 2 Engineer, is retiring this month, after 43 years of service. The Headquarters offices are being restructured into two divisions. District 3 Engineer Dave Jones has been promoted to Division of Engineering and Products and Plans Administrator/Chief Engineer and District 6 Engineer Blake Rindlisbacher will be the new Division of Engineering Services Administrator.

Chairman Whitehead thanked Director Ness and the Executive Team for the reports. The Board congratulated District Engineers Jones and Rindlisbacher on their promotions.

Informal Luncheon with the Aeronautics Advisory Board (AAB). The two boards met informally during lunch.

<u>Aeronautics' Annual Report</u>. AAB Chairman Rodger Sorensen reported on global aviation issues, noting commercial airlines recorded a profit this past year and commercial airlines U.S. Airways and American merged. He also commented on the increased use of unmanned aerial systems, also known as drones. AAB Member Chip Kemper said the agriculture aviation industry is doing well; however, he believes there will be challenges in the future, mainly due to drone activity, which could potentially create hazards for aircraft. The aviation fire activity was fairly steady this past year in eastern Idaho.

AAB Member Dan Scott commended the Division of Aeronautics for its search and rescue program. AAB Member Mark Sweeney said aviation activity in the state increased last year. He expressed concern with the decreasing number of pilots. In 2013, there were 25% fewer pilots than in 2000. He believes a bigger emphasis is needed on aviation education and promoting aviation. In response to a question from Member Kempton, AAB Sweeney responded that he believes one of the key reasons for the decline in pilots is the cost to become a pilot.

Aeronautics Administrator (AA) Mike Pape provided a financial report. Revenue is currently over projections due to more flights, as the majority of revenue is from jet fuel. Expenses are less than budgeted, but are expected to be on track. A high priority is to reduce the carryover of Trustee and Benefits expenses. In other areas, the federal government shutdown due to a lack of appropriations bills was a concern last year. No aircraft could be registered during that period and traffic control towers were negatively impacted. He also reported on the use of the state aircraft. State employees saved 3,600 hours by flying instead of driving.

Staff provided reports on various programs and activities. In 2013, \$450,000 was provided as matching funds for 19 Federal Aviation Administration grants to general aviation airports for rehabilitation, planning, and new facility projects. For 2014, \$700,000 is estimated to be available. Work is underway to improve the grant management process, with goals of less annual carryover and faster grant payouts. The volunteer program continues to be a valuable asset, as 335 man hours provided assistance with activities at eight of the state-owned airports. Recreational usage at the state airports was up 10% over the last two years. Last year, there were 33 aviation accidents with 12 fatalities in Idaho compared to 39 accidents with 5 fatalities in 2012. Pilots making poor decisions appeared to be the most common factor in the incidents.

AA Pape also reported that the avionics in the King Air need to be replaced.

Vice Chairman Coleman made a motion, seconded by Member Vassar, and passed unopposed, to approve the following resolution:

RES. NO. WHEREAS, the Idaho Transportation Department's Aircraft Operation function ITB14-03 is a critical program utilized by a variety of state agencies to perform state business effectively across the state; and

WHEREAS, safety of aircraft operations is the highest priority of every flight; and

WHEREAS, the 35 year old avionics (flight instruments, navigation, auto pilot, and associated systems) in the King Air are obsolete and safety and reliability may become a factor in current operations.

NOW THEREFORE BE IT RESOLVED, that the Idaho Transportation Board authorizes the Director to make the necessary business decisions to fund the replacement of the avionics systems for the King Air at an estimated cost of \$300,000 from current budgets within the State Aeronautics Fund.

Chairman Whitehead thanked the AAB members and staff for the report.

<u>Delegation – Pacific Northwest Economic Region (PNWER)</u>. Idaho Lieutenant Governor Brad Little introduced the PNWER delegation and provided background on the coalition. Comprised of the public and private sector, it addresses various issues of the region, including energy and transportation.

Alana DeLong from Alberta, Canada said there is interest in utilizing the Port of Lewiston; however, a good, reliable corridor to Canada is needed. She stressed the importance of access to markets and moving products.

Bruce Agnew said PNWER would like Idaho to take the lead on harmonizing vehicle size and weight standards in the region and to streamline the permitting process. Another initiative it would like Idaho to help coordinate is Fast Forward Northwest. This public private partnership would improve international market access for exports by eliminating bottlenecks on rail corridors and improving road and port connections.

The Board expressed support for increased usage of the Port of Lewiston and standardizing the truck size and weight regulations. Chairman Whitehead thanked the delegation for its presentation.

<u>Public Transportation Advisory Council (PTAC) Board Policy</u>. PTAC Chair Kathleen Simko discussed proposed revisions to Board Policy 4039 Public Transportation Advisory Council, formerly B-28-04. PTAC supports inclusion of language stating that a philosophy of reducing reliance on federal funding should be pursued. During her tenure on PTAC, there have never been sufficient funds for Idaho's transit needs. No service can be funded solely through fares. PTAC members believe other funding sources need to be identified.

Member Vassar understands PTAC's position; however, she does not support including that language in the Board policy. There is no state funding source for public transportation.

Member Kempton asked if the desire is to reduce reliance on federal funds, would Idaho decline additional federal money? PTAC Chair Simko replied that she does not anticipate additional federal funds would be rejected; however, she believes it is important to seek other funding sources.

Member Vassar made a motion to delete the statement "A philosophy of reducing reliance on Federal funding should be appropriately pursued" from the draft policy. Member DeLorenzo seconded the motion and it passed unopposed.

Member Vassar made a motion to approve the revised Board Policy 4039 Public Transportation Advisory Council. Member Kempton seconded the motion and it passed unanimously.

<u>Policy Introduction</u>. Human Resources (HR) staff introduced board and corresponding administrative policy changes. The legal authority and a purpose statement were added to all of the documents.

Board Policy 4019 Equal Employment Opportunity and Fair Employment Practices combined former policies B-18-07 Code of Fair Employment Practices and B-18-09 Equal Employment Opportunity/Affirmative Action. The consensus of the Board was to hold this policy for further review and discussion.

No additional changes are proposed to Board Policy 4033 Workplace Violence, formerly B-23-03. Minor changes are recommended to the corresponding administrative policy 5033 to ensure compliance with Idaho Code.

Member Vassar made a motion to approve Board Policy 4033 Workplace Violence. Member DeLorenzo seconded the motion and it passed unopposed.

Member Kempton made a motion to recommend Director approval of 5033 Workplace Violence. Member Vassar seconded the motion and it passed unanimously.

Minor changes are proposed to Board Policy 4055 Harassment in the Workplace, formerly B-18-04. More substantive changes are proposed to the corresponding administrative policy. By unanimous consent, the Board held 4055 and 5055 for further review and discussion.

The proposed change to Board Policy 4056 Employee Safety and Risk Management Program, formerly B-23-01, is the removal of a reference to a rule. No changes are proposed to the corresponding administrative policy 5056.

Vice Chairman Coleman made a motion to approve Board Policy 4056 Employee Safety and Risk Management Program. Member Vassar seconded the motion and it passed unopposed.

Member Vassar made a motion to recommend Director approval of 5056 Employee Safety and Risk Management Program. Member Kempton seconded the motion and it passed unopposed.

Administrative Policy Review. HR staff summarized changes to three stand-alone Administrative Policies: 5520 Internship Program, formerly A-01-17; 5521 Standard and Flextime Work Schedules, formerly A-06-03; and 5523 Alcohol and Drug-Free Workplace, formerly A-18-12. It also recommended deleting Administrative Policies A-06-44 Workplace Accommodations and A-18-08 Return to Work because the polices are procedural-based and the information is included in manuals.

The Board had no objection to the proposed changes to the stand-alone administrative policies.

WHEREUPON, the Idaho Transportation Board's regular monthly meeting officially adjourned at 3 PM.

signed JERRY WHITEHEAD, Chairman Idaho Transportation Board

Read and Approved February 19, 2014 Boise, Idaho

Board Agenda Item



Meeting Date .	Jan. 15 &	16, 2014
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Consent Item 🛛 Information Item 🗌

Amount of Presentation Time Needed

Presenter's Name	Presenter's Title	Initials	Reviewed By
Dave Kuisti	District 2 Engineer	DBK	
Preparer's Name	Preparer's Title	Initials	
Dave Ellis	Trans. Staff Engineer Assistant	DLE	

Subject

Designate Express	way - Lewiston to T	horncreek Road & Proposed Thorncreek Road to Moscow project.
Key Number	District	Route Number
Various & 09294	2	US-95

Background Information

Per IDAPA 39.03.42, the Idaho Transportation Board has the authority to designate a highway as an 'Expressway' for the purpose of access control.

The change in IDAPA 39.03.42 removes the former access control designation for these roadway segments; therefore, Board action is needed to maintain access control through the formal designation of 'Expressway'.

An 'Expressway' is a segment of highway for use as a through highway, with partially controlled access, accessible only at locations specified by the Idaho Transportation Department. These specific routes meet the definition in IDAPA 39.03.42.010.31, wherein 'Expressways' are characterized by medians, limited at-grade intersections and high speeds.

District 2 requests the Board designate these segments along with the proposed US-95 project, Thorncreek Road to Moscow as an 'Expressway' with intersections/approaches only at locations specified by the Idaho Transportation Department. Existing approaches will be granted for existing segments on US-95. For the proposed Thorncreek Road to Moscow project access will be granted at locations agreed upon during the design phase with individual property owners.

Recommendations

Designate US-95 MP 312.675 to MP 323.360 (Segment Code 001540), US-95 MP 323.360 to MP 330.407 & MP 330.407 to MP 337.668 (Segment Code 001539) and Proposed US-95 Thorncreek to Moscow project (MP 337.668 to MP 344.004), as Expressways.

Board Action

Approved

Deferred

Other

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REQUEST FOR 'EXPRESSWAY' DESIGNATION

Location: U.S. Highway 95, Lewiston N. City Limits to Top of the Lewiston Hill (Segment Code Change). Project Nos.: FF-RF-4114(23), FF-4114(28), DP-F-4114 (31) and TQF-4114(32). Segment Code: 001540 MP 312.675 to MP 323.360 Speed limit: 65 MPH

The right-of-way for these three projects was acquired and the highway was constructed in the 1970's on a new alignment using Partial Control Type E Access.

At-grade intersections include: NOTE: Bold are County Road Approaches.

M.P. 312.857 – Field Approach, Left; (M.P. 312.857 – Commercial Approach, Right; (M.P. 317.440 – Frontage Road/Scenic Overlook, Left; (M.P. 317.440 – Spur Road, Right; (M.P. 318.017 – Frontage Road, Left (Spiral); (M.P. 318.017 – Frontage Road, Right; (Access Rd) (M.P. 318.320 – Field Approach, Left; (M.P. 318.320 – Access Road, Right; (M.P. 319.613 – US 195 Spur, Left; (

(Lat. 46.43983992, Long. 116.98537290) (Lat. 46.43983992, Long. 116.98537290) (Lat. 46.46225242, Long. 117.00849087) (Lat. 46.46225242, Long. 117.00849087) (Lat. 46.46354709, Long. 117.01989378) (Lat. 46.46635591, Long. 117.02486358) (Lat. 46.46635591, Long. 117.02486358) (Lat. 46.48101623, Long. 117.03776165)



REQUEST FOR 'EXPRESSWAY' DESIGNATION

Location: U.S. Highway 95, Top of the Lewiston Hill to Genesee. Project Nos.: NH-4110 (133). Segment Code: 001539 MP 323.360 to MP 330.407 Speed limit: 65 MPH

The right-of-way for this project was acquired and the highway was constructed in 2005 & 2006 on a new alignment using Type IV Access Control where existing private approaches will be allowed to remain.

At-grade intersections for Project NH-4110 (133); Top of Lewiston Hill to Genesee include: NOTE: **Bold are County Road Approaches.**

SOUTHBOUND STATIONS - LT. (METRIC)
18+11.626 - Field Approach
26+89.585 – Leon Road
26+86.740 – Field Approach
38+46.962 – Field Approach
44+00.000 – Field Approach
50+30.000 – Field Approach
53+40.000 – Field Approach
58+20.000 – Field Approach
60+44.409 – Joint Use Field Approach
66+78.753 – Field Approach
75+51.857 – Field Approach
77+08.675 - Joint Use Residential Approach
78+68.685 – Field Approach
81+07.852 – Field Approach
86+52.291 – Field Approach
96+04.238 – Field Approach
97+40.716 – Residential Approach
102+30.000 – Field Approach
104+93.664 – Field Approach
116+37.877 – Hillside Road
112+43.005 – Field Approach
121+30.000 – Field Approach
121+88.492 – W. Cow Creek Road
125+53.251 – Field Approach
132+53.764 – Uniontown Road
136+48.858 – Residential Approach

NORTHBOUND STATIONS - RT. (METRIC)
1018+58.841-1020+27.135 - Turnout
1020+41.173 – Field Approach
1026+70.824 - Field Approach
1029+67.979 – Residential Approach
1035+70.866 - Field Approach
1038+28.201 – S. Evans Road
1043+86.691 - Field Approach
1048+83.197 – Joint Use Residential Approach
1058+14.566 - Field Approach
1060+34.485 - Field Approach
1066+61.805 - Field Approach
1073+55.000 - Field Approach
1075+26.290 - Field Approach
1078+43.088 - Field Approach
1080+85.961 - Field Approach
1086+39.775 - Field Approach
1096+13.895 - Field Approach
1097+56.964 – N. Evans Road
1105+10.229 – Residential Approach
1112+58.189 - Field Approach
1116+59.208 – Joint Use Field Approach
1122+09.823 – E. Cow Creek Road
1132+71.543 – Genesee-Juliaetta Road



REQUEST FOR 'EXPRESSWAY' DESIGNATION

Location: U.S. Highway 95, Genesee to Thorncreek Road. Project Nos.: NH-4110 (140). Segment Code: 001539 MP 330.407 to MP 337.668 Speed limit: 65 MPH

The right-of-way for this project was acquired and the highway was constructed in 2005 & 2006 on a new alignment using Type IV Access Control where existing private approaches will be allowed to remain.

At-grade intersections for Project NH-4110 (140); Genesee to Thorncreek Road include: NOTE: **Bold are County Road Approaches.**

SOUTHBOUND STATIONS - LT. (METRIC)
21+21.848 - Residential Approach
26+67.325 – Field Approach
33+09.875 – Residential Approach
37+56.699 – Field Approach
43+05.820 – Borgen Road
51+06.000 – Residential Approach
56+20.113 – Field Approach
59+44.716 – Kluss Road
64+55.000 – Residential Approach
71+64.375 – Sather Road
77+82.199 – Residential Approach
80+01.510 - Field Approach
88+80.000 - Residential Approach
91+64.000 – Field Approach
102+23.983 – Field Approach
106+29.985 – Residential Approach
114+16.790 – Residential Approach
120+13.729 – Thorncreek Road
124+77.985 – Field Approach

NORTHBOUND STATIONS - RT. (METRIC)
1021+21.863 - Field Approach
1026+67.494 – Neyens Road
1033+06.127 – Residential Approach
1037+49.772 - Field Approach
1042+99.047 - Residential Approach
1047+38.159 – Residential Approach
1051+08.254 - Field Approach
1054+10.000 – Field Approach
1056+21.635 - Field Approach
1064+62.732 - Field Approach
1071+75.376 – Sather Road
1080+26.500 Hove Road
1092+04.633 - Field Approach
1102+54.494 - Field Approach
1106+69.247 – Old 95
1114+53.866 – Martinson Road
1120+36.434 - Field Approach
1124+91.649 - Field Approach



REQUEST FOR 'EXPRESSWAY' DESIGNATION:

Location: U.S. Highway 95, Thorncreek Road to Moscow (Proposed Route). Project Nos.: DHP-NH-4110 (156). Segment Code: 001539 MP 337.668 to MP 344.004 Speed limit: 65 MPH

District 2 is currently in the final stages of completing the Environmental Impact Statement for Thorncreek to Moscow and is currently addressing comments generated from the Public Hearing and FHWA before the Final Environmental Impact Statement can be submitted for review. Many comments were related to access control and the FHWA has required ITD to better define how future access control will be limited so that highway safety will not be compromised in the future by new access points.

District 2 is proposing to use the "Expressway" designation to control access on the proposed highway. An 'Expressway' is a segment of highway for use as a through highway, with partially controlled access, accessible only at locations specified by the Idaho Transportation Department. These specific routes meet the definition in IDAPA 39.03.42.010.31, wherein 'Expressways' are characterized by medians, limited atgrade intersections and high speeds. Only existing private approaches will be allowed to remain. The exact approach locations are not yet known. The approach locations will be discussed and negotiated with the property owners during the right-of-way acquisition phase of the project.

ADDENDUM 1 US-95 THORNCREEK ROAD TO MOSCOW AASHTO HIGHWAY SAFETY MANUAL ANALYSIS ON ALTERNATIVES CARRIED FORWARD DHP-NH-4110 (156) KEY # 09294

December 31, 2014

PREPARED BY	
DISTRICT 2 PROJECT DEVELOPMENT ENGINEER	
Curtis J. Arnzen, P.E.	
01/01/15 Date	



DHP-NH-4110 (156); Key No. 9294; Thorncreek to Moscow

December 31, 2014

Introduction

This Addendum was written to address safety for Alternative Modified W4. Alternative Modified W4 includes a slight alignment shift in Alternative W4 to avoid a cultural resource. Alternative Modified W4 is slightly shorter than Alternative W4; therefore, it is predicted to be slightly safer than Alternative W4. The conclusions and recommendations of the AASHTO Highway Safety Manual Analysis dated September 13, 2013 remain valid and calculations and conclusions regarding Alternative W4 in the AASHTO Highway Safety Manual Analysis.

Summary

The following results for Alternative Modified W4 are in Table 1 below:

	Table 1: Predicte	ed Crashes For Propose	ed Alternative Modifie	d W4
	Completio	on Year 2017	Crashes From 2	017 Through 2036
Alternative	Total Crashes	Fatal and Injury Crashes	Total Crashes	Fatal and Injury Crashes
Modified W4	9.2	4.6	218.0	107.0

The following results for Alternative Modified W4 are in Table 2 below:

Table 2: Pro	edicted Crashes Fo	r Proposed Alternative	Modified W4 and Rer	naining US-95 Loop
	Completio	on Year 2017	Crashes From 2	017 Through 2036
Alternative	Total Crashes	Fatal and Injury Crashes	Total Crashes	Fatal and Injury Crashes
Modified W4	10.5	5.0	244.9	116.2

Economic Cost of Crashes

There were no differences in any of the economic cost of crashes between Alternatives W4 and Modified W4 due to rounding to significant figures. The following results for Alternative Modified W4 are in Table 3 below:

Table 3	3: Total Economic	Cost of Crashes on the	Proposed Alternati	ve Modified W4
	Complet	ion Year 2017	From 201	7 Through 2036
Alternative	Economic Cost	Difference From E2	Economic Cost	Difference From E2
Modified W4	\$1,400,000	\$300,000	\$32,000,000	\$6,000,000

DHP-NH-4110 (156); Key No. 9294; Thorncreek to Moscow

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Table 4		Cost of Crashes on the he Remaining US-95 Lo		
	Completi	ion Year 2017	From 201	7 Through 2036
Alternative	Economic Cost	Difference From E2	Economic Cost	Difference From E2
Modified W4	\$1,500,000	\$200,000	\$35,000,000	\$5,500,000

The following results for Alternative Modified W4 are in Table 4 below:

Calculation Methodology for Action Alternatives

Predictive Calculations on Proposed Alignments

The calculations of crashes on the different highway sections and intersections are nearly the same for Alternatives W4 and Modified W4. The only calculations that change are the calculations for rural divided four-lane highway. The length of rural divided four lane highway for Alternative Modified W4 is 0.04 miles shorter than Alternative W4. The new calculation sheets and crash results for the rural divided four-lane highway for Alternative Modified W4 are in the Appendix of this Addendum.

Predictive Calculations on the Remaining US-95 Loop

The calculations on the remaining US-95 Loop are the same for Alternatives W4 and Modified W4.

Wild Animal Crashes

The wild animal crash potential is the same for Alternatives W4 and Modified W4.

Crashes Relating to Unfavorable Weather Conditions

The weather conditions are the same for Alternatives W4 and Modified W4.

Crash Prediction Results for Proposed Alternatives

Alternative Modified W4

Alternative Modified W4 is predicted to have slightly fewer crashes than Alternative W4 due to 0.04 mile shorter length.

The calculations of crashes on the different highway sections and intersections including the remaining US-95 Loop are nearly the same for Alternatives W4 and Modified W4. The only

DHP-NH-4110 (156); Key No. 9294; Thorncreek to Moscow

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calculations that change are the calculations for rural divided four-lane highway. The length of rural divided four lane highway for Alternative Modified W4 is 0.04 miles shorter than Alternative W4. The new calculation sheets and crash results for rural divide four-lane highway for Alternative Modified W4 are in the Appendix of this Addendum. The remainder of the calculations including the calculations for the remaining US-95 Loop of Alternative Modified W4 did not change as a result of the alignment shift and are in Appendix E of the AASHTO Highway Safety Manual Analysis dated September 13, 2013 within the Alternative W4 calculations.

Tab	ole 13: HSM Crash	Results for Alterna	ative Modified W4	
	Constructio	on Year 2017	Crashes From 20	017 Through 2036
	Total Crashes	Fatal and Injury Crashes	Total Crashes	Fatal and Injury Crashes
Rural Divided Multilane Segment	6.9	3.8	161.8	87.3
Suburban Segment	1.1	0.3	26.2	7.9
Eid Road Intersection	0.3	0.1	8.2	3.5
Jacksha Road Intersection	0.3	0.1	7.7	3.3
South Old US-95 Intersection	0.2	0.1	5.2	1.7
North Old US-95 Intersection	0.4	0.1	8.8	3.4
Total	9.2	4.6*	218.0*	107.0*

Table 13, shown below, summarizes the predicted crashes for Alternative Modified W4.

*Note: Differences between the total number and the sum of components are due to rounding. The actual numbers that have not been rounded can be found in Appendix E.

Table 14, shown below, summarizes the predicted crashes for Alternative Modified W4 and the remaining US-95 Loop.

	r	on Year 2017	W4 and Remaining	05-95 Loop 017 Through 2036
	Total Crashes	Fatal and Injury Crashes	Total Crashes	Fatal and Injury Crashes
Modified W4 Alternative	9.2	4.6	218.0	107.0
Remaining US-95 Loop	1.3	0.4	26.9	9.2
Total	10.5	5.0	244.9	116.2

DHP-NH-4110 (156); Key No. 9294; Thorncreek to Moscow

December 31, 2014

An estimate of the economic cost of all accidents on Alternative Modified W4 can be calculated using the HSM Crash Results shown above, the economic costs of the different crash types reported in Idaho Traffic Crashes 2012, the average frequency of the different injury and fatal accidents on Idaho's Highways, and the average multiple car crash frequency.

The estimated economic cost of crashes on Alternative Modified W4 between 2017 and 2036 is calculated to be about \$32,000,000 and the estimated economic cost of crashes on Alternative Modified W4 and the remaining US-95 Loop is calculated to be about \$35,000,000. These costs are identical to Alternative W4 due to rounding to significant figures.

Conclusion

The following results for Alternative Modified W4 are in Table 15 below:

	Table 15: Predict	ed Crashes For Propos	sed Alternative Modifi	ed W4
	Completio	on Year 2017	Crashes From 2	017 Through 2036
Alternative	Total Crashes	Fatal and Injury Crashes	Total Crashes	Fatal and Injury Crashes
Modified W4	9.2	4.6	218.0	107.0

The following results for Alternative Modified W4 are in Table 16 below:

Table 16: Pr	edicted Crashes Fo	r Proposed Alternative	e Modified W4 and Re	maining US-95 Loop
	Completio	on Year 2017	Crashes From 2	017 Through 2036
Alternative	Total Crashes	Fatal and Injury Crashes	Total Crashes	Fatal and Injury Crashes
Modified W4	10.5	5.0	244.9	116.2

In conclusion, this Addendum documents from a safety perspective Alternatives W4 and Modified W4 are nearly the same and provides the required calculations.

ADDENDUM 1 US-95 THORNCREEK ROAD TO MOSCOW AASHTO HIGHWAY SAFETY MANUAL ANALYSIS ON ALTERNATIVES CARRIED FORWARD DHP-NH-4110 (156) KEY # 09294 December 31, 2014

APPENDIX E REVISIONS

Modified W4		To the second second		Modified W4			Modified W4		
4.6	2017	San Con		10.5	2017		9.2	2017	
4.7	2018			10.7	2018		9.4	2018	
4.7	2019			10.8	2019		9.5	2019	
4.8	2020		To	11.0	2020		9.7	2020	
4.9	2021		tal Pred	11.2	2021		9.9	2021	
5.0	2022		licted Fa	11.4	2022	Propos	10.0	2022	
5.0	2023	Propo	atal and	11.5	2023	ed Modifie	10.2	2023	Prop
5.1	2024	Proposed Alignment Total Fatal and Injury Crash Summary by Year	Injury	11.7	2024	Proposed Modified W4 Alternative and Existing US-95 Loop Total Crash Summary by	10.4	2024	Proposed Modified W4 Alternative Total Crash Summary by Year
5.2	2025	ent Total Fa	Crashes	11.9	2025	native and	10.6	2025	ed W4 Alte
5.3	2026	ital and Inju	of Moc	12.1	2026	Existing US	10.8	2026	rnative Tot
5.4	2027	JITY Crash Si	dified W	12.3	2027	-95 Loop To	10.9	2027	al Crash Su
5.5	2028	immary by	14 Alter	12.5	2028	otal Crash S	11.1	2028	mmary by
5.5	2029	Year	native E	12.7	2029	ummary by	11.3	2029	lear
5.6	2030		3etweer	12.9	2030	Year	11.5	2030	EVER NORM
5.7	2031	State of the	n 2017 a	13.1	2031		11.7	2031	
5.8	2032		Total Predicted Fatal and Injury Crashes of Modified W4 Alternative Between 2017 and 2036	13.3	2032		11.9	2032	
5.9	2033		σ	13.5	2033		12.1	2033	
6.0	2034			13.7	2034		12.3	2034	
6.1	2035			14.0	2035		12.6	2035	
6.2	2036			14.2	2036		12.8	2036	
107.0	Total			244.9	Total		218.0	Total	

Total Predicted Crashes of Modified W4 Alternative Between 2017 and 2036

	Modified W4	
	4.6	1107
	4.7	0107
	4.7	ETD7
	4.8	0707
	4.9	1707
Propose	5.0	7707
d Alignmen	5.0	2023
t and Existi	5.1	2024
ng US-95 Lo	5.2	5707
oop Total Fa	5.3	2079
tal and Inlu	5.4	1707
Jrv Crash Si	5.5	2028
Immarv by	5.5	5202
by Year	5.6	2030
	5.7	2031
Charles and	5.8	2032
STREET, STREET, ST	5.9	2033
	6.0	2034
A COLORADO	6.1	2035
	6.2	2036
Constanting of the second	107.0	Total

Modified W4

2017 5.0

2018 5.1

2019 5.2

2020 5.3

2021 5.3

2022

2023 5.5

2024 5.6

2025

2026

2027 5.8

2028 5.9

2029 6.0

2030

2031 6.2

2032

2033 6.4

2034

2035

2036 6.7

Total 116.2

244.945	14.2	14.0	13.7	13.5	13.3	13.1	12.9	12.7	12.5	12.3	12.1	11.9	11.7	11.5	11.4	11.2	11.0	10.8	10.7	10.5	Total (Crashes/year)
26.9	1.4	1.4	1.4	1.4	1.4	1.4	1.4	1.4	1.4	1.3	1.3	1.3	1.3	1.3	1.3	1.3	1.3	1.3	1.3	1.3	Total
2.465	0.129	0.129	0.128	0.127	0.127	0.126	0.125	0.125	0.124	0.124	0.123	0.122	0.122	0.121	0.120	0.120	0.119	0.119	0.118	0.117	North Clyde
1.667	0.087	0.087	0.087	0.086	0.086	0.085	0.085	0.084	0.084	0.084	0.083	0.083	0.082	0.082	0.081	0.081	0.081	0.080	0.080	0.079	Cameron
0.478	0.024	0.024	0.024	0.024	0.024	0.024	0.024	0.024	0.024	0.024	0.024	0.024	0.024	0.024	0.024	0.024	0.024	0.023	0.023	0.023	South Clyde
0.170	0.009	0.009	0.009	0.009	0.009	0.009	0.009	0.009	0.009	0.008	0.008	0.008	0.008	0.008	0.008	0.008	0.008	0.008	0.008	0.008	Skyline
0.340	0.017	0.017	0.017	0.017	0.017	0.017	0.017	0.017	0.017	0.017	0.017	0.017	0.017	0.017	0.017	0.017	0.017	0.017	0.017	0.017	Snow
0.539	0.028	0.028	0.027	0.027	0.027	0.027	0.027	0.027	0.027	0.027	0.027	0.027	0.027	0.027	0.027	0.027	0.027	0.026	0.026	0.026	Zeitler
Total	2036	2035	2034	2033	2032	2031	2030	2029	2028	2027	2026	2025	2024	2023	2022	2021	2020	2019	2018	2017	Intersection
10.299	0.563	0.557	0.552	0.547	0.542	0.536	0.531	0.526	0.521	0.517	0.512	0.507	0.502	0.497	0.493	0.488	0.484	0.479	0.475	0.470	22
5.449	0.287	0.285	0.284	0.282	0.281	0.279	0.278	0.276	0.275	0.273	0.272	0.270	0.269	0.267	0.266	0.264	0.263	0.261	0.260	0.259	21
1.945	0.102	0.101	0.101	0.100	0.100	0.099	0.099	0.098	0.098	0.097	0.097	0.096	0.096	0.096	0.095	0.095	0.094	0.094	0.093	0.093	20
0.297	0.015	0.015	0.015	0.015	0.015	0.015	0.015	0.015	0.015	0.015	0.015	0.015	0.015	0.015	0.015	0.015	0.015	0.015	0.015	0.015	19
1.042	0.053	0.053	0.053	0.053	0.053	0.053	0.052	0.052	0.052	0.052	0.052	0.052	0.052	0.052	0.052	0.052	0.051	0.051	0.051	0.051	18
0.320	0.016	0.016	0.016	0.016	0.016	0.016	0.016	0.016	0.016	0.016	0.016	0.016	0.016	0.016	0.016	0.016	0.016	0.016	0.016	0.016	17
0.521	0.026	0.026	0.026	0.026	0.026	0.026	0.026	0.026	0.026	0.026	0.026	0.026	0.026	0.026	0.026	0.026	0.026	0.026	0.026	0.026	16
0.238	0.012	0.012	0.012	0.012	0.012	0.012	0.012	0.012	0.012	0.012	0.012	0.012	0.012	0.012	0.012	0.012	0.012	0.012	0.012	0.012	15
0.420	0.021	0.021	0.021	0.021	0.021	0.021	0.021	0.021	0.021	0.021	0.021	0.021	0.021	0.021	0.021	0.021	0.021	0.021	0.021	0.021	14
0.183	0.009	0.009	0.009	0.009	0.009	0.009	0.009	0.009	0.009	0.009	0.009	0.009	0.009	0.009	0.009	0.009	0.009	0.009	0.009	0.009	13
0.419	0.021	0.021	0.021	0.021	0.021	0.021	0.021	0.021	0.021	0.021	0.021	0.021	0.021	0.021	0.021	0.021	0.021	0.021	0.021	0.021	12
0.146	0.007	0.007	0.007	0.007	0.007	0.007	0.007	0.007	0.007	0.007	0.007	0.007	0.007	0.007	0.007	0.007	0.007	0.007	0.007	0.007	11
Total	2036	2035	2034	2033	2032	2031	2030	2029	2028	2027	2026	2025	2024	2023	2022	2021	2020	2019	2018	2017	Segment
									and the second			A STATE				State of the					Exisiting US-95
218.0	12.8	12.6	12.3	12.1	11.9	11.7	11.5	11.3	11.1	10.9	10.8	10.6	10.4	10.2	10.0	9.9	9.7	9.5	9.4	9.2	Total
8.810	0.521	0.511	0.502	0.493	0.484	0.475	0.466	0.458	0.450	0.442	0.434	0.426	0.419	0.411	0.404	0.397	0.390	0.383	0.376	0.370	North Old US-95
5,170	0.310	0.304	0.298	0.292	0.287	0.281	0.275	0.270	0.265	0.259	0.254	0.249	0.244	0.240	0.235	0.230	0.226	0.221	0.217	0.213	South Old US-95
7.740	0.464	0.455	0.446	0.438	0.429	0.420	0.412	0.404	0.396	0.388	0.381	0.373	0.366	0.359	0.352	0.345	0.338	0.331	0.325	0.318	Jacksha
8.235	0.494	0.484	0.475	0.465	0.456	0.447	0.439	0.430	0.421	0.413	0.405	0.397	0.389	0.382	0.374	0.367	0.359	0.352	0.345	0.339	Eid
Total	2036	2035	2034	2033	2032	2031	2030	2029	2028	2027	2026	2025	2024	2023	2022	2021	2020	2019	2018	2017	Intersection
26.243	1.522	1.497	1.473	1.450	1.427	1.404	1.382	1.360	1.338	1.317	1.296	1.275	1.255	1.235	1.216	1.196	1.178	1.159	1.141	1.123	Suburban
161.810	9.457	9.298	9.142	8.989	8.838	8.689	8.544	8.400	8.259	8.121	7.984	7.850	7.718	7.589	7.461	7.336	7.213	7.092	6.973	6.856	Rural Divided
Total	2036	2035	2034	2033	2032	2031	2030	2029	2028	2027	2026	2025	2024	2023	2022	2021	2020	2019	2018	2017	Segment
				The state								Ser Lange	and the second	Contraction of							New Alignment
									ummary	wooifled w-4 Alternative Jotal Crash Summary	lotai	iternativ	0 VV-4 A	MODITIE							

Modified W-4 Alternative Total Crash Summary

Total Crashes between 2017 and 2036 244.9

116.198	6.7	6.6	6.5	6.4	6.3	6.2	6.1	6.0	5.9	5.8	5.7	5.7	5.6	5.5	5.4	5.3	5.3	5.2	5.1	5.0	Total (Crashes/year)
9.179	0.5	0.5	0.5	0.5	0.5	0.5	0.5	0.5	0.5	0.5	0.5	0.5	0.5	0.4	0.4	0.4	0.4	0.4	0.4	0.4	Total
1.023	0.054	0.053	0.053	0.053	0.053	0.052	0.052	0.052	0.052	0.051	0.051	0.051	0.050	0.050	0.050	0.050	0.049	0.049	0.049	0.049	North Clyde
0.692	0.036	0.036	0.036	0.036	0.036	0.035	0.035	0.035	0.035	0.035	0.034	0.034	0.034	0.034	0.034	0.034	0.033	0.033	0.033	0.033	Cameron
0.198	0.010	0.010	0.010	0.010	0.010	0.010	0.010	0.010	0.010	0.010	0.010	0.010	0.010	0.010	0.010	0.010	0.010	0.010	0.010	0.010	South Clyde
0.070	0.004	0.004	0.004	0.004	0.004	0.004	0.004	0.004	0.004	0.004	0.004	0.003	0.003	0.003	0.003	0.003	0.003	0.003	0.003	0.003	Skyline
0.141	0.007	0.007	0.007	0.007	0.007	0.007	0.007	0.007	0.007	0.007	0.007	0.007	0.007	0.007	0.007	0.007	0.007	0.007	0.007	0.007	Snow
0.224	0.011	0.011	0.011	0.011	0.011	0.011	0.011	0.011	0.011	0.011	0.011	0.011	0.011	0.011	0.011	0.011	0.011	0.011	0.011	0.011	Zeitler
Total	2036	2035	2034	2033	2032	2031	2030	2029	2028	2027	2026	2025	2024	2023	2022	2021	2020	2019	2018	2017	Intersection
3.306	0.181	0.179	0.177	0.176	0.174	0.172	0.171	0.169	0.167	0.166	0.164	0.163	0.161	0.160	0.158	0.157	0.155	0.154	0.152	0.151	22
1.749	0.092	0.092	0.091	0.091	0.090	0.090	0.089	0.089	0.088	0.088	0.087	0.087	0.086	0.086	0.085	0.085	0.084	0.084	0.083	0.083	21
0.624	0.033	0.033	0.032	0.032	0.032	0.032	0.032	0.032	0.031	0.031	0.031	0.031	0.031	0.031	0.031	0.030	0.030	0.030	0.030	0.030	20
0.095	0.005	0.005	0.005	0.005	0.005	0.005	0.005	0.005	0.005	0.005	0.005	0.005	0.005	0.005	0.005	0.005	0.005	0.005	0.005	0.005	19
0.335	0.017	0.017	0.017	0.017	0.017	0.017	0.017	0.017	0.017	0.017	0.017	0.017	0.017	0.017	0.017	0.017	0.017	0.016	0.016	0.016	18
0.103	0.005	0.005	0.005	0.005	0.005	0.005	0.005	0.005	0.005	0.005	0.005	0.005	0.005	0.005	0.005	0.005	0.005	0.005	0.005	0.005	17
0.167	0.009	0.008	0.008	0.008	0.008	0.008	0.008	0.008	0.008	0.008	0.008	0.008	0.008	0.008	0.008	0.008	0.008	0.008	0.008	0.008	16
0.076	0.004	0.004	0.004	0.004	0.004	0.004	0.004	0.004	0.004	0.004	0.004	0.004	0.004	0.004	0.004	0.004	0.004	0.004	0.004	0.004	15
0.135	0.007	0.007	0.007	0.007	0.007	0.007	0.007	0.007	0.007	0.007	0.007	0.007	0.007	0.007	0.007	0.007	0.007	0.007	0.007	0.007	14
0.059	0.003	0.003	0.003	0.003	0.003	0.003	0.003	0.003	0.003	0.003	0.003	0.003	0.003	0.003	0.003	0.003	0.003	0.003	0.003	0.003	13
0.134	0.007	0.007	0.007	0.007	0.007	0.007	0.007	0.007	0.007	0.007	0.007	0.007	0.007	0.007	0.007	0.007	0.007	0.007	0.007	0.007	12
0.047	0.002	0.002	0.002	0.002	0.002	0.002	0.002	0.002	0.002	0.002	0.002	0.002	0.002	0.002	0.002	0.002	0.002	0.002	0.002	0.002	11
Total	2036	2035	2034	2033	2032	2031	2030	2029	2028	2027	2026	2025	2024	2023	2022	2021	2020	2019	2018	2017	Segment
									1.800		1. 1. 1. 1. 1. 1. 1. 1. 1. 1. 1. 1. 1. 1		No. States	STAND STAND							Exisiting US-95
107.0	6.2	6.1	6.0	5.9	5.8	5.7	5.6	5.5	5.5	5.4	5.3	5.2	5.1	5.0	5.0	4.9	4.8	4.7	4.7	4.6	Total
3.351	0.196	0.193	0.189	0.186	0.183	0.180	0.177	0.174	0.171	0.168	0.165	0.162	0.160	0.157	0.154	0.152	0.149	0.147	0.145	0.142	North Old US-95
1.663	0.098	0.097	0.095	0.093	0.091	0.090	0.088	0.087	0.085	0.083	0.082	0.080	0.079	0.078	0.076	0.075	0.073	0.072	0.071	0.069	South Old US-95
3.292	0.195	0.191	0.188	0.184	0.181	0.178	0.175	0.171	0.168	0.165	0.162	0.159	0.156	0.153	0.151	0.148	0.145	0.143	0.140	0.137	Jacksha
3.536	0.209	0.206	0.202	0.198	0.194	0.191	0.187	0.184	0.181	0.177	0.174	0.171	0.168	0.165	0.162	0.159	0.156	0.153	0.150	0.148	Eid
Total	2036	2035	2034	2033	2032	2031	2030	2029	2028	2027	2026	2025	2024	2023	2022	2021	2020	2019	2018	2017	Intersection
7.886	0.454	0.447	0.440	0.433	0.427	0.420	0.414	0.408	0.402	0.396	0.390	0.384	0.378	0.373	0.367	0.362	0.356	0.351	0.346	0.341	Suburban
87.291	5.035	4.958	4.882	4.807	4.733	4.660	4.589	4.519	4.449	4.381	4.314	4.248	4.182	4.118	4.055	3.993	3.932	3.871	3.812	3.753	Rural Divided
Total	2036	2035	2034	2033	2032	2031	2030	2029	2028	2027	2026	2025	2024	2023	2022	2021	2020	2019	2018	2017	Segment
																					New Alignment
								Modified W-4 Alternative Total Fatal and Injury Crash Summary	Crash S	d Injury	Fatal an	le Total	Iternativ	d W-4 A	VIODITIE						

Modified W-4 Alternative Total Fatal and Injury Crash Summary

Total Crashes between 2017 and 2036 116.2

	Proposed Modified W4 Alternati	ve
Segment	Total Crashes	Fatal and Injury Crashes
Rural Divided	6.856	3.753
Suburban	1.123	0.341
Intersection	Total Crashes	Fatal and Injury Crashes
Eid Rd.	0.339	0.148
Jacksha Rd.	0.318	0.137
Old US-95 South	0.213	0.069
Old US-95 North	0.370	0.142
Subtotal	9.218	4.591

Modified W-4 Alternative Safety Summary 2017

Proposed Modified	W4 Alternative
Total (Crashes/year)	9.2
Fatal and Injury	4.6
Property Damage Only	4.6

	Existing US-95	
Segment	Total Crashes	Fatal and Injury Crashes
11	0.007	0.002
12	0.021	0.007
13	0.009	0.003
14	0.021	0.007
15	0.012	0.004
16	0.026	0.008
17	0.016	0.005
18	0.051	0.016
19	0.015	0.005
20	0.093	0.030
21	0.259	0.083
22	0.470	0.151
Intersection	Total Crashes	Fatal and Injury Crashes
Zeitler Rd.	0.026	0.011
Snow Rd.	0.017	0.007
Skyview Dr.	0.008	0.003
Clyde Rd. South	0.023	0.010
Cameron Rd.	0.079	0.033
Clyde Rd. North	0.117	0.049
Subtotal	1.269	0.433

Existing US-	95 Loop
Total (Crashes/year)	1.3
Fatal and Injury	0.4
Property Damage Only	0.8

Proposed Modified W4 Alternative	and Existing US-95 Loop
Total (Crashes/year)	10.5
Fatal and Injury	5.0
Property Damage Only	5.5

	Worksheet 1A General Information and Input Da	ta for Rural Multilane Road	lway Segments		
General	Information		Location Information		
Analyst Agency or Company	CJA, KJB ITD District 2	Roadway Roadway Section	US-95, Thorncreek to Moscow Modified W4 Rurat - Divided		
Date Performed	12/30/14	Jurisdiction Analysis Year	Latah Co, ID 2017		
Inp	ut Data	Base Conditions	Site Conditions		
Roadway type (divided / undivided)		Undivided	Divided		
Length of segment, L (mi)			6.35		
AADT (veh/day)	AADT _{MAX} = 89,300 (veh/day)		5,920		
Lane width (ft)		12	12		
Shoulder width (ft) - right shoulder width for divided	[if differ for directions of travel, use average width]	8	8		
Shoulder type - right shoulder type for divided		Paved	Paved		
Median width (ft) - for divided only		30	40		
Side Slopes - for undivided only		1:7 or flatter	Not Applicable		
Lighting (present/not present)		Not Present	Not Present		
Auto speed enforcement (present/not present)		Not Present	Not Present		
Calibration Factor, Cr		1.00	1.00		

	Worksheet 1B (a) Crash	h Modification Factors for R	ural Multilane Divided Ro	badway Segments	
(1)	(2)	(3)	(4)	(5)	(6)
CMF for Lane Width	CMF for Right Shoulder Width	CMF for Median Width	CMF for Lighting	CMF for Automated Speed Enforcement	Combined CMF
CMF 1rd	CMF 2rd	CMF 3rd	CMF 4rd	CMF 5rd	CMF comb
from Equation 11-16	from Table 11-17	from Table 11-18	from Equation 11-17	from Section 11.7.2	(1)*(2)*(3)*(4)*(5)
1.00	1.00	0.99	1.00	1.00	0.99

(1)	1	(2)		(3)	(4)	(5)	(6)	(7)
Crash Severity Level	SI	PF Coefficient	s	N spf rd	Overdispersion	Combined CMFs	Calibration	Predicted average crash
	fr	om Table 11-5	5		Parameter, k	(6) from Worksheet	Factor, Cr	frequency, N predicted raid)
	а	b	С	from Equation 11-9	from Equation 11-10	1B (a)		(3)*(5)*(6)
Total	-9.025	1.049	1.549	6.925	0.033	0.99	1.00	6.856
Fatal and Injury (FI)	-8.837	0.958	1.687	3.791	0.029	0.99	1.00	3.753
Fatal and Injury ^a (FI ^a)	-8.505	0.874	1.740	2.547	0.028	0.99	1.00	2.522
Property Damage Only (PDO)	-					-		(7) _{TOTAL} - (7) _{FI} 3.102

NOTE: * Using the KABCO scale, these include only KAB crashes. Crashes with severity level C (possible injury) are not included.

(1)	(2)	(3)	(4)	(5)	(6)	(7)	(8)	(9)
Collision Type	Proportion of Collision Type(TOTAL)		Proportion of Collision Type(FI)	N predicted rs(d) (Fi) (crashes/year)	Proportion of Collision Type (FI ^a)	N predicted rs (FI ^a) (crashes/year)	Proportion of Collision Type (PDO)	N predicted rs(d) (PDO) (crashes/year)
	from Table 11-6	(7)TOTAL from Worksheet 1C (a)	from Table 11- 6	(7)FI from Worksheet 1C (a)	from Table 11-6	(7) _{FI} ^a from Worksheet 1C (a)	from Table 11-6	(7)PDO from Worksheet 1C (a)
Total	1.000	6.856	1.000	3.753	1.000	2.522	1.000	3.102
		(2)*(3) _{TOTAL}		(4)x(5) _{F1}		(6)*(7) _{F1} ^a		(8)*(9) PDO
Head-on collision	0.006	0.041	0.013	0.049	0.018	0.045	0.002	0.006
Sideswipe collision	0.043	0.295	0.027	0.101	0.022	0.055	0.053	0.164
Rear-end collision	0.116	0.795	0.163	0.612	0.114	0.288	0.088	0.273
Angle collision	0.043	0.295	0.048	0.180	0.045	0.113	0.041	0.127
Single-vehicle collision	0.768	5.265	0.727	2.729	0.778	1.962	0.792	2.457
Other collision	0.024	0.165	0.022	0.083	0.023	0.058	0.024	0.074

NOTE: * Using the KABCO scale, these include only KAB crashes. Crashes with sevenity level C (possible injury) are not included.

	Worksheet 1E Summary Results for Rural Multila	ne Roadway Segments	
(1)	(2)	(3)	(4)
Crash severity level	Predicted average crash frequency (crashes/year)	Roadway segment length (mi)	Crash rate (crashes/mi/year)
-	(7) from Worksheet 1C (a) or (b)		(2)/(3)
Total	6.856	6.4	1.1
Fatal and Injury (FI)	3.753	6.4	0.6
Fatal and Injury ^a (FI ^a)	2.522	6.4	0.4
Property Damage Only (PDO)	3.102	6.4	0.5

NOTE: * Using the KABCO scale, these include only KAB crashes. Crashes with severity level C (possible injury) are not included.

	Proposed Modified W4 Alternat	ive
Segment	Total Crashes	Fatal and Injury Crashes
Rural Divided	6.973	3.812
Suburban	1.141	0.346
Intersection	Total Crashes	Fatal and Injury Crashes
Eid Rd.	0.345	0.150
Jacksha Rd.	0.325	0.140
Old US-95 South	0.217	0.071
Old US-95 North	0.376	0.145
Subtotal	9.377	4.663

Modified W-4 Alternative Safety Summary 2018

Proposed Modified W4 Alternative				
Total (Crashes/year)	9.4			
Fatal and Injury	4.7			
Property Damage Only	4.7			

Existing US-95			
Segment	Total Crashes	Fatal and Injury Crashes	
11	0.007	0.002	
12	0.021	0.007	
13	0.009	0.003	
14	0.021	0.007	
15	0.012	0.004	
16	0.026	0.008	
17	0.016	0.005	
18	0.051	0.016	
19	0.015	0.005	
20	0.093	0.030	
21	0.260	0.083	
22	0.475	0.152	
Intersection	Total Crashes	Fatal and Injury Crashes	
Zeitler Rd.	0.026	0.011	
Snow Rd.	0.017	0.007	
Skyview Dr.	0.008	0.003	
Clyde Rd. South	0.023	0.010	
Cameron Rd.	0.080	0.033	
Clyde Rd. North	0.118	0.049	
Subtotal	1.277	0.435	

Existing US-95 Loop				
Total (Crashes/year)	1.3			
Fatal and Injury	0.4			
Property Damage Only	0.8			

Proposed Modified W4 Alternative and Existing US-95 Loop				
Total (Crashes/year)	10.7			
Fatal and Injury	5.1			
Property Damage Only	5.6			

	Worksheet 1A General Information and Input Da	ta for Rural Multilarie Road			
Ge	eneral Information	Location Information			
Analyst Agency or Company	CJA, KJB ITD District 2	Roadway Roadway Section	US-95, Thorncreek to Moscow Modified W4 Rural - Divided		
Date Performed	12/30/14	Jurisdiction Analysis Year	Latah Co, ID 2018		
	Input Data	Base Conditions	Site Conditions		
Roadway type (divided / undivided)		Undivided	Divided		
Length of segment, L (mi)		-	6.35		
AADT (veh/day)	AADT _{MAX} = 89,300 (veh/day)	-	6,016		
Lane width (ft)		12	12		
Shoulder width (ft) - right shoulder width for	divided [if differ for directions of travel, use average width]	8	8		
Shoulder type - right shoulder type for divide	d	Paved	Paved		
Median width (ft) - for divided only		30	40		
Side Slopes - for undivided only		1:7 or flatter	Not Applicable		
Lighting (present/not present)		Not Present	Not Present		
Auto speed enforcement (present/not prese	nt)	Not Present	Not Present		
Calibration Factor, Cr		1.00	1.00		

Worksheet 1B (a) Crash Modification Factors for Rural Multilane Divided Roadway Segments					
(1)	(2)	(3)	(4)	(5)	(6)
CMF for Lane Width	CMF for Right Shoulder Width	CMF for Median Width	CMF for Lighting	CMF for Automated Speed Enforcement	Combined CMF
CMF 1rd	CMF 2rd	CMF 3rd	CMF 4rd	CMF 5rd	CMF comb
from Equation 11-16	from Table 11-17	from Table 11-18	from Equation 11-17	from Section 11.7.2	(1)*(2)*(3)*(4)*(5
1.00	1.00	0.99	1.00	1.00	0.99

(1)		(2)		(3)	(4)	(5)	(6)	(7)
Crash Severity Level	S	PF Coefficient	ts	N spf rd	Overdispersion	Combined CMFs	Calibration	Predicted average crash
	from Table 11-5		ole 11-5		Parameter, k	(6) from Worksheet	Factor, Cr	frequency, N predicted raid)
	а	b	С	from Equation 11-9	from Equation 11-10	1B (a)		(3)*(5)*(6)
Total	-9.025	1.049	1.549	7.043	0.033	0.99	1.00	6.973
Fatal and Injury (FI)	-8.837	0.958	1.687	3.850	0.029	0.99	1.00	3.812
Fatal and Injury ^a (Fl ^a)	-8.505	0.874	1.740	2.584	0.028	0.99	1.00	2.558
Property Damage Only (PDO)				-		_		(7) _{TOTAL} - (7) _{FI} 3.161

NOTE: * Using the KABCO scale, these include only KAB crashes. Crashes with severity level C (possible injury) are not included.

	Workshe	et 1D (a) Crashes by Seve	erity Level and	Collision Type for Ru	ral Multilane	Divided Roadway Segr	nents	
(1)	(2)	(3)	(4)	(5)	(6)	(7)	(8)	(9)
Collision Type	Proportion of Collision Type(TOTAL)	(Proportion of Collision Type(FI)	(crashes/year)	Proportion of Collision Type (Fl ^a)	N predicted rs (FI ^a) (crashes/year)	Proportion of Collision Type (PDO)	N predicted rs(d) (PDO) (crashes/year)
	from Table 11-6	(7)TOTAL from Worksheet 1C (a)	from Table 11- 6	(7)FI from Worksheet 1C (a)	from Table 11-6	(7) _{FI} ^a from Worksheet 1C (a)	from Table 11-6	(7)PDO from Worksheet 1C (a)
Total	1.000	6.973	1.000	3.812	1.000	2.558	1.000	3.161
		(2)*(3) _{TOTAL}		(4)x(5) _{F1}		(6)*(7) _{F1} ^a		(8)*(9) PDO
Head-on collision	0.006	0.042	0.013	0.050	0.018	0.046	0.002	0.006
Sideswipe collision	0.043	0.300	0.027	0.103	0.022	0.056	0.053	0.168
Rear-end collision	0.116	0.809	0.163	0.621	0.114	0.292	0.088	0.278
Angle collision	0.043	0.300	0.048	0.183	0.045	0.115	0.041	0.130
Single-vehicle collision	0.768	5.355	0.727	2.771	0.778	1.990	0.792	2.504
Other collision	0.024	0.167	0.022	0.084	0.023	0.059	0.024	0.076

NOTE: * Using the KABCO scale, these include only KAB crashes. Crashes with severity level C (possible injury) are not included.

	Worksheet 1E Summary Results for Rural Multila	ne Roadway Segments		
(1)	(2)	(3)	(4)	
Crash severity level	Predicted average crash frequency (crashes/year)	Roadway segment length (mi)	Crash rate (crashes/mi/year)	
	(7) from Worksheet 1C (a) or (b)	7 · · · · · · F	(2)/(3)	
Total	6.973	6.4	1.1	
Fatal and Injury (FI)	3.812	6.4	0.6	
Fatal and Injury ^a (FI ^a)	2.558	6.4	0.4	
Property Damage Only (PDO)	3.161	6.4	0.5	

NOTE: * Using the KABCO scale, these include only KAB crashes. Crashes with severity level C (possible injury) are not included.

Proposed Modified W4 Alternative				
Segment	Total Crashes	Fatal and Injury Crashes		
Rural Divided	7.092	3.871		
Suburban	1.159	0.351		
Intersection	Total Crashes	Fatal and Injury Crashes		
Eid Rd.	0.352	0.153		
Jacksha Rd.	0.331	0.143		
Old US-95 South	0.221	0.072		
Old US-95 North	0.383	0.147		
Subtotal	9.539	4.737		

Modified W-4 Alternative Safety Summary 2019

Proposed Modified W4 Alternative				
Total (Crashes/year)	9.5			
Fatal and Injury	4.7			
Property Damage Only	4.8			

Existing US-95			
Segment	Total Crashes	Fatal and Injury Crashes	
11	0.007	0.002	
12	0.021	0.007	
13	0.009	0.003	
14	0.021	0.007	
15	0.012	0.004	
16	0.026	0.008	
17	0.016	0.005	
18	0.051	0.016	
19	0.015	0.005	
20	0.094	0.030	
21	0.261	0.084	
22	0.479	0.154	
Intersection	Total Crashes	Fatal and Injury Crashes	
Zeitler Rd.	0.026	0.011	
Snow Rd.	0.017	0.007	
Skyview Dr.	0.008	0.003	
Clyde Rd. South	0.023	0.010	
Cameron Rd.	0.080	0.033	
Clyde Rd. North	0.119	0.049	
Subtotal	1.285	0.438	

Existing US-95 Loop			
Total (Crashes/year)	1.3		
Fatal and Injury	0.4		
Property Damage Only	0.8		

Proposed Modified W4 Alternative and Existing US-95 Loop			
Total (Crashes/year)	10.8		
Fatal and Injury	5.2		
Property Damage Only	5.6		

G	eneral Information	Location Information			
Analyst Agency or Company	CJA, KJB ITD District 2	ITD District 2 Roadway Section			
Date Performed	12/30/14	Jurisdiction Analysis Year	Latah Co, ID 2019		
	Input Data	Base Conditions	Site Conditions		
Roadway type (divided / undivided)		Undivided	Divided		
Length of segment, L (mi)			6.35		
AADT (veh/day)	AADT _{MAX} = 89,300 (veh/day)		6,114		
Lane width (ft)		12	12		
Shoulder width (ft) - right shoulder width for	divided [if differ for directions of travel, use average width]	8	8		
Shoulder type - right shoulder type for divid	ed	Paved	Paved		
Median width (ft) - for divided only		30	40		
Side Slopes - for undivided only		1:7 or flatter	Not Applicable		
Lighting (present/not present)		Not Present	Not Present		
Auto speed enforcement (present/not pres	ent)	Not Present	Not Present		
Calibration Factor, Cr		1.00	1.00		

Worksheet 1B (a) Crash Modification Factors for Rural Multilane Divided Roadway Segments					
(1)	(2)	(3)	(4)	(5)	(6)
CMF for Lane Width	CMF for Right Shoulder Width	CMF for Median Width	CMF for Lighting	CMF for Automated Speed Enforcement	Combined CMF
CMF 1rd	CMF 2rd	CMF 3rd	CMF 4rd	CMF 5rd	CMF comb
from Equation 11-16	from Table 11-17	from Table 11-18	from Equation 11-17	from Section 11.7.2	(1)*(2)*(3)*(4)*(5
1.00	1.00	0.99	1.00	1.00	0.99

(1)		(2)		(3)	(4)	(5)	(6)	(7)	
Crash Severity Level	SPF Coefficients from Table 11-5				N spf rd	Overdispersion	Combined CMFs	Calibration	Predicted average crash
			Parameter, k		(6) from Worksheet	Factor, Cr	frequency, N predicted ra(d)		
	а	b	С	from Equation 11-9	from Equation 11-10	1B (a)		(3)*(5)*(6)	
Total	-9.025	1.049	1.549	7.164	0.033	0.99	1.00	7.092	
Fatal and Injury (FI)	-8.837	0.958	1.687	3.910	0.029	0.99	1.00	3.871	
Fatal and Injury ^a (FI ^a)	-8.505	0.874	1.740	2.620	0.028	0.99	1.00	2.594	
Property Damage Only (PDO)				_	-			(7) _{TOTAL} - (7) _{FI}	
	-							3.221	

NOTE: * Using the KABCO scale, these include only KAB crashes. Crashes with severity level C (possible injury) are not included.

(1)	(2)	(3)	(4)	(5)	(6)	(7)	(8)	(9)
Collision Type	Proportion of Collision Type(TOTAL)	N prodicted rs(d) (TOTAL) (crashes/year)	Proportion of Collision Type(FI)	N predicted rs(d) (FI) (crashes/year)	Proportion of Collision	N predicted rs (FI ^a) (crashes/year)	Proportion of Collision Type (PDO)	N predicted rs(d) (PDO) (crashes/year)
	from Table 11-6	(7)TOTAL from Worksheet 1C (a)	from Table 11- 6	(7)FI from Worksheet 1C (a)	from Table 11-6	(7) _{Fl} ^a from Worksheet 1C (a)	from Table 11-6	(7)PDO from Worksheet 1C (a)
Total	1.000	7.092	1.000	3.871	1.000	2.594	1.000	3.221
		(2)*(3) _{TOTAL}		(4)x(5) _{F1}		(6)*(7) _{F1} ^a		(8)*(9) PDO
Head-on collision	0.006	0.043	0.013	0.050	0.018	0.047	0.002	0.006
Sideswipe collision	0.043	0.305	0.027	0.105	0.022	0.057	0.053	0.171
Rear-end collision	0.116	0.823	0.163	0.631	0.114	0.296	0.088	0.283
Angle collision	0.043	0.305	0.048	0.186	0.045	0.117	0.041	0.132
Single-vehicle collision	0.768	5.447	0.727	2.814	0.778	2.018	0.792	2.551
Other collision	0.024	0.170	0.022	0.085	0.023	0.060	0.024	0.077

NOTE: * Using the KABCO scale, these include only KAB crashes. Crashes with severity level C (possible injury) are not included.

Worksheet 1E – Summary Results for Rural Multilane Roadway Segments				
(1)	(2)	(3)	(4)	
Crash severity level	Predicted average crash frequency (crashes/year)	Roadway segment length (mi)	Crash rate (crashes/mi/year) (2)/(3)	
	(7) from Worksheet 1C (a) or (b)			
otal	7.092	6.4	1.1	
atal and Injury (FI)	3.871	6.4	0.6	
Fatal and Injury ^a (FI ^a)	2.594	6.4	0.4	
Property Damage Only (PDO)	3.221	6.4	0.5	

NOTE: * Using the KABCO scale, these include only KAB crashes. Crashes with severity level C (possible injury) are not included.

Proposed Modified W4 Alternative			
Segment	Total Crashes	Fatal and Injury Crashes	
Rural Divided	7.213	3.932	
Suburban	1.178	0.356	
Intersection	Total Crashes	Fatal and Injury Crashes	
Eid Rd.	0.359	0.156	
Jacksha Rd.	0.338	0.145	
Old US-95 South	0.226	0.073	
Old US-95 North	0.390	0.149	
Subtotal	9.703	4.812	

Modified W-4 Alternative Safety Summary 2020

Proposed Modified W4 Alternative			
Total (Crashes/year)	9.7		
Fatal and Injury	4.8		
Property Damage Only	4.9		

Existing US-95				
Segment	Total Crashes	Fatal and Injury Crashes		
11	0.007	0.002		
12	0.021	0.007		
13	0.009	0.003		
14	0.021	0.007		
15	0.012	0.004		
16	0.026	0.008		
17	0.016	0.005		
18	0.051	0.017		
19	0.015	0.005		
20	0.094	0.030		
21	0.263	0.084		
22	0.484	0.155		
Intersection	Total Crashes	Fatal and Injury Crashes		
Zeitler Rd.	0.027	0.011		
Snow Rd.	0.017	0.007		
Skyview Dr.	0.008	0.003		
Clyde Rd. South	0.024	0.010		
Cameron Rd.	0.081	0.033		
Clyde Rd. North	0.119	0.049		
Subtotal	1.293	0.441		

Existing US-95 Loop			
Total (Crashes/year)	1.3		
Fatal and Injury	0.4		
Property Damage Only	0.9		

Proposed Modified W4 Alternative and Existing US-95 Loop			
Total (Crashes/year)	11.0		
Fatal and Injury	5.3		
Property Damage Only	5.7		

	Seneral Information	Location Information			
Analyst Agency or Company	CJA, KJB ITD District 2	Roadway Roadway Section	US-95, Thorncreek to Moscow Modified W4 Rural - Divided		
Date Performed	12/30/14	Jurisdiction Analysis Year	Latah Co, ID 2020		
	Input Data	Base Conditions	Site Conditions		
Roadway type (divided / undivided)		Undivided	Divided		
Length of segment, L (mi)			6.35		
AADT (veh/day)	AADT _{MAX} = 89,300 (veh/day)	-	6,214		
Lane width (ft)		12	12		
Shoulder width (ft) - right shoulder width for	r divided [if differ for directions of travel, use average width]	8	8		
Shoulder type - right shoulder type for divi	ded	Paved	Paved		
Median width (ft) - for divided only		30	40		
Side Slopes - for undivided only		1:7 or flatter	Not Applicable		
Lighting (present/not present)		Not Present	Not Present		
Auto speed enforcement (present/not pres	ent)	Not Present	Not Present		
Calibration Factor, Cr		1.00	1.00		

Worksheet 1B (a) Crash Modification Factors for Rural Multilane Divided Roadway Segments						
(1)	(2)	(5)	(6)			
CMF for Lane Width	CMF for Right Shoulder Width	CMF for Median Width	CMF for Lighting	CMF for Automated Speed Enforcement	Combined CMF	
CMF 1rd	CMF 2rd	CMF 3rd	CMF 4rd	CMF 5rd	CMF comb	
from Equation 11-16	from Table 11-17	from Table 11-18	from Equation 11-17	from Section 11.7.2	(1)*(2)*(3)*(4)*(5)	
1.00	1.00	0.99	1.00	1.00	0.99	

(1)		(2)		(3)	(4)	(5)	(6)	(7)
Crash Severity Level	S	PF Coefficient	ts	N spf rd	Overdispersion	Combined CMFs	Calibration	Predicted average crash
	from Table 11-5			Parameter, k	(6) from Worksheet	Factor, Cr	frequency, N predicted rais	
	а	b	С	from Equation 11-9	from Equation 11-10	1B (a)		(3)*(5)*(6)
Total	-9.025	1.049	1.549	7.286	0.033	0.99	1.00	7.213
Fatal and Injury (FI)	-8.837	0.958	1.687	3.971	0.029	0.99	1.00	3.932
Fatal and Injury ^a (FI ^a)	-8.505	0.874	1.740	2.658	0.028	0.99	1.00	2.631
Property Damage Only (PDO)					-	_		(7) _{TOTAL} - (7) _{FI}
······································								3.281

NOTE: * Using the KABCO scale, these include only KAB crashes. Crashes with severity level C (possible injury) are not included.

	Worksheet 1D (a) Crashes by Severity Level and Collision Type for Rural Multilane Divided Roadway Segments								
(1)	(2)	(3)	(4)	(5)	(6)	(7)	(8)	(9)	
Collision Type	Proportion of Collision Type(TOTAL)	N predicted rs(d) (TOTAL) (crashes/year)	Proportion of Collision Type(FI)	N predicted rs(d) (FI) (crashes/year)	Proportion of Collision Type (Fl ^a)	N predicted rs (FI ^a) (crashes/year)	Proportion of Collision Type (PDO)	N predicted rs(d) (PDO) (crashes/year)	
	from Table 11-6	(7)TOTAL from Worksheet 1C (a)	from Table 11- 6	(7)FI from Worksheet 1C (a)	from Table 11-6	(7) _{Fl} ^a from Worksheet 1C (a)	from Table 11-6	(7)PDO from Worksheet 1C (a)	
Total	1.000	7.213	1.000	3.932	1.000	2.631	1.000	3.281	
		(2)*(3) _{TOTAL}		(4)x(5) _{FI}		(6)*(7) _{FI} ^a		(8)*(9) PDO	
Head-on collision	0.006	0.043	0.013	0.051	0.018	0.047	0.002	0.007	
Sideswipe collision	0.043	0.310	0.027	0.106	0.022	0.058	0.053	0.174	
Rear-end collision	0.116	0.837	0.163	0.641	0.114	0.300	0.088	0.289	
Angle collision	0.043	0.310	0.048	0.189	0.045	0.118	0.041	0.135	
Single-vehicle collision	0.768	5.540	0.727	2.858	0.778	2.047	0.792	2.599	
Other collision	0.024	0.173	0.022	0.086	0.023	0.061	0.024	0.079	

NOTE: * Using the KABCO scale, these include only KAB crashes. Crashes with severity level C (possible injury) are not included.

Worksheet 1E Summary Results for Rural Multilane Roadway Segments						
(1)	(2)	(3)	(4) Crash rate (crashes/mi/year)			
Crash severity level	Predicted average crash frequency (crashes/year)	Roadway segment length (mi)				
	(7) from Worksheet 1C (a) or (b)	7 7 7 7 7	(2)/(3)			
Total	7.213	6.4	1.1			
Fatal and Injury (FI)	3.932	6.4	0.6			
Fatal and Injury ^a (FI ^a)	2.631	6.4	0.4			
Property Damage Only (PDO)	3.281	6.4	0.5			

NOTE: * Using the KABCO scale, these include only KAB crashes. Crashes with severity level C (possible injury) are not included.

Proposed Modified W4 Alternative				
Segment	Total Crashes	Fatal and Injury Crashes		
Rural Divided	7.336	3.993		
Suburban	1.196	0.362		
Intersection	Total Crashes	Fatal and Injury Crashes		
Eid Rd.	0.367	0.159		
Jacksha Rd.	0.345	0.148		
Old US-95 South	0.230	0.075		
Old US-95 North	0.397	0.152		
Subtotal	9.871	4.888		

Modified W-4 Alternative Safety Summary 2021

Proposed Modified W4 Alternative				
Total (Crashes/year)	9.9			
Fatal and Injury	4.9			
Property Damage Only	5.0			

Existing US-95				
Segment	Total Crashes	Fatal and Injury Crashes		
11	0.007	0.002		
12	0.021	0.007		
13	0.009	0.003		
14	0.021	0.007		
15	0.012	0.004		
16	0.026	0.008		
17	0.016	0.005		
18	0.052	0.017		
19	0.015	0.005		
20	0.095	0.030		
21	0.264	0.085		
22	0.488	0.157		
Intersection	Total Crashes	Fatal and Injury Crashes		
Zeitler Rd.	0.027	0.011		
Snow Rd.	0.017	0.007		
Skyview Dr.	0.008	0.003		
Clyde Rd. South	0.024	0.010		
Cameron Rd.	Cameron Rd. 0.081 0			
Clyde Rd. North	0.120	0.050		
Subtotal	1.301	0.443		

Existing US-95 Loop				
Total (Crashes/year)	1.3			
Fatal and Injury	0.4			
Property Damage Only	0.9			

Proposed Modified W4 Alternative and Existing US-95 Loop				
Total (Crashes/year)	11.2			
Fatal and Injury	5.3			
Property Damage Only	5.8			

Gei	eral Information	Location Information			
Analyst Agency or Company Date Performed	CJA, KJB ITD District 2 12/30/14	Roadway Roadway Section Jurisdiction	US-95, Thorncreek to Moscow Modified W4 Rural - Divided Latah Co, ID		
		Analysis Year	2021		
	Input Data	Base Conditions	Site Conditions		
Roadway type (divided / undivided)		Undivided	Divided		
Length of segment, L (mi)			6.35		
AADT (veh/day)	AADT _{MAX} = 89,300 (veh/day)		6,315		
Lane width (ft)		12	12		
Shoulder width (ft) - right shoulder width for d	vided [if differ for directions of travel, use average width]	8	8		
Shoulder type - right shoulder type for divided		Paved	Paved		
Median width (ft) - for divided only		30	40		
Side Slopes - for undivided only		1:7 or flatter	Not Applicable		
Lighting (present/not present)		Not Present	Not Present		
Auto speed enforcement (present/not present)	Not Present	Not Present		
Calibration Factor, Cr		1.00	1.00		

Worksheet 1B (a) Crash Modification Factors for Rural Multilane Divided Roadway Segments						
(1)	(2)	(3)	(4)	(5)	(6)	
CMF for Lane Width	CMF for Right Shoulder Width	CMF for Median Width	CMF for Lighting	CMF for Automated Speed Enforcement	Combined CMF	
CMF 1rd	CMF 2rd	CMF 3rd	CMF 4rd	CMF 5rd	CMF comb	
from Equation 11-16	from Table 11-17	from Table 11-18	from Equation 11-17	from Section 11.7.2	(1)*(2)*(3)*(4)*(5	
1.00	1.00	0.99	1.00	1.00	0.99	

(1)	(2)		(3)	(4)	(5) Combined CMFs (6) from Worksheet	(6) Calibration Factor, Cr		
Crash Severity Level	SPF Coefficients from Table 11-5			N spf rd				Overdispersion Parameter, k
	Total	-9.025	1.049	1.549	7.410	0.033	0.99	1.00
Fatal and Injury (FI)	-8.837	0.958	1.687	4.033	0.029	0.99	1.00	3.993
Fatal and Injury ^a (FI ^a)	-8.505	0.874	1.740	2.695	0.028	0.99	1.00	2.668
Property Damage Only (PDO)	-	-	-		-		-	(7) _{TOTAL} - (7) _{F1} 3.343

NOTE: * Using the KABCO scale, these include only KAB crashes. Crashes with severity level C (possible injury) are not included.

(1)	(2)	(3)	(4)	(5)	(6)	(7)	(8)	(9)
Collision Type	Proportion of Collision Type(TOTAL)		Proportion of Collision Type(FI)	N predicted rs(d) (FI) (crashes/year)	Proportion of Collision Type (Fl ^a)	N predicted rs (FI ^a) (crashes/year)	Proportion of Collision Type (PDO)	(crashes/year)
	from Table 11-6	(7)TOTAL from Worksheet 1C (a)	from Table 11- 6	(7)FI from Worksheet 1C (a)	from Table 11-6	(7) _{Fl} ^a from Worksheet 1C (a)	from Table 11-6	(7)PDO from Worksheet 1C (a)
Total 1.000	1.000	7.336	1.000	3.993	1.000	2.668	1.000	3.343
		(2)*(3) _{TOTAL}		(4)x(5) _{F1}		(6)*(7) _{F1} ^a		(8)*(9) PDO
Head-on collision	0.006	0.044	0.013	0.052	0.018	0.048	0.002	0.007
Sideswipe collision	0.043	0.315	0.027	0.108	0.022	0.059	0.053	0.177
Rear-end collision	0.116	0.851	0.163	0.651	0.114	0.304	0.088	0.294
Angle collision	0.043	0.315	0.048	0.192	0.045	0.120	0.041	0.137
Single-vehicle collision	0.768	5.634	0.727	2.903	0.778	2.076	0.792	2.648
Other collision	0.024	0.176	0.022	0.088	0.023	0.061	0.024	0.080

NOTE: * Using the KABCO scale, these include only KAB crashes. Crashes with severity level C (possible injury) are not included.

Worksheet 1E – Summary Results for Rural Multilane Roadway Segments						
(1)	(2)	(3)	(4) Crash rate (crashes/mi/year) (2)/(3)			
Crash severity level	Predicted average crash frequency (crashes/year)	Roadway segment length (mi)				
	(7) from Worksheet 1C (a) or (b)					
Total	7.336	6.4	1.2			
Fatal and Injury (FI)	3.993	6.4	0.6			
Fatal and Injury ^a (FI ^a)	2.668	6.4	0.4			
Property Damage Only (PDO)	3.343	6.4	0.5			

NOTE: * Using the KABCO scale, these include only KAB crashes. Crashes with severity level C (possible injury) are not included.

Proposed Modified W4 Alternative				
Segment	Total Crashes	Fatal and Injury Crashes		
Rural Divided	7.461	4.055		
Suburban	1.216	0.367		
Intersection	Total Crashes	s Fatal and Injury Crashes		
Eid Rd.	0.374	0.162		
Jacksha Rd.	0.352			
Old US-95 South	US-95 South 0.235			
Old US-95 North	0.404	0.154		
Subtotal	10.041	4.965		

Proposed Modified W4 Alternative				
Total (Crashes/year)	10.0			
Fatal and Injury	5.0			
Property Damage Only	5.1			

Existing US-95			
Segment	Total Crashes	Fatal and Injury Crashes	
11	0.007	0.002	
12	0.021	0.007	
13	0.009	0.003	
14	0.021	0.007	
15	0.012	0.004	
16	0.026	0.008	
17	0.016	0.005	
18	0.052	0.017	
19	0.015	0.005	
20	0.095	0.031	
21	0.266	0.085	
22	0.493	0.158	
Intersection Total Crashes		Fatal and Injury Crashes	
Zeitler Rd.	0.027	0.011	
Snow Rd.	0.017	0.007	
Skyview Dr.	0.008	0.003	
Clyde Rd. South	0.024	0.010	
Cameron Rd.	0.081	0.034	
Clyde Rd. North	0.120	0.050	
Subtotal	1.309	0.446	

Existing US-95 Loop				
Total (Crashes/year)	1.3			
Fatal and Injury	0.4			
Property Damage Only	0.9			

Proposed Modified W4 Alternative and Existing US-95 Loop				
Total (Crashes/year)	11.4			
Fatal and Injury	5.4			
Property Damage Only	5.9			

	Worksheet 1A General Information and Input Da	ta for Rural Multilane Roa	adway Segments	
Gene	eral Information	Location Information		
Analyst Agency or Company	CJA, KJB ITD District 2	Roadway Roadway Section	US-95, Thorncreek to Moscow Modified W4 Rural - Divided	
Date Performed	12/30/14	Jurisdiction Analysis Year	Latah Co, ID 2022	
	Input Data	Base Conditions	Site Conditions	
Roadway type (divided / undivided)		Undivided	Divided	
Length of segment, L (mi)			6.35	
AADT (veh/day)	AADT _{MAX} = 89,300 (veh/day)	-	6,417	
Lane width (ft)		12	12	
Shoulder width (ft) - right shoulder width for div	ided [if differ for directions of travel, use average width]	8	8	
Shoulder type - right shoulder type for divided		Paved	Paved	
Median width (ft) - for divided only		30	40	
Side Slopes - for undivided only		1:7 or flatter	Not Applicable	
Lighting (present/not present)		Not Present	Not Present	
Auto speed enforcement (present/not present)		Not Present	Not Present	
Calibration Factor, Cr		1.00	1.00	

Worksheet 1B (a) Crash Modification Factors for Rural Multilane Divided Roadway Segments						
(1)	(2)	(3)	(4)	(5)	(6)	
CMF for Lane Width	CMF for Right Shoulder Width	CMF for Median Width	CMF for Lighting	CMF for Automated Speed Enforcement	Combined CMF	
CMF 1rd	CMF 2rd	CMF 3rd	CMF 4rd	CMF 5rd	CMF comb	
from Equation 11-16	from Table 11-17	from Table 11-18	from Equation 11-17	from Section 11.7.2	(1)*(2)*(3)*(4)*(5	
1.00	1.00	0.99	1.00	1.00	0.99	

(1)	(2)		(3)	(4)	(5)	(6)	(7)	
Crash Severity Level	S	PF Coefficient	ts	N spf rd	Overdispersion	Combined CMFs	Calibration	Predicted average crash
	f	rom Table 11-	5		Parameter, k	(6) from Worksheet	Factor, Cr	frequency, N predicted raid)
	а	b	С	from Equation 11-9	from Equation 11-10	1B (a)		(3)*(5)*(6)
Total	-9.025	1.049	1.549	7.537	0.033	0.99	1.00	7.461
Fatal and Injury (FI)	-8.837	0.958	1.687	4.096	0.029	0.99	1.00	4.055
Fatal and Injury ^a (FI ^a)	-8.505	0.874	1.740	2.734	0.028	0.99	1.00	2.706
Property Damage Only (PDO)	-	-	-				-	(7) _{TOTAL} - (7) _{FI} 3.406

(1)	(2)	(3)	(4)	(5)	(6)	(7)	(8)	(9)
Collision Type	Proportion of Collision Type(TOTAL)	N predicted rs(d) (TOTAL) (crashes/year)	Proportion of Collision Type(FI)	N predicted rs(d) (FI) (crashes/year)	Proportion of Collision Type (FI ^a)	N predicted rs (FI ^a) (crashes/year)	Proportion of Collision Type (PDO)	N predicted rs(d) (PDO) (crashes/year)
	from Table	(7)TOTAL from Worksheet 1C	from Table 11-	(7)FI from Worksheet	from Table	(7) FI ^a from Worksheet	from Table	(7)PDO from Worksheet 1C
	11-6	(a)	6	1C (a)	11-6	1C (a)	11-6	(a)
Total	1.000	7.461	1.000	4.055	1.000	2.706	1.000	3.406
		(2)*(3) _{TOTAL}		(4)x(5) _{FI}		(6)*(7) _{F1} ^a		(8)*(9) PDO
Head-on collision	0.006	0.045	0.013	0.053	0.018	0.049	0.002	0.007
Sideswipe collision	0.043	0.321	0.027	0.109	0.022	0.060	0.053	0.181
Rear-end collision	0.116	0.866	0.163	0.661	0.114	0.309	0.088	0.300
Angle collision	0.043	0.321	0.048	0.195	0.045	0.122	0.041	0.140
Single-vehicle collision	0.768	5.730	0.727	2.948	0.778	2.105	0.792	2.698
Other collision	0.024	0.179	0.022	0.089	0.023	0.062	0.024	0.082

NOTE: * Using the KABCO scale, these include only KAB crashes. Crashes with severity level C (possible injury) are not included.

Worksheet 1E Summary Results for Rural Multilane Roadway Segments					
(1)	(2)	(3)	(4)		
Crash severity level	Predicted average crash frequency (crashes/year)	Roadway segment length (mi)	Crash rate (crashes/mi/year) (2)/(3)		
	(7) from Worksheet 1C (a) or (b)				
Total	7.461	6.4	1.2		
Fatal and Injury (FI)	4.055	6.4	0.6		
Fatal and Injury ^a (FI ^a)	2.706	6.4	0.4		
Property Damage Only (PDO)	3.406	6.4	0.5		

Proposed Modified W4 Alternative				
Segment	Total Crashes	Fatal and Injury Crashes		
Rural Divided	7.589	4.118		
Suburban	1.235	0.373		
Intersection	Total Crashes	Fatal and Injury Crashes		
Eid Rd.	0.382	0.165		
Jacksha Rd.	0.359	0.153		
Old US-95 South 0.240		0.078		
Old US-95 North	0.411 0.			
Subtotal	10.215	5.044		

Proposed Modified W4 Alternative				
Total (Crashes/year)	10.2			
Fatal and Injury	5.0			
Property Damage Only	5.2			

Existing US-95				
Segment	Total Crashes	Fatal and Injury Crashes		
11	0.007	0.002		
12	0.021	0.007		
13	0.009	0.003		
14	0.021	0.007		
15	0.012	0.004		
16	0.026	0.008		
17	0.016	0.005		
18	0.052	0.017		
19	0.015	0.005		
20	0.096	0.031		
21	0.267	0.086		
22	0.497	0.160		
Intersection	Total Crashes	Fatal and Injury Crashes		
Zeitler Rd.	0.027	0.011		
Snow Rd.	0.017	0.007		
Skyview Dr.	0.008	0.003		
Clyde Rd. South	0.024	0.010		
Cameron Rd.	0.082	0.034		
Clyde Rd. North	0.121	0.050		
Subtotal	1.317	0.449		

Existing US-95 Loop				
Total (Crashes/year)	1.3			
Fatal and Injury	0.4			
Property Damage Only	0.9			

Proposed Modified W4 Alternative and Existing US-95 Loop				
Total (Crashes/year)	11.5			
Fatal and Injury	5.5			
Property Damage Only	6.0			

	Worksheet 1A General Information and Input Da	ta for Rural Multilane Road	way Segments		
Ge	neral Information		Location Information		
Analyst Agency or Company	ITD District 2 Roadway Section		US-95, Thorncreek to Moscow Modified W4 Rural - Divided		
Date Performed			Latah Co, ID 2023		
	Input Data	Base Conditions	Site Conditions		
Roadway type (divided / undivided)		Undivided	Divided		
Length of segment, L (mi)			6.35		
AADT (veh/day)	AADT _{MAX} = 89,300 (veh/day)		6,522		
Lane width (ft)		12	12		
Shoulder width (ft) - right shoulder width for d	ivided [if differ for directions of travel, use average width]	8	8		
Shoulder type - right shoulder type for divided	1	Paved	Paved		
Median width (ft) - for divided only		30	40		
Side Slopes - for undivided only		1:7 or flatter	Not Applicable		
Lighting (present/not present)		Not Present	Not Present		
Auto speed enforcement (present/not presen	t)	Not Present	Not Present		
Calibration Factor, Cr		1.00	1.00		

Worksheet 1B (a) Crash Modification Factors for Rural Multilane Divided Roadway Segments						
(1)	(2)	(3)	(4)	(5)	(6)	
CMF for Lane Width	CMF for Right Shoulder Width	CMF for Median Width	CMF for Lighting	CMF for Automated Speed Enforcement	Combined CMF	
CMF 1rd	CMF 2rd	CMF 3rd	CMF 4rd	CMF 5rd	CMF comb	
from Equation 11-16	from Table 11-17	from Table 11-18	from Equation 11-17	from Section 11.7.2	(1)*(2)*(3)*(4)*(5	
1.00	1.00	0.99	1.00	1.00	0.99	

(1)		(2)		(3)	(4)	(5)	(6)	(7)
Crash Severity Level	S	PF Coefficient	ts	N spf rd	Overdispersion	Combined CMFs	Calibration	Predicted average crash
	f	rom Table 11-	5		Parameter, k	(6) from Worksheet	Factor, Cr	frequency, N predicted ra(d)
	а	b	С	from Equation 11-9	from Equation 11-10	1B (a)		(3)*(5)*(6)
Total	-9.025	1.049	1.549	7.666	0.033	0.99	1.00	7.589
Fatal and Injury (FI)	-8.837	0.958	1.687	4.160	0.029	0.99	1.00	4.118
Fatal and Injury ^a (FI ^a)	-8.505	0.874	1.740	2.772	0.028	0.99	1.00	2.745
Property Damage Only (PDO)						-		(7) _{TOTAL} - (7) _{FI}
(100)								3.471

(1)	(2)	(3)	(4)	(5)	(6)	(7)	(8)	(9)
Collision Type	Proportion of Collision Type(TOTAL)	N prodicted rs(d) (TOTAL) (crashes/year)	Proportion of Collision Type(FI)	N predicted rs(d) (FI) (crashes/year)	Proportion of Collision Type (FI ^a)	N predicted rs (FI [*]) (crashes/year)	Proportion of Collision Type (PDO)	N predicted rs(d) (PDO) (crashes/year)
	from Table	(7)TOTAL from Worksheet 1C	from Table 11-	(7)FI from Worksheet	from Table	(7) FI ^a from Worksheet	from Table	(7)PDO from Worksheet 1C
	11-6	(a)	6	1C (a)	11-6	1C (a)	11-6	(a)
Total	1.000	7.589	1.000	4.118	1.000	2.745	1.000	3.471
		(2)*(3) _{TOTAL}		(4)x(5) _{F1}		(6)*(7) _{FI} ^a		(8)*(9) PDO
Head-on collision	0.006	0.046	0.013	0.054	0.018	0.049	0.002	0.007
Sideswipe collision	0.043	0.326	0.027	0.111	0.022	0.060	0.053	0.184
Rear-end collision	0.116	0.880	0.163	0.671	0.114	0.313	0.088	0.305
Angle collision	0.043	0.326	0.048	0.198	0.045	0.124	0.041	0.142
Single-vehicle collision	0.768	5.828	0.727	2.994	0.778	2.135	0.792	2.749
Other collision	0.024	0.182	0.022	0.091	0.023	0.063	0.024	0.083

NOTE: * Using the KABCO scale, these include only KAB crashes. Crashes with severity level C (possible injury) are not included.

Worksheet 1E Summary Results for Rural Multilane Roadway Segments					
(1)	(2)	(3)	(4)		
Crash severity level	Predicted average crash frequency (crashes/year)	Roadway segment length (mi)	Crash rate (crashes/mi/year) (2)/(3)		
- F	(7) from Worksheet 1C (a) or (b)	7			
Total	7.589	6.4	1.2		
Fatal and Injury (FI)	4.118	6.4	0.6		
Fatal and Injury ^a (FI ^a)	2.745	6.4	0.4		
Property Damage Only (PDO)	3.471	6.4	0.5		

Proposed Modified W4 Alternative				
Segment	Total Crashes	Fatal and Injury Crashes		
Rural Divided	7.718	4.182		
Suburban	1.255	0.378		
Intersection	Total Crashes	Fatal and Injury Crashes		
Eid Rd.	0.389	0.168		
Jacksha Rd.	0.366	0.156		
Old US-95 South 0.244 0.07		0.079		
Old US-95 North	0.419	0.160		
Subtotal	10.391	5.124		

Proposed Modified W4 Alternative				
Total (Crashes/year)	10.4			
Fatal and Injury	5.1			
Property Damage Only	5.3			

	Existing US-95				
Segment	Total Crashes	Fatal and Injury Crashes 0.002			
11	0.007				
12	0.021	0.007			
13	0.009	0.003			
14	0.021	0.007			
15	0.012	0.004			
16	0.026	0.008			
17	0.016	0.005			
18	0.052	0.017			
19	0.015	0.005			
20	0.096	0.031			
21	0.269	0.086			
22	0.502	0.161			
Intersection	Total Crashes	Fatal and Injury Crashes			
Zeitler Rd.	0.027	0.011			
Snow Rd.	0.017	0.007			
Skyview Dr.	0.008	0.003			
Clyde Rd. South	0.024	0.010			
Cameron Rd.	0.082	0.034			
Clyde Rd. North	0.122	0.050			
Subtotal	1.325	0.452			

Existing US-95 Loop				
Total (Crashes/year)	1.3			
Fatal and Injury	0.5			
Property Damage Only	0.9			

Proposed Modified W4 Alternative and Existing US-95 Loop				
Total (Crashes/year)	11.7			
Fatal and Injury	5.6			
Property Damage Only	6.1			

	Worksheet 1A General Information and Input Da	ta for Rural Multilane Roa	adway Segments	
Genera	al Information		Location Information	
Analyst Agency or Company	CJA, KJB ITD District 2	Roadway Roadway Section	US-95, Thorncreek to Moscow Modified W4 Rural - Divided	
Date Performed	12/30/14	Jurisdiction Analysis Year	Latah Co, ID 2024	
In	put Data	Base Conditions	Site Conditions	
Roadway type (divided / undivided)		Undivided	Divided	
Length of segment, L (mi)			6.35	
AADT (veh/day)	AADT _{MAX} = 89,300 (veh/day)		6,628	
Lane width (ft)		12	12	
Shoulder width (ft) - right shoulder width for divid	ed [if differ for directions of travel, use average width]	8	8	
Shoulder type - right shoulder type for divided		Paved	Paved	
Median width (ft) - for divided only		30	40	
Side Slopes - for undivided only		1:7 or flatter	Not Applicable	
Lighting (present/not present)		Not Present	Not Present	
Auto speed enforcement (present/not present)		Not Present	Not Present	
Calibration Factor, Cr		1.00	1.00	

Worksheet 1B (a) Crash Modification Factors for Rural Multilane Divided Roadway Segments					
(1)	(2)	(3)	(4)	(5)	(6)
CMF for Lane Width	CMF for Right Shoulder Width	CMF for Median Width	CMF for Lighting	CMF for Automated Speed Enforcement	Combined CMF
CMF 1rd	CMF 2rd	CMF 3rd	CMF 4rd	CMF 5rd	CMF comb
from Equation 11-16	from Table 11-17	from Table 11-18	from Equation 11-17	from Section 11.7.2	(1)*(2)*(3)*(4)*(5
1.00	1.00	0.99	1.00	1.00	0.99

(1)		(2)		(3)	(4)	(5)	(6)	(7)
Crash Severity Level		PF Coefficient		N spf rd	Overdispersion	Combined CMFs	Calibration	Predicted average crash
	f	rom Table 11-5	5		Parameter, k	(6) from Worksheet	Factor, Cr	frequency, N predicted ra(d)
	а	b	С	from Equation 11-9	from Equation 11-10	1B (a)		(3)*(5)*(6)
Total	-9.025	1.049	1.549	7.796	0.033	0.99	1.00	7.718
Fatal and Injury (FI)	-8.837	0.958	1.687	4.225	0.029	0.99	1.00	4.182
Fatal and Injury ^a (FI ^a)	-8.505	0.874	1.740	2.812	0.028	0.99	1.00	2.784
Property Damage Only (PDO)	-	-	-	-	-	-		(7) _{TOTAL} - (7) _{F1} 3.536

(1)	(2)	(3)	(4)	(5)	(6)	(7)	(8)	(9)
Collision Type	Proportion of Collision Type(TOTAL)	N predicted rs(d) (TOTAL) (crashes/year)	Proportion of Collision Type(FI)	N predicted rs(d) (FI) (crashes/year)	Proportion of Collision Type (FI ^a)	N predicted rs (FI ^a) (crashes/year)	Proportion of Collision Type (PDO)	N predicted rs(d) (PDO) (crashes/year)
	from Table	(7)TOTAL from Worksheet 1C		• /	from Table	(7) FI ^a from Worksheet	from Table	(7)PDO from Worksheet 1C
	11-6	(a)	6	1C (a)	11-6	1C (a)	11-6	(a)
Total	1.000	7.718	1.000	4.182	1.000	2.784	1.000	3.536
		(2)*(3) _{TOTAL}		(4)x(5) _{F1}		(6)*(7) _{FI} ^a		(8)*(9) _{PDO}
Head-on collision	0.006	0.046	0.013	0.054	0.018	0.050	0.002	0.007
Sideswipe collision	0.043	0.332	0.027	0.113	0.022	0.061	0.053	0.187
Rear-end collision	0.116	0.895	0.163	0.682	0.114	0.317	0.088	0.311
Angle collision	0.043	0.332	0.048	0.201	0.045	0.125	0.041	0.145
Single-vehicle collision	0.768	5.928	0.727	3.041	0.778	2.166	0.792	2.800
Other collision	0.024	0.185	0.022	0.092	0.023	0.064	0.024	0.085

NOTE: * Using the KABCO scale, these include only KAB crashes. Crashes with severity level C (possible injury) are not included.

Worksheet 1E Summary Results for Rural Multilane Roadway Segments					
(1)	(2)	(3)	(4)		
Crash severity level	Predicted average crash frequency (crashes/year)	Roadway segment length (mi)	Crash rate (crashes/mi/year) (2)/(3)		
	(7) from Worksheet 1C (a) or (b)	7			
Total	7.718	6.4	1.2		
Fatal and Injury (FI)	4.182	6.4	0.7		
Fatal and Injury ^a (FI ^a)	2.784	6.4	0.4		
Property Damage Only (PDO)	3.536	6.4	0.6		

Proposed Modified W4 Alternative				
Segment	Total Crashes	Fatal and Injury Crashes		
Rural Divided	7.850	4.248		
Suburban	1.275	0.384		
Intersection	Total Crashes	Fatal and Injury Crashes		
Eid Rd.	0.397	0.171		
Jacksha Rd.	0.373	0.159		
Old US-95 South	0.249	0.080		
Old US-95 North	0.426	0.162		
Subtotal	10.571	5.205		

Proposed Modified W4 Alternative				
Total (Crashes/year)	10.6			
Fatal and Injury	5.2			
Property Damage Only	5.4			

Existing US-95				
Segment	Total Crashes	Fatal and Injury Crashes		
11	0.007	0.002		
12	0.021	0.007		
13	0.009	0.003		
14	0.021	0.007		
15	0.012	0.004		
16	0.026	0.008		
17	0.016	0.005		
18	0.052	0.017		
19	0.015	0.005		
20	0.096	0.031		
21 0.270		0.087		
22	0.507	0.163		
Intersection	Total Crashes	Fatal and Injury Crashes		
Zeitler Rd.	0.027	0.011		
Snow Rd.	0.017	0.007		
Skyview Dr.	0.008	0.003		
Clyde Rd. South	0.024	0.010		
Cameron Rd.	0.083	0.034		
Clyde Rd. North	0.122	0.051		
Subtotal	1.333	0.454		

Existing US-95 Loop				
Total (Crashes/year)	1.3			
Fatal and Injury	0.5			
Property Damage Only	0.9			

Proposed Modified W4 Alternative and Existing US-95 Loop				
Total (Crashes/year)	11.9			
Fatal and Injury	5.7			
Property Damage Only	6.2			

	Worksheet 1A General Information and Input Da	ta for Rural Multilane Ro	adway Segments
	General Information		Location Information
Analyst Agency or Company	CJA, KJB ITD District 2	Roadway Roadway Section	US-95, Thorncreek to Moscow Modified W4 Rural - Divided
Date Performed	12/30/14 Jurisdiction Analysis Year		Latah Co, ID 2025
	Input Data	Base Conditions	Site Conditions
Roadway type (divided / undivided)		Undivided	Divided
Length of segment, L (mi)		-	6.35
AADT (veh/day)	AADT _{MAX} = 89,300 (veh/day)	-	6,736
Lane width (ft)		12	12
Shoulder width (ft) - right shoulder width for	r divided [if differ for directions of travel, use average width]	8	8
Shoulder type - right shoulder type for divi	ded	Paved	Paved
Median width (ft) - for divided only		30	40
Side Slopes - for undivided only		1:7 or flatter	Not Applicable
Lighting (present/not present)		Not Present	Not Present
Auto speed enforcement (present/not pres	sent)	Not Present	Not Present
Calibration Factor, Cr		1.00	1.00

Worksheet 1B (a) Crash Modification Factors for Rural Multilane Divided Roadway Segments						
(1)	(2)	(3)	(4)	(5)	(6)	
CMF for Lane Width	CMF for Right Shoulder Width	CMF for Median Width	CMF for Lighting	CMF for Automated Speed Enforcement	Combined CMF	
CMF 1rd	CMF 2rd	CMF 3rd	CMF 4rd	CMF 5rd	CMF comb	
from Equation 11-16	from Table 11-17	from Table 11-18	from Equation 11-17	from Section 11.7.2	(1)*(2)*(3)*(4)*(5	
1.00	1.00	0.99	1.00	1.00	0.99	

(1)	(1) (2)		(2) (3)		(3)	(4)	(5)	(6)	(7)
Crash Severity Level	S	PF Coefficient	s	N spf rd	Overdispersion	Combined CMFs	Calibration	Predicted average crash	
	from Table 11-5			Parameter, k	(6) from Worksheet	Factor, Cr	frequency, N predicted reid)		
	а	b	С	from Equation 11-9	from Equation 11-10	1B (a)		(3)*(5)*(6)	
Total	-9.025	1.049	1.549	7.929	0.033	0.99	1.00	7.850	
Fatal and Injury (FI)	-8.837	0.958	1.687	4.291	0.029	0.99	1.00	4.248	
Fatal and Injury ^a (FI ^a)	-8.505	0.874	1.740	2.852	0.028	0.99	1.00	2.823	
Property Damage Only (PDO)	-	-	-	-	-	-	-	(7) _{TOTAL} - (7) _{FI} 3.603	

Worksheet 1D (a) Crashes by Severity Level and Collision Type for Rural Multilane Divided Roadway Segments								
(1)	(2)	(3)	(4)	(5)	(6)	(7)	(8)	(9)
Collision Type	Proportion of Collision Type(TOTAL)	N predicted rs(d) (TOTAL) (crashes/year)	Proportion of Collision Type(FI)	N predicted rs(d) (FI) (crashes/year)	Proportion of Collision Type (FI ^a)	N predicted rs (FI ^a) (crashes/year)	Proportion of Collision Type (PDO)	N predicted rs(d) (PDO) (crashes/year)
	from Table 11-6	(7)TOTAL from Worksheet 1C (a)	from Table 11- 6	(7)FI from Worksheet 1C (a)	from Table 11-6	(7) FI ^a from Worksheet 1C (a)	from Table 11-6	(7)PDO from Worksheet 1C (a)
Total	1.000	7.850	1.000	4.248	1.000	2.823	1.000	3.603
		(2)*(3) _{TOTAL}		(4)x(5) _{F1}		(6)*(7) _{F1} ^a		(8)*(9) PDO
Head-on collision	0.006	0.047	0.013	0.055	0.018	0.051	0.002	0.007
Sideswipe collision	0.043	0.338	0.027	0.115	0.022	0.062	0.053	0.191
Rear-end collision	0.116	0.911	0.163	0.692	0.114	0.322	0.088	0.317
Angle collision	0.043	0.338	0.048	0.204	0.045	0.127	0.041	0.148
Single-vehicle collision	0.768	6.029	0.727	3.088	0.778	2.196	0.792	2.853
Other collision	0.024	0.188	0.022	0.093	0.023	0.065	0.024	0.086

NOTE: * Using the KABCO scale, these include only KAB crashes. Crashes with severity level C (possible injury) are not included.

Worksheet 1E Summary Results for Rural Multilane Roadway Segments							
(1)	(2)	(3)	(4)				
Crash severity level	Predicted average crash frequency (crashes/year)	Roadway segment length (mi)	Crash rate (crashes/mi/year)				
	(7) from Worksheet 1C (a) or (b)		(2)/(3)				
Total	7.850	6.4	1.2				
Fatal and Injury (FI)	4.248	6.4	0.7				
Fatal and Injury ^a (Fl ^a)	2.823	6.4	0.4				
Property Damage Only (PDO)	3.603	6.4	0.6				

	Proposed Modified W4 Alternat	ve
Segment	Total Crashes	Fatal and Injury Crashes
Rural Divided	7.984	4.314
Suburban	1.296	0.390
Intersection	Total Crashes	Fatal and Injury Crashes
Eid Rd.	0.405	0.174
Jacksha Rd.	0.381	0.162
Old US-95 South	0.254	0.082
Old US-95 North	0.434	0.165
Subtotal	10.754	5.287

Proposed Modified W4 Alternative				
Total (Crashes/year)	10.8			
Fatal and Injury	5.3			
Property Damage Only	5.5			

Existing US-95				
Segment	Total Crashes	Fatal and Injury Crashes		
11	0.007	0.002		
12	0.021	0.007		
13	0.009	0.003		
14	0.021	0.007		
15	0.012	0.004		
16	0.026	0.008		
17	0.016	0.005		
18	0.052	0.017		
19	0.015	0.005		
20	0.097	0.031		
21	0.272	0.087		
22	0.512	0.164		
Intersection	Total Crashes	Fatal and Injury Crashes		
Zeitler Rd.	0.027	0.011		
Snow Rd.	0.017	0.007		
Skyview Dr.	0.008	0.004		
Clyde Rd. South	0.024	0.010		
Cameron Rd.	0.083	0.034		
Clyde Rd. North	0.123	0.051		
Subtotal	1.342	0.457		

Existing US-95 Loop					
Total (Crashes/year)	1.3				
Fatal and Injury	0.5				
Property Damage Only	0.9				

Proposed Modified W4 Alternative and Existing US-95 Loop				
Total (Crashes/year)	12.1			
Fatal and Injury	5.7			
Property Damage Only	6.4			

	Worksheet 1A General Information and Input Da	ta for Rural Multilane Roa	adway Segments		
Ger	neral Information	Location Information			
Analyst Agency or Company Date Performed	CJA, KJB ITD District 2 12/30/14	Roadway Roadway Section Jurisdiction Analysis Year	US-95, Thorncreek to Moscow Modified W4 Rural - Divided Latah Co, ID 2026		
	Input Data	Base Conditions	Site Conditions		
Roadway type (divided / undivided)		Undivided	Divided		
Length of segment, L (mi)			6.35		
AADT (veh/day)	AADT _{MAX} = 89,300 (veh/day)	-	6,845		
Lane width (ft)		12	12		
Shoulder width (ft) - right shoulder width for di	ivided [if differ for directions of travel, use average width]	8	8		
Shoulder type - right shoulder type for divided		Paved	Paved		
Median width (ft) - for divided only		30	40		
Side Slopes - for undivided only		1:7 or flatter	Not Applicable		
Lighting (present/not present)		Not Present	Not Present		
Auto speed enforcement (present/not presen	t)	Not Present	Not Present		
Calibration Factor, Cr		1.00	1.00		

Worksheet 1B (a) Crash Modification Factors for Rural Multilane Divided Roadway Segments						
(1)	(2)	(3)	(4)	(5)	(6)	
CMF for Lane Width	CMF for Right Shoulder Width	CMF for Median Width	CMF for Lighting	CMF for Automated Speed Enforcement	Combined CMF	
CMF 1rd	CMF 2rd	CMF 3rd	CMF 4rd	CMF 5rd	CMF comb	
from Equation 11-16	from Table 11-17	from Table 11-18	from Equation 11-17	from Section 11.7.2	(1)*(2)*(3)*(4)*(5	
1.00	1.00	0.99	1.00	1.00	0.99	

(1)		(2)		(3)	(4)	(5)	(6)	(7)
Crash Severity Level	S	PF Coefficient	s	N spf rd	Overdispersion	Combined CMFs	Calibration	Predicted average crash
	from Table 11-5			Parameter, k	(6) from Worksheet	Factor, Cr	frequency, N predicted ra(d)	
	а	b	С	from Equation 11-9	from Equation 11-10	1B (a)		(3)*(5)*(6)
Total	-9.025	1.049	1.549	8.065	0.033	0.99	1.00	7.984
Fatal and Injury (FI)	-8.837	0.958	1.687	4.357	0.029	0.99	1.00	4.314
Fatal and Injury ^a (FI ^a)	-8.505	0.874	1.740	2.892	0.028	0.99	1.00	2.863
Property Damage Only (PDO)	-	-	-	-	-	-	-	(7) _{TOTAL} - (7) _{FI} 3.670

(1)	(2)	(3)	(4)	(5)	(6)	(7)	(8)	(9)
of C	Proportion of Collision Type(TOTAL)	N predicted rs(d) (TOTAL) (crashes/year)	Proportion of Collision Type(FI)	N predicted rs(d) (FI) (crashes/year)	Proportion of Collision Type (FI ^a)	N predicted rs (FI ^a) (crashes/year)	Proportion of Collision Type (PDO)	N predicted re(d) (PDO) (crashes/year)
	from Table	(7)TOTAL from Worksheet 1C	from Table 11-	(7)FI from Worksheet	from Table	(7) FI ^a from Worksheet	from Table	(7)PDO from Worksheet 1C
	11-6	(a)	6	1C (a)	11-6	1C (a)	11-6	(a)
Total	1.000	7.984	1.000	4.314	1.000	2.863	1.000	3.670
		(2)*(3) _{TOTAL}		(4)x(5) _{F1}		(6)*(7) _{F1} ^a		(8)*(9) PDO
Head-on collision	0.006	0.048	0.013	0.056	0.018	0.052	0.002	0.007
Sideswipe collision	0.043	0.343	0.027	0.116	0.022	0.063	0.053	0.195
Rear-end collision	0.116	0.926	0.163	0.703	0.114	0.326	0.088	0.323
Angle collision	0.043	0.343	0.048	0.207	0.045	0.129	0.041	0.150
Single-vehicle collision	0.768	6.132	0.727	3.136	0.778	2.228	0.792	2.907
Other collision	0.024	0.192	0.022	0.095	0.023	0.066	0.024	0.088

NOTE: * Using the KABCO scale, these include only KAB crashes. Crashes with severity level C (possible injury) are not included.

	Worksheet 1E Summary Results for Rural Multila	ne Roadway Segments		
(1)	(2)	(3)	(4) Crash rate (crashes/mi/year)	
Crash severity level	Predicted average crash frequency (crashes/year)	Roadway segment length (mi)		
	(7) from Worksheet 1C (a) or (b)		(2)/(3)	
Fotal	7.984	6.4	1.3	
Fatal and Injury (FI)	4.314	6.4	0.7	
Fatal and Injury ^a (FI ^a)	2.863	6.4	0.5	
Property Damage Only (PDO)	3.670	6.4	0.6	

Proposed Modified W4 Alternative						
Segment	Total Crashes	Fatal and Injury Crashes				
Rural Divided	8.121	4.381				
Suburban	1.317	0.396				
Intersection	Total Crashes	Fatal and Injury Crashes				
Eid Rd.	0.413	0.177				
Jacksha Rd.	0.388	0.165				
Old US-95 South	0.259	0.083				
Old US-95 North	0.442	0.168				
Subtotal	10.940	5.371				

Proposed Modified	W4 Alternative
Total (Crashes/year)	10.9
Fatal and Injury	5.4
Property Damage Only	5.6

Existing US-95						
Segment	Total Crashes	Fatal and Injury Crashes				
11	0.007	0.002				
12	0.021	0.007				
13	0.009	0.003				
14	0.021	0.007				
15	0.012	0.004				
16	0.026	0.008				
17	0.016	0.005				
18	0.052	0.017				
19	0.015	0.005				
20	0.097	0.031				
21	0.273	0.088				
22	0.517	0.166				
Intersection	Total Crashes	Fatal and Injury Crashes				
Zeitler Rd.	0.027	0.011				
Snow Rd.	0.017	0.007				
Skyview Dr.	0.008	0.004				
Clyde Rd. South	0.024	0.010				
Cameron Rd.	0.084	0.035				
Clyde Rd. North	0.124	0.051				
Subtotal	1.350	0.460				

Existing US-	95 Loop
Total (Crashes/year)	1.3
Fatal and Injury	0.5
Property Damage Only	0.9

Proposed Modified W4 Alternative and Existing US-95 Loop					
Total (Crashes/year)	12.3				
Fatal and Injury	5.8				
Property Damage Only	6.5				

	Worksheet 1A General Information and Input Da	ta for Rural Multilane Roady	way Segments		
Genera	Information	Location Information			
Analyst Agency or Company	CJA, KJB ITD District 2	Roadway Roadway Section	US-95, Thorncreek to Moscow Modified W4 Rural - Divided		
Date Performed	12/30/14	Jurisdiction Analysis Year	Latah Co, ID 2027		
Ing	out Data	Base Conditions	Site Conditions		
Roadway type (divided / undivided)		Undivided	Divided		
Length of segment, L (mi)			6.35		
AADT (veh/day)	AADT _{MAX} = 89,300 (veh/day)	-	6,957		
Lane width (ft)	Autoria and and a second and a second and a later	12	12		
Shoulder width (ft) - right shoulder width for divide	d [if differ for directions of travel, use average width]	8	8		
Shoulder type - right shoulder type for divided		Paved	Paved		
Median width (ft) - for divided only		30	40		
Side Slopes - for undivided only		1:7 or flatter	Not Applicable		
Lighting (present/not present)		Not Present	Not Present		
Auto speed enforcement (present/not present)		Not Present	Not Present		
Calibration Factor, Cr		1.00	1.00		

Worksheet 1B (a) Crash Modification Factors for Rural Multilane Divided Roadway Segments							
(1)	(2)	(3)	(4)	(5)	(6)		
CMF for Lane Width	CMF for Right Shoulder Width	CMF for Median Width	CMF for Lighting	CMF for Automated Speed Enforcement	Combined CMF		
CMF 1rd	CMF 2rd	CMF 3rd	CMF 4rd	CMF 5rd	CMF comb		
from Equation 11-16	from Table 11-17	from Table 11-18	from Equation 11-17	from Section 11.7.2	(1)*(2)*(3)*(4)*(5		
1.00	1.00	0.99	1.00	1.00	0.99		

(1) (2)		(3)	(4)	(5)	(6)	(7)		
Crash Severity Level	SPF Coefficients from Table 11-5		N spf rd	Overdispersion	Combined CMFs	Calibration Factor, Cr	Predicted average crash frequency, N predicted raid	
				Parameter, k	(6) from Worksheet			
	а	b	С	from Equation 11-9	from Equation 11-10	1B (a)		(3)*(5)*(6)
Total	-9.025	1.049	1.549	8.203	0.033	0.99	1.00	8.121
Fatal and Injury (FI)	-8.837	0.958	1.687	4.425	0.029	0.99	1.00	4.381
Fatal and Injury ^a (FI ^a)	-8.505	0.874	1.740	2.933	0.028	0.99	1.00	2.904
Property Damage Only (PDO)	-	-	-	-	-		-	(7) _{TOTAL} - (7) _{FI} 3.740

	Workshee	et 1D (a) Crashes by Seve	erity Level and	Collision Type for Ru	ral Multilane	Divided Roadway Segr	nents	
(1)	(2)	(3)	(4)	(5)	(6)	(7)	(8)	(9)
Collision Type	Proportion of Collision Type(TOTAL)	N predicted rs(d) (TOTAL) (crashes/year)	Proportion of Collision Type(FI)	N predicted rs(d) (Fi) (crashes/year)	Proportion of Collision Type (FI ^a)	N predicted rs (FI ^a) (crashes/year)	Proportion of Collision Type (PDO)	N predicted rs(d) (PDO) (crashes/year)
	from Table	(7)TOTAL from Worksheet 1C	from Table 11-	(7)FI from Worksheet	from Table	(7) FI ^a from Worksheet	from Table	(7)PDO from Worksheet 1C
	11-6	(a)	6	1C (a)	11-6	1C (a)	11-6	(a)
Total	1.000	8.121	1.000	4.381	1.000	2.904	1.000	3.740
		(2)*(3) _{TOTAL}		(4)x(5) _{F1}		(6)*(7) _{FI} ^a		(8)*(9) PDO
Head-on collision	0.006	0.049	0.013	0.057	0.018	0.052	0.002	0.007
Sideswipe collision	0.043	0.349	0.027	0.118	0.022	0.064	0.053	0.198
Rear-end collision	0.116	0.942	0.163	0.714	0.114	0.331	0.088	0.329
Angle collision	0.043	0.349	0.048	0.210	0.045	0.131	0.041	0.153
Single-vehicle collision	0.768	6.237	0.727	3.185	0.778	2.259	0.792	2.962
Other collision	0.024	0.195	0.022	0.096	0.023	0.067	0.024	0.090

NOTE: * Using the KABCO scale, these include only KAB crashes. Crashes with severity level C (possible injury) are not included.

Worksheet 1E Summary Results for Rural Multilane Roadway Segments						
(1)	(2)	(3)	(4)			
Crash severity level	Predicted average crash frequency (crashes/year)	Roadway segment length (mi)	Crash rate (crashes/mi/year)			
	(7) from Worksheet 1C (a) or (b)		(2)/(3)			
Fotal	8.121	6.4	1.3			
Fatal and Injury (FI)	4.381	6.4	0.7			
Fatal and Injury ^a (FI ^a)	2.904	6.4	0.5			
Property Damage Only (PDO)	3.740	6.4	0.6			

Segment	Total Crashes	Fatal and Injury Crashes		
Rural Divided	8.259 4.4			
Suburban	1.338	0.402		
Intersection	Total Crashes	Fatal and Injury Crashes		
Eid Rd.	0.421	0.181		
Jacksha Rd.	0.396	0.168		
Old US-95 South	0.265			
Old US-95 North	0.450	0.171		
Subtotal	11.129	5.456		

Proposed Modified W4 Alternative				
Total (Crashes/year)	11.1			
Fatal and Injury	5.5			
Property Damage Only	5.7			

Existing US-95			
Segment	Total Crashes	Fatal and Injury Crashes	
11	0.007	0.002	
12	0.021	0.007	
13	0.009	0.003	
14	0.021	0.007	
15	0.012	0.004	
16	0.026	0.008	
17	0.016	0.005	
18	0.052	0.017	
19	0.015	0.005	
20	0.098	0.031	
21	0.275	0.088	
22	0.521	0.167	
Intersection	Total Crashes	Fatal and Injury Crashes	
Zeitler Rd.	0.027	0.011	
Snow Rd.	0.017	0.007	
Skyview Dr.	0.009	0.004	
Clyde Rd. South 0.024		0.010	
Cameron Rd.	0.084	0.035	
Clyde Rd. North	0.124	0.052	
Subtotal	1.358	0.463	

Existing US-95 Loop				
Total (Crashes/year)	1.4			
Fatal and Injury	0.5			
Property Damage Only	0.9			

Proposed Modified W4 Alternative and Existing US-95 Loop				
Total (Crashes/year)	12.5			
Fatal and Injury	5.9			
Property Damage Only	6.6			

	Worksheet 1A General Information and Input Da	ita for Rural Multilane Ro	adway Segments	
G	eneral Information	Location Information		
Analyst Agency or Company Date Performed	CJA, KJB ITD District 2 12/30/14	Roadway Roadway Section Jurisdiction	US-95, Thorncreek to Moscow Modified W4 Rural - Divided Latah Co. ID	
Bate i chomica	12/00/14	Analysis Year	2028	
	Input Data	Base Conditions	Site Conditions	
Roadway type (divided / undivided)		Undivided	Divided	
Length of segment, L (mi)			6.35	
AADT (veh/day)	AADT _{MAX} = 89,300 (veh/day)		7,070	
Lane width (ft)		12	12	
Shoulder width (ft) - right shoulder width for	divided [if differ for directions of travel, use average width]	8	8	
Shoulder type - right shoulder type for divid	ed	Paved	Paved	
Median width (ft) - for divided only		30	40	
Side Slopes - for undivided only		1:7 or flatter	Not Applicable	
Lighting (present/not present)		Not Present	Not Present	
Auto speed enforcement (present/not prese	ent)	Not Present	Not Present	
Calibration Factor, Cr		1.00	1.00	

Worksheet 1B (a) Crash Modification Factors for Rural Multilane Divided Roadway Segments						
(1)	(2)	(3)	(4)	(5)	(6)	
CMF for Lane Width	CMF for Right Shoulder Width	CMF for Median Width	CMF for Lighting	CMF for Automated Speed Enforcement	Combined CMF	
CMF 1rd	CMF 2rd	CMF 3rd	CMF 4rd	CMF 5rd	CMF comb	
from Equation 11-16	from Table 11-17	from Table 11-18	from Equation 11-17	from Section 11.7.2	(1)*(2)*(3)*(4)*(5)	
1.00	1.00	0.99	1.00	1.00	0.99	

(1)	(2)		(3)	(4)	(5)	(6)	(7)	
Crash Severity Level	S	PF Coefficient	ts	N spf rd	Overdispersion	Combined CMFs	Calibration	Predicted average crash
	f	rom Table 11-	5		Parameter, k	(6) from Worksheet	Factor, Cr	frequency, N predicted raid)
	а	b	С	from Equation 11-9	from Equation 11-10	1B (a)		(3)*(5)*(6)
Total	-9.025	1.049	1.549	8.343	0.033	0.99	1.00	8.259
Fatal and Injury (FI)	-8.837	0.958	1.687	4.494	0.029	0.99	1.00	4.449
Fatal and Injury ^a (FI ^a)	-8.505	0.874	1.740	2.975	0.028	0.99	1.00	2.945
Property Damage Only (PDO)		-			-			(7) _{TOTAL} - (7) _{FI} 3.810

(1)	(2)	(3)	(4)	(5)	(6)	(7)	(8)	(9)
Collision Type	Proportion of Collision Type(TOTAL)		Proportion of Collision Type(FI)	N predicted rs(d) (FI) (crashes/year)	Proportion of Collision Type (FI ^a)	N predicted rs (FI ^a) (crashes/year)	Proportion of Collision Type (PDO)	N predicted rs(d) (PDO) (crashes/year)
	from Table	(7)TOTAL from Worksheet 1C	from Table 11-	(7)FI from Worksheet	from Table	(7) FI ^a from Worksheet	from Table	(7)PDO from Worksheet 1C
	11-6	(a)	6	1C (a)	11-6	1C (a)	11-6	(a)
Total	1.000	8.259	1.000	4.449	1.000	2.945	1.000	3.810
		(2)*(3) _{TOTAL}		(4)x(5) _{F1}		(6)*(7) _{F1} ^a		(8)*(9) PDO
Head-on collision	0.006	0.050	0.013	0.058	0.018	0.053	0.002	0.008
Sideswipe collision	0.043	0.355	0.027	0.120	0.022	0.065	0.053	0.202
Rear-end collision	0.116	0.958	0.163	0.725	0.114	0.336	0.088	0.335
Angle collision	0.043	0.355	0.048	0.214	0.045	0.133	0.041	0.156
Single-vehicle collision	0.768	6.343	0.727	3.235	0.778	2.291	0.792	3.017
Other collision	0.024	0.198	0.022	0.098	0.023	0.068	0.024	0.091

NOTE: * Using the KABCO scale, these include only KAB crashes. Crashes with severity level C (possible injury) are not included.

Worksheet 1E Summary Results for Rural Multilane Roadway Segments						
(1)	(2)	(3)	(4)			
Crash severity level	Predicted average crash frequency (crashes/year)	Roadway segment length (mi)	Crash rate (crashes/mi/year) (2)/(3) 1.3			
	(7) from Worksheet 1C (a) or (b)					
Total	8.259	6.4				
Fatal and Injury (FI)	4.449	6.4	0.7			
Fatal and Injury ^a (FI ^a)	2.945	6.4	0.5			
Property Damage Only (PDO)	3.810	6.4	0.6			

Proposed Modified W4 Alternative				
Segment	Total Crashes	Fatal and Injury Crashes		
Rural Divided	8.400	4.519		
Suburban	1.360	0.408		
Intersection	Total Crashes	Fatal and Injury Crashes		
Eid Rd.	0.430	0.184		
Jacksha Rd.	0.404 0.1			
Old US-95 South	0.270	0.087		
Old US-95 North	0.458	0.174		
Subtotal	11.322	5.542		

Proposed Modified W4 Alternative				
Total (Crashes/year)	11.3			
Fatal and Injury	5.5			
Property Damage Only	5.8			

Existing US-95			
Segment	Total Crashes	Fatal and Injury Crashes	
11	0.007	0.002	
12	0.021	0.007	
13	0.009	0.003	
14	0.021	0.007	
15	0.012	0.004	
16	0.026	0.008	
17	0.016	0.005	
18	0.052	0.017	
19	0.015	0.005	
20	0.098	0.032	
21	0.276 0.0		
22	0.526	0.169	
Intersection	Total Crashes	Fatal and Injury Crashes	
Zeitler Rd.	0.027	0.011	
Snow Rd.	0.017	0.007	
Skyview Dr.	0.009	0.004	
Clyde Rd. South	0.024	0.010	
Cameron Rd.	0.084	0.035	
Clyde Rd. North	0.125	0.052	
Subtotal	1.367	0.466	

Existing US-95 Loop					
Total (Crashes/year)	1.4				
Fatal and Injury	0.5				
Property Damage Only	0.9				

Proposed Modified W4 Alternative and Existing US-95 Loop				
Total (Crashes/year)	12.7			
Fatal and Injury	6.0			
Property Damage Only	6.7			

	Worksheet 1A General Information and Input Da	ta for Rural Multilane Road	Iway Segments		
Gene	ral Information	Location Information			
Analyst Agency or Company	Dmpany CJA, KJB Roadway ITD District 2 Roadway Section		US-95, Thorncreek to Moscow Modified W4 Rural - Divided		
Date Performed	12/30/14	Jurisdiction Analysis Year	Latah Co, ID 2029		
	Input Data	Base Conditions	Site Conditions		
Roadway type (divided / undivided)		Undivided	Divided		
Length of segment, L (mi)			6.35		
AADT (veh/day)	AADT _{MAX} = 89,300 (veh/day)	-	7,185		
Lane width (ft)	••••••••	12	12		
Shoulder width (ft) - right shoulder width for divi	ided [if differ for directions of travel, use average width]	8	8		
Shoulder type - right shoulder type for divided		Paved	Paved		
Median width (ft) - for divided only		30	40		
Side Slopes - for undivided only		1:7 or flatter	Not Applicable		
Lighting (present/not present)		Not Present	Not Present		
Auto speed enforcement (present/not present)		Not Present	Not Present		
Calibration Factor, Cr		1.00	1.00		

Worksheet 1B (a) – Crash Modification Factors for Rural Multilane Divided Roadway Segments					
(1)	(2)	(3)	(4)	(5)	(6)
CMF for Lane Width	CMF for Right Shoulder Width	CMF for Median Width	CMF for Lighting	CMF for Automated Speed Enforcement	Combined CMF
CMF 1rd	CMF 2rd	CMF 3rd	CMF 4rd	CMF 5rd	CMF comb
from Equation 11-16	from Table 11-17	from Table 11-18	from Equation 11-17	from Section 11.7.2	(1)*(2)*(3)*(4)*(5)
1.00	1.00	0.99	1.00	1.00	0.99

(1)	(2)		(3)	(4)	(5)	(6)	(7)	
Crash Severity Level	S	PF Coefficien	ts	N spf rd	Overdispersion	Combined CMFs	Calibration	Predicted average crash
	f	rom Table 11-	5		Parameter, k	(6) from Worksheet	Factor, Cr	frequency, N predicted ra(d)
	а	b	С	from Equation 11-9	from Equation 11-10	1B (a)		(3)*(5)*(6)
Total	-9.025	1.049	1.549	8.485	0.033	0.99	1.00	8.400
Fatal and Injury (FI)	-8.837	0.958	1.687	4.564	0.029	0.99	1.00	4.519
Fatal and Injury ^a (FI ^a)	-8.505	0.874	1.740	3.017	0.028	0.99	1.00	2.987
Property Damage Only (PDO)					-		·	(7) _{TOTAL} - (7) _{FI}
roperty barnage only (1 bo)								3.882

	Workshee	et 1D (a) Crashes by Seve	erity Level and	Collision Type for Ru	ral Multilane	Divided Roadway Segn	nents	
(1)	(2)	(3)	(4)	(5)	(6)	(7)	(8)	(9)
Collision Type	Proportion of Collision Type(TOTAL)	N predicted rs(d) (TOTAL) (crashes/year)	Proportion of Collision Type(FI)	N predicted rs(d) (FI) (crashes/year)	Proportion of Collision	N predicted rs (FI ^a) (crashes/year)	Proportion of Collision Type (PDO)	N predicted rs(d) (PDO) (crashes/year)
	from Table 11-6	(7)TOTAL from Worksheet 1C (a)	from Table 11- 6	(7)⊧i from Worksheet 1C (a)	from Table 11-6	(7) _{Fl} ^a from Worksheet 1C (a)	from Table 11-6	(7)PDO from Worksheet 1C (a)
Total	1.000	8.400	1.000	4.519	1.000	2.987	1.000	3.882
		(2)*(3) _{TOTAL}		(4)x(5) _{F1}		(6)*(7) _{F1} ^a		(8)*(9) PDO
Head-on collision	0.006	0.050	0.013	0.059	0.018	0.054	0.002	0.008
Sideswipe collision	0.043	0.361	0.027	0.122	0.022	0.066	0.053	0.206
Rear-end collision	0.116	0.974	0.163	0.737	0.114	0.341	0.088	0.342
Angle collision	0.043	0.361	0.048	0.217	0.045	0.134	0.041	0.159
Single-vehicle collision	0.768	6.451	0.727	3.285	0.778	2.324	0.792	3.074
Other collision	0.024	0.202	0.022	0.099	0.023	0.069	0.024	0.093

NOTE: * Using the KABCO scale, these include only KAB crashes. Crashes with severity level C (possible injury) are not included.

Worksheet 1E – Summary Results for Rural Multilane Roadway Segments					
(1)	(2)	(3)	(4)		
Crash severity level	Predicted average crash frequency (crashes/year)	Roadway segment length (mi)	Crash rate (crashes/mi/year)		
	(7) from Worksheet 1C (a) or (b)	7	(2)/(3)		
Total	8.400	6.4	1.3		
Fatal and Injury (FI)	4.519	6.4	0.7		
Fatal and Injury ^a (FI ^a)	2.987	6.4	0.5		
Property Damage Only (PDO)	3.882	6.4	0.6		

Proposed Modified W4 Alternative				
Segment	Total Crashes	Fatal and Injury Crashes		
Rural Divided	8.544	4.589		
Suburban	1.382 0.41			
Intersection	Total Crashes	Fatal and Injury Crashes		
Eid Rd.	0.439	0.187		
Jacksha Rd.	0.412	0.175		
Old US-95 South 0.275		0.088		
Old US-95 North	0.466	0.177		
Subtotal	11.518	5.630		

Proposed Modified W4 Alternative				
Total (Crashes/year)	11.5			
Fatal and Injury	5.6			
Property Damage Only	5.9			

Existing US-95			
Segment	Total Crashes	Fatal and Injury Crashes	
11	0.007	0.002	
12	0.021	0.007	
13	0.009	0.003	
14	0.021	0.007	
15	0.012	0.004	
16	0.026	0.008	
17	0.016	0.005	
18	0.052	0.017	
19	0.015	0.005	
20	0.099	0.032	
21	21 0.278		
22	0.531	0.171	
Intersection	Total Crashes	Fatal and Injury Crashes	
Zeitler Rd.	0.027	0.011	
Snow Rd.	0.017	0.007	
Skyview Dr.	0.009	0.004	
Clyde Rd. South	0.024	0.010	
Cameron Rd.	0.085	0.035	
Clyde Rd. North	0.125	0.052	
Subtotal	1.376	0.469	

Existing US-95 Loop					
Total (Crashes/year)	1.4				
Fatal and Injury	0.5				
Property Damage Only	0.9				

Proposed Modified W4 Alternative and Existing US-95 Loop				
Total (Crashes/year)	12.9			
Fatal and Injury	6.1			
Property Damage Only	6.8			

	Worksheet 1A General Information and Input Da	ta for Rural Multilane Ro	adway Segments	
(General Information	Location Information		
Analyst Agency or Company Date Performed	CJA, KJB ITD District 2 12/30/14	Roadway Roadway Section Jurisdiction Analysis Year	US-95, Thorncreek to Moscow Modified W4 Rural - Divided Latah Co, ID 2030	
	Input Data	Base Conditions	Site Conditions	
Roadway type (divided / undivided)		Undivided	Divided	
Length of segment, L (mi)			6.35	
AADT (veh/day)	AADT _{MAX} = 89,300 (veh/day)	-	7,302	
Lane width (ft)		12	12	
Shoulder width (ft) - right shoulder width fo	r divided [if differ for directions of travel, use average width]	8	8	
Shoulder type - right shoulder type for divis	ded	Paved	Paved	
Median width (ft) - for divided only		30	40	
Side Slopes - for undivided only		1:7 or flatter	Not Applicable	
Lighting (present/not present)		Not Present	Not Present	
Auto speed enforcement (present/not pres	ent)	Not Present	Not Present	
Calibration Factor, Cr		1.00	1.00	

Worksheet 1B (a) Crash Modification Factors for Rural Multilane Divided Roadway Segments					
(1)	(2)	(3)	(4)	(5)	(6)
CMF for Lane Width	CMF for Right Shoulder Width	CMF for Median Width	CMF for Lighting	CMF for Automated Speed Enforcement	Combined CMF
CMF 1rd	CMF 2rd	CMF 3rd	CMF 4rd	CMF 5rd	CMF comb
from Equation 11-16	from Table 11-17	from Table 11-18	from Equation 11-17	from Section 11.7.2	(1)*(2)*(3)*(4)*(5
1.00	1.00	0.99	1.00	1.00	0.99

(1)	(2)		(3)	(4)	(5)	(6)	(7)	
Crash Severity Level			N spf rd	Overdispersion	Combined CMFs	Calibration	Predicted average crash	
	a	rom Table 11-5	c	from Equation 11-9	from Equation 11-10	(6) from Worksheet 1B (a)	Factor, Cr	(3)*(5)*(6)
Total	-9.025	1.049	1.549	8.630	0.033	0.99	1.00	8.544
Fatal and Injury (FI)	-8.837	0.958	1.687	4.635	0.029	0.99	1.00	4.589
Fatal and Injury ^a (FI ^a)	-8.505	0.874	1.740	3.060	0.028	0.99	1.00	3.030
Property Damage Only (PDO)		-			-	-		(7) _{TOTAL} - (7) _{F1} 3.955

	Workshe	et 1D (a) Crashes by Seve	erity Level and	Collision Type for Ru	irai Multilane	Divided Roadway Segr	nents	
(1)	(2)	(3)	(4)	(5)	(6)	(7)	(8)	(9)
Collision Type	Proportion of Collision Type(TOTAL)	N predicted rs(d) (TOTAL) (crashes/year)	Proportion of Collision Type(FI)	N predicted rs(d) (FI) (crashes/year)	Proportion of Collision Type (Fl ^a)	N predicted rs (FI ^a) (crashes/year)	Proportion of Collision Type (PDO)	N predicted rs(d) (PDO) (crashes/year)
	from Table	(7)TOTAL from Worksheet 1C	from Table 11-	(7)FI from Worksheet	from Table	(7) FI ^a from Worksheet	from Table	(7)PDO from Worksheet 1C
	11-6	(a)	6	1C (a)	11-6	1C (a)	11-6	(a)
Total	1.000	8.544	1.000	4.589	1.000	3.030	1.000	3.955
		(2)*(3) _{TOTAL}		(4)x(5) _{F1}		(6)*(7) _{F1} ^a		(8)*(9) PDO
Head-on collision	0.006	0.051	0.013	0.060	0.018	0.055	0.002	0.008
Sideswipe collision	0.043	0.367	0.027	0.124	0.022	0.067	0.053	0.210
Rear-end collision	0.116	0.991	0.163	0.748	0.114	0.345	0.088	0.348
Angle collision	0.043	0.367	0.048	0.220	0.045	0.136	0.041	0.162
Single-vehicle collision	0.768	6.561	0.727	3.336	0.778	2.357	0.792	3.132
Other collision	0.024	0.205	0.022	0.101	0.023	0.070	0.024	0.095

NOTE: * Using the KABCO scale, these include only KAB crashes. Crashes with severity level C (possible injury) are not included.

Worksheet 1E Summary Results for Rural Multilane Roadway Segments					
(1)	(2)	(3)	(4)		
Crash severity level	Predicted average crash frequency (crashes/year)	Roadway segment length (mi)	Crash rate (crashes/mi/year)		
	(7) from Worksheet 1C (a) or (b)	7	(2)/(3)		
Total	8.544	6.4	1.3		
Fatal and Injury (FI)	4.589	6.4	0.7		
Fatal and Injury ^a (FI ^a)	3.030	6.4	0.5		
Property Damage Only (PDO)	3.955	6.4	0.6		

Proposed Modified W4 Alternative				
Segment	Total Crashes	Fatal and Injury Crashes		
Rural Divided	8.689	4.660		
Suburban	1.404	0.420		
Intersection	Total Crashes	Fatal and Injury Crashes		
Eid Rd.	0.447	0.191		
Jacksha Rd.	0.420	0.178		
Old US-95 South	0.281	0.090		
Old US-95 North	0.475	0.180		
Subtotal	11.717	5.719		

Proposed Modified W4 Alternative				
Total (Crashes/year)	11.7			
Fatal and Injury	5.7			
Property Damage Only	6.0			

Existing US-95				
Segment	Total Crashes	Fatal and Injury Crashes		
11	0.007	0.002		
12	0.021	0.007		
13	0.009	0.003		
14	0.021	0.007		
15	0.012	0.004		
16	0.026	0.008		
17	0.016	0.005		
18	0.053	0.017		
19	0.015	0.005		
20	0.099	0.032		
21	0.279	0.090		
22	0.536	0.172		
Intersection	Total Crashes	Fatal and Injury Crashes		
Zeitler Rd.	0.027	0.011		
Snow Rd.	0.017	0.007		
Skyview Dr.	0.009	0.004		
Clyde Rd. South	0.024	0.010		
Cameron Rd.	0.085	0.035		
Clyde Rd. North	0.126	0.052		
Subtotal	1.384	0.471		

Existing US-95 Loop				
Total (Crashes/year)	1.4			
Fatal and Injury	0.5			
Property Damage Only	0.9			

Proposed Modified W4 Alternative and Existing US-95 Loop					
Total (Crashes/year)	13.1				
Fatal and Injury	6.2				
Property Damage Only	6.9				

	Worksheet 1A General Information and Input Da	ta for Rural Multilane Road	way Segments	
G	eneral Information	Location Information		
Analyst Agency or Company	CJA, KJB ITD District 2	Roadway Roadway Section	US-95, Thorncreek to Moscow Modified W4 Rural - Divided	
Date Performed	formed 12/30/14 Jurisdiction Analysis Year		Latah Co, ID 2031	
	Input Data	Base Conditions	Site Conditions	
Roadway type (divided / undivided)		Undivided	Divided	
Length of segment, L (mi)			6.35	
AADT (veh/day)	AADT _{MAX} = 89,300 (veh/day)	-	7,421	
Lane width (ft)		12	12	
Shoulder width (ft) - right shoulder width for	divided [if differ for directions of travel, use average width]	8	8	
Shoulder type - right shoulder type for divid	ed	Paved	Paved	
Median width (ft) - for divided only		30	40	
Side Slopes - for undivided only		1:7 or flatter	Not Applicable	
Lighting (present/not present)		Not Present	Not Present	
Auto speed enforcement (present/not prese	ent)	Not Present	Not Present	
Calibration Factor, Cr		1.00	1.00	

Worksheet 1B (a) Crash Modification Factors for Rural Multilane Divided Roadway Segments						
(1)	(2)	(3)	(4)	(5)	(6)	
CMF for Lane Width	CMF for Right Shoulder Width	CMF for Median Width	CMF for Lighting	CMF for Automated Speed Enforcement	Combined CMF	
CMF 1rd	CMF 2rd	CMF 3rd	CMF 4rd	CMF 5rd	CMF comb	
from Equation 11-16	from Table 11-17	from Table 11-18	from Equation 11-17	from Section 11.7.2	(1)*(2)*(3)*(4)*(5)	
1.00	1.00	0.99	1.00	1.00	0.99	

(1)		(2)		(3)	(4)	(5)	(6)	(7)
Crash Severity Level	SPF Coefficients from Table 11-5		N spf rd	Overdispersion Parameter, k	Combined CMFs	Calibration	Predicted average crash	
					(6) from Worksheet	Factor, Cr	frequency, N predicted raid	
	а	b	С	from Equation 11-9	from Equation 11-10	1B (a)		(3)*(5)*(6)
Total	-9.025	1.049	1.549	8.777	0.033	0.99	1.00	8.689
Fatal and Injury (FI)	-8.837	0.958	1.687	4.708	0.029	0.99	1.00	4.660
Fatal and Injury ^a (FI ^a)	-8.505	0.874	1.740	3.104	0.028	0.99	1.00	3.073
Property Damage Only (PDO)		-	-		-			(7) _{TOTAL} - (7) _{FI} 4.029

(1)	(2)	(3)	(4)	(5)	(6)	(7)	(8)	(9)
of C Typ from	Proportion of Collision Type(TOTAL)		Proportion of Collision Type(FI)	N predicted rs(d) (FI) (crashes/year)	Proportion of Collision Type (FI*)	N predicted rs (FI ^a) (crashes/year)	Proportion of Collision Type (PDO)	N predicted rs(d) (PDO) (crashes/year)
	from Table	(7)TOTAL from Worksheet 1C	from Table 11-	(7)FI from Worksheet	from Table	(7) Fl ^a from Worksheet	from Table	(7)PDO from Worksheet 1C
	11-6	(a)	6	1C (a)	11-6	1C (a)	11-6	(a)
Total	1.000	8.689	1.000	4.660	1.000	3.073	1.000	4.029
		(2)*(3) _{TOTAL}		(4)x(5) _{F1}		(6)*(7) _{F1} ^a		(8)*(9) PDO
Head-on collision	0.006	0.052	0.013	0.061	0.018	0.055	0.002	0.008
Sideswipe collision	0.043	0.374	0.027	0.126	0.022	0.068	0.053	0.214
Rear-end collision	0.116	1.008	0.163	0.760	0.114	0.350	0.088	0.355
Angle collision	0.043	0.374	0.048	0.224	0.045	0.138	0.041	0.165
Single-vehicle collision	0.768	6.673	0.727	3.388	0.778	2.390	0.792	3.191
Other collision	0.024	0.209	0.022	0.103	0.023	0.071	0.024	0.097

NOTE: * Using the KABCO scale, these include only KAB crashes. Crashes with severity level C (possible injury) are not included.

	Worksheet 1E Summary Results for Rural Multila	ne Roadway Segments		
(1)	(2)	(3)	(4)	
rash severity level	Predicted average crash frequency (crashes/year)	Roadway segment length (mi)	Crash rate (crashes/mi/year)	
	(7) from Worksheet 1C (a) or (b)		(2)/(3)	
Total	8.689	6.4	1.4	
Fatal and Injury (FI)	4.660	6.4	0.7	
Fatal and Injury ^a (FI ^a)	3.073	6.4	0.5	
Property Damage Only (PDO)	4.029	6.4	0.6	

	Proposed Modified W4 Alternati	ve
Segment	Total Crashes	Fatal and Injury Crashes
Rural Divided	8.838	4.733
Suburban	1.427	0.427
Intersection	Total Crashes	Fatal and Injury Crashes
Eid Rd.	0.456	0.194
Jacksha Rd.	0.429	0.181
Old US-95 South	0.287	0.091
Old US-95 North	0.484	0.183
Subtotal	11.920	5.810

Proposed Modified W4 Alternative				
Total (Crashes/year)	11.9			
Fatal and Injury	5.8			
Property Damage Only	6.1			

Existing US-95				
Segment	Total Crashes	Fatal and Injury Crashes		
11	0.007	0.002		
12	0.021	0.007		
13	0.009	0.003		
14	0.021	0.007		
15	0.012	0.004		
16	0.026	0.008		
17	0.016	0.005		
18	0.053	0.017		
19	0.015	0.005		
20	0.100	0.032		
21	0.281	0.090		
22	0.542	0.174		
Intersection	Total Crashes	Fatal and Injury Crashes		
Zeitler Rd.	0.027	0.011		
Snow Rd.	0.017	0.007		
Skyview Dr.	0.009	0.004		
Clyde Rd. South	0.024	0.010		
Cameron Rd.	0.086	0.036		
Clyde Rd. North	0.127	0.053		
Subtotal	1.393	0.474		

Existing US-95 Loop				
Total (Crashes/year)	1.4			
Fatal and Injury	0.5			
Property Damage Only	0.9			

Proposed Modified W4 Alternative and Existing US-95 Loop				
Total (Crashes/year)	13.3			
Fatal and Injury	6.3			
Property Damage Only	7.0			

	Worksheet 1A General Information and Input Da	ta for Rural Multilane Road	Iway Segments		
Gener	al Information	Location Information			
Analyst Agency or Company	CJA, KJB ITD District 2	Roadway Roadway Section	US-95, Thorncreek to Moscow Modified W4 Rural - Divided		
Date Performed	12/30/14	Jurisdiction Analysis Year	Latah Co, ID 2032		
In	put Data	Base Conditions	Site Conditions		
Roadway type (divided / undivided)		Undivided	Divided		
Length of segment, L (mi)			6.35		
AADT (veh/day)	AADT _{MAX} = 89,300 (veh/day)	-	7,541		
Lane width (ft)		12	12		
Shoulder width (ft) - right shoulder width for divid	ed [if differ for directions of travel, use average width]	8	8		
Shoulder type - right shoulder type for divided		Paved	Paved		
Median width (ft) - for divided only		30	40		
Side Slopes - for undivided only		1:7 or flatter	Not Applicable		
Lighting (present/not present)		Not Present	Not Present		
Auto speed enforcement (present/not present)		Not Present	Not Present		
Calibration Factor, Cr		1.00	1.00		

Worksheet 1B (a) Crash Modification Factors for Rural Multilane Divided Roadway Segments						
(1)	(2)	(3)	(4)	(5)	(6)	
CMF for Lane Width	CMF for Right Shoulder Width	CMF for Median Width	CMF for Lighting	CMF for Automated Speed Enforcement	Combined CMF	
CMF 1rd	CMF 2rd	CMF 3rd	CMF 4rd	CMF 5rd	CMF comb	
from Equation 11-16	from Table 11-17	from Table 11-18	from Equation 11-17	from Section 11.7.2	(1)*(2)*(3)*(4)*(5)	
1.00	1.00	0.99	1.00	1.00	0.99	

(1)		(2)		(3)	(4)	(5)	(6)	(7)
Crash Severity Level	SPF Coefficients from Table 11-5		N spf rd	Overdispersion Parameter, k	Combined CMFs (6) from Worksheet	Calibration Factor, Cr	Predicted average crash frequency, N predicted raid	
	а	b	с	from Equation 11-9	from Equation 11-10	1B (a)	10	(3)*(5)*(6)
Total	-9.025	1.049	1.549	8.927	0.033	0.99	1.00	8.838
Fatal and Injury (FI)	-8.837	0.958	1.687	4.781	0.029	0.99	1.00	4.733
Fatal and Injury ^a (FI ^a)	-8.505	0.874	1.740	3.148	0.028	0.99	1.00	3.116
Property Damage Only (PDO)	-	-	-		-	-		(7) _{TOTAL} - (7) _{FI} 4.105

	Workshee	et 1D (a) Crashes by Seve	erity Level and	Collision Type for Ru	ral Multilane	Divided Roadway Segr	nents	
(1)	(2)	(3)	(4)	(5)	(6)	(7)	(8)	(9)
Collision Type	Proportion of Collision Type(TOTAL)	N predicted rs(d) (TOTAL) (crashes/year)	Proportion of Collision Type(FI)	N predicted ra(d) (Fi) (crashes/year)	Proportion of Collision Type (El ^a)	N predicted rs (FI ^a) (crashes/year)	Proportion of Collision Type (PDO)	N predicted rs(d) (PDO) (crashes/year)
	from Table 11-6	(7)TOTAL from Worksheet 1C (a)	from Table 11- 6	(7)FI from Worksheet 1C (a)	from Table 11-6	(7) _{Fl} ^a from Worksheet 1C (a)	from Table 11-6	(7)PDO from Worksheet 1C (a)
Total	1.000	8.838	1.000	4.733	1.000	3.116	1.000	4.105
		(2)*(3) _{TOTAL}		(4)x(5) _{F1}		(6)*(7) _{Fi} ^a		(8)*(9) PDO
Head-on collision	0.006	0.053	0.013	0.062	0.018	0.056	0.002	0.008
Sideswipe collision	0.043	0.380	0.027	0.128	0.022	0.069	0.053	0.218
Rear-end collision	0.116	1.025	0.163	0.771	0.114	0.355	0.088	0.361
Angle collision	0.043	0.380	0.048	0.227	0.045	0.140	0.041	0.168
Single-vehicle collision	0.768	6.787	0.727	3.441	0.778	2.424	0.792	3.251
Other collision	0.024	0.212	0.022	0.104	0.023	0.072	0.024	0.099

NOTE: * Using the KABCO scale, these include only KAB crashes. Crashes with severity level C (possible injury) are not included.

Worksheet 1E Summary Results for Rural Multilane Roadway Segments						
(1)	(2)	(3)	(4)			
Crash severity level	Predicted average crash frequency (crashes/year)	Roadway segment length (mi)	Crash rate (crashes/mi/year)			
5	(7) from Worksheet 1C (a) or (b)	7 · · · · · · · · · · · · · · ·	(2)/(3)			
Total	8.838	6.4	1.4			
Fatal and Injury (FI)	4.733	6.4	0.7			
Fatal and Injury ^a (FI ^a)	3.116	6.4	0.5			
Property Damage Only (PDO)	4.105	6.4	0.6			

	Proposed Modified W4 Alternati	ve
Segment	Total Crashes	Fatal and Injury Crashes
Rural Divided	8.989	4.807
Suburban	1.450	0.433
Intersection	Total Crashes	Fatal and Injury Crashes
Eid Rd.	0.465	0.198
Jacksha Rd.	0.438	0.184
Old US-95 South	0.292	0.093
Old US-95 North	0.493	0.186
Subtotal	12.126	5.902

Proposed Modified W4 Alternative					
Total (Crashes/year)	12.1				
Fatal and Injury	5.9				
Property Damage Only	6.2				

Existing US-95					
Segment	Total Crashes	Fatal and Injury Crashes			
11	0.007	0.002			
12	0.021	0.007			
13	0.009	0.003			
14	0.021	0.007			
15	0.012	0.004			
16	0.026	0.008			
17	0.016	0.005			
18	0.053	0.017			
19	0.015	0.005			
20	0.100	0.032			
21	0.282	0.091			
22	0.547	0.176			
Intersection	Total Crashes	Fatal and Injury Crashes			
Zeitler Rd.	0.027	0.011			
Snow Rd.	0.017	0.007			
Skyview Dr.	0.009	0.004			
Clyde Rd. South	0.024	0.010			
Cameron Rd.	0.086	0.036			
Clyde Rd. North	0.127	0.053			
Subtotal	1.402	0.477			

Existing US-95 Loop					
Total (Crashes/year)	1.4				
Fatal and Injury	0.5				
Property Damage Only	0.9				

Proposed Modified W4 Alternative and Existing US-95 Loop				
Total (Crashes/year)	13.5			
Fatal and Injury	6.4			
Property Damage Only	7.1			

	Worksheet 1A General Information and Input Da	ta for Rural Multilane Road	tway Segments		
Gener	al Information	Location Information			
Analyst Agency or Company	CJA, KJB ITD District 2	Roadway Roadway Section	US-95, Thorncreek to Moscow Modified W4 Rural - Divided		
Date Performed	12/30/14 Jurisdiction Analysis Year		Latah Co, ID 2033		
Ir	nput Data	Base Conditions	Site Conditions		
Roadway type (divided / undivided)		Undivided	Divided		
Length of segment, L (mi)			6.35		
AADT (veh/day)	AADT _{MAX} = 89,300 (veh/day)	-	7,664		
Lane width (ft)		12	12		
Shoulder width (ft) - right shoulder width for divid	led [if differ for directions of travel, use average width]	8	8		
Shoulder type - right shoulder type for divided		Paved	Paved		
Median width (ft) - for divided only		30	40		
Side Slopes - for undivided only		1:7 or flatter	Not Applicable		
Lighting (present/not present)		Not Present	Not Present		
Auto speed enforcement (present/not present)		Not Present	Not Present		
Calibration Factor, Cr		1.00	1.00		

Worksheet 1B (a) Crash Modification Factors for Rural Multilane Divided Roadway Segments							
(1)	(2)	(3)	(4)	(5)	(6)		
CMF for Lane Width	CMF for Right Shoulder Width	CMF for Median Width	CMF for Lighting	CMF for Automated Speed Enforcement	Combined CMF		
CMF 1rd	CMF 2rd	CMF 3rd	CMF 4rd	CMF 5rd	CMF comb		
from Equation 11-16	from Table 11-17	from Table 11-18	from Equation 11-17	from Section 11.7.2	(1)*(2)*(3)*(4)*(5)		
1.00	1.00	0.99	1.00	1.00	0.99		

(1)		(2)		(3)	(4)	(5)	(6)	(7)
Crash Severity Level	S	PF Coefficien	ts	N spf rd	Overdispersion	Combined CMFs	Calibration	Predicted average crash
	1	from Table 11-5			Parameter, k	(6) from Worksheet	Factor, Cr	frequency, N predicted rate
	а	b	С	from Equation 11-9	from Equation 11-10	1B (a)		(3)*(5)*(6)
Total	-9.025	1.049	1.549	9.079	0.033	0.99	1.00	8.989
Fatal and Injury (FI)	-8.837	0.958	1.687	4.855	0.029	0.99	1.00	4.807
Fatal and Injury ^a (FI ^a)	-8.505	0.874	1.740	3.192	0.028	0.99	1.00	3.160
Property Damage Only (PDO)		-		_	-		-	(7) _{TOTAL} - (7) _{FI}
roporty barnage only (1 bo)							_	4.182

(1)	(2)	(3)	(4)	(5)	(6)	(7)	(8)	(9)
Collision Type	Proportion of Collision Type(TOTAL)	N predicted rs(d) (TOTAL) (crashes/year)	Proportion of Collision Type(FI)	N predicted rs(d) (Fi) (crashes/year)	Proportion of Collision Type (FI ^a)	N predicted rs (FI ^a) (crashes/year)	Proportion of Collision Type (PDO)	N predicted rs(d) (PDO) (crashes/year)
	from Table	(7)TOTAL from Worksheet 1C	from Table 11-	(7)FI from Worksheet	from Table	(7) FI ^a from Worksheet	from Table	(7)PDO from Worksheet 1C
	11-6	(a)	6	1C (a)	11-6	1C (a)	11-6	(a)
Total	1.000	8.989	1.000	4.807	1.000	3.160	1.000	4.182
		(2)*(3) _{TOTAL}		(4)x(5) _{FI}		(6)*(7) _{F1} ^a		(8)*(9) PDO
Head-on collision	0.006	0.054	0.013	0.062	0.018	0.057	0.002	0.008
Sideswipe collision	0.043	0.387	0.027	0.130	0.022	0.070	0.053	0.222
Rear-end collision	0.116	1.043	0.163	0.784	0.114	0.360	0.088	0.368
Angle collision	0.043	0.387	0.048	0.231	0.045	0.142	0.041	0.171
Single-vehicle collision	0.768	6.903	0.727	3.495	0.778	2.459	0.792	3.312
Other collision	0.024	0.216	0.022	0.106	0.023	0.073	0.024	0.100

NOTE: * Using the KABCO scale, these include only KAB crashes. Crashes with severity level C (possible injury) are not included.

Worksheet 1E Summary Results for Rural Multilane Roadway Segments						
(1)	(2)	(3)	(4) Crash rate (crashes/mi/year)			
Crash severity level	Predicted average crash frequency (crashes/year)	Roadway segment length (mi)				
	(7) from Worksheet 1C (a) or (b)		(2)/(3)			
Total	8.989	6.4	1.4			
Fatal and Injury (FI)	4.807	6.4	0.8			
Fatal and Injury ^a (FI ^a)	3.160	6.4	0.5			
Property Damage Only (PDO)	4.182	6.4	0.7			

Proposed Modified W4 Alternative				
Segment	Total Crashes	Fatal and Injury Crashes		
Rural Divided	9.142	4.882		
Suburban	1.473	0.440		
Intersection	Total Crashes	Fatal and Injury Crashes		
Eid Rd.	0.475	0.202		
Jacksha Rd.	0.446	0.188		
Old US-95 South	JS-95 South 0.298			
Old US-95 North	0.502	0.189		
Subtotal	12.337	5.996		

Proposed Modified W4 Alternative				
Total (Crashes/year)	12.3			
Fatal and Injury	6.0			
Property Damage Only	6.3			

Existing US-95			
Segment	Total Crashes	Fatal and Injury Crashes	
11	0.007	0.002	
12	0.021	0.007	
13	0.009	0.003	
14	0.021	0.007	
15	0.012	0.004	
16	0.026	0.008	
17	0.016	0.005	
18	0.053	0.017	
19	0.015	0.005	
20	0.101	0.032	
21	0.284	0.091	
22	0.552	0.177	
Intersection	Total Crashes	Fatal and Injury Crashes	
Zeitler Rd.	0.027	0.011	
Snow Rd.	0.017	0.007	
Skyview Dr.	0.009	0.004	
Clyde Rd. South	0.024	0.010	
Cameron Rd.	0.087	0.036	
Clyde Rd. North	0.128	0.053	
Subtotal	1.411	0.480	

Existing US-95 Loop				
Total (Crashes/year)	1.4			
Fatal and Injury	0.5			
Property Damage Only	0.9			

Proposed Modified W4 Alternative and Existing US-95 Loop				
Total (Crashes/year)	13.7			
Fatal and Injury	6.5			
Property Damage Only	7.3			

	Worksheet 1A General Information and Input Da	ta for Rural Multilane Roa	dway Segments	
	eneral Information		Location Information	
Analyst Agency or Company	CJA, KJB ITD District 2	Roadway Roadway Section	US-95, Thorncreek to Moscow Modified W4 Rural - Divided	
Date Performed	12/30/14	Jurisdiction Analysis Year	Latah Co, ID 2034	
	Input Data	Base Conditions	Site Conditions	
Roadway type (divided / undivided)		Undivided	Divided	
Length of segment, L (mi)			6.35	
AADT (veh/day)	AADT _{MAX} = 89,300 (veh/day)	-	7,789	
Lane width (ft)		12	12	
Shoulder width (ft) - right shoulder width for	divided [if differ for directions of travel, use average width]	8	8	
Shoulder type - right shoulder type for divide	ed	Paved	Paved	
Median width (ft) - for divided only		30	40	
Side Slopes - for undivided only		1:7 or flatter	Not Applicable	
Lighting (present/not present)		Not Present	Not Present	
Auto speed enforcement (present/not prese	ent)	Not Present	Not Present	
Calibration Factor, Cr		1.00	1.00	

Worksheet 1B (a) Crash Modification Factors for Rural Multilane Divided Roadway Segments						
(1)	(2)	(3)	(4)	(5)	(6)	
CMF for Lane Width	CMF for Right Shoulder Width	CMF for Median Width	CMF for Lighting	CMF for Automated Speed Enforcement	Combined CMF	
CMF 1rd	CMF 2rd	CMF 3rd	CMF 4rd	CMF 5rd	CMF comb	
from Equation 11-16	from Table 11-17	from Table 11-18	from Equation 11-17	from Section 11.7.2	(1)*(2)*(3)*(4)*(5	
1.00	1.00	0.99	1.00	1.00	0.99	

(1)	(2)		(3)	(4)	(5)	(6)	(7)	
Crash Severity Level	S	PF Coefficient	ts	N spf rd	Overdispersion	Combined CMFs	Calibration	Predicted average crash
	f	rom Table 11-	5		Parameter, k	(6) from Worksheet	Factor, Cr	frequency, N predicted ra(d)
	а	b	С	from Equation 11-9	from Equation 11-10	1B (a)		(3)*(5)*(6)
Total	-9.025	1.049	1.549	9.234	0.033	0.99	1.00	9.142
Fatal and Injury (FI)	-8.837	0.958	1.687	4.931	0.029	0.99	1.00	4.882
Fatal and Injury ^a (FI ^a)	-8.505	0.874	1.740	3.238	0.028	0.99	1.00	3.205
Property Damage Only (PDO)	-		-	-	_			(7) _{TOTAL} - (7) _{F1} 4.260

(1)	(2)	(3)	(4)	(5)	(6)	(7)	(8)	(9)
Collision Type	Proportion of Collision Type(TOTAL)	N predicted rs(d) (TOTAL) (crashes/year)	Proportion of Collision Type(FI)	N predicted rs(d) (Fi) (crashes/year)	Proportion of Collision Type (Fl ^a)	N predicted rs (FI ^a) (crashes/year)	Proportion of Collision Type (PDO)	N predicted rs(d) (PDO) (crashes/year)
	from Table	(7)TOTAL from Worksheet 1C	from Table 11-	(7)FI from Worksheet	from Table	(7) _{Fl} ^a from Worksheet	from Table	(7)PDO from Worksheet 1C
	11-6	(a)	6	1C (a)	11-6	1C (a)	11-6	(a)
Total	1.000	9.142	1.000	4.882	1.000	3.205	1.000	4.260
		(2)*(3) _{TOTAL}		(4)x(5) _{F1}		(6)*(7) _{FI} ^a		(8)*(9) PDO
Head-on collision	0.006	0.055	0.013	0.063	0.018	0.058	0.002	0.009
Sideswipe collision	0.043	0.393	0.027	0.132	0.022	0.071	0.053	0.226
Rear-end collision	0.116	1.060	0.163	0.796	0.114	0.365	0.088	0.375
Angle collision	0.043	0.393	0.048	0.234	0.045	0.144	0.041	0.175
Single-vehicle collision	0.768	7.021	0.727	3.549	0.778	2.494	0.792	3.374
Other collision	0.024	0.219	0.022	0.107	0.023	0.074	0.024	0.102

NOTE: * Using the KABCO scale, these include only KAB crashes. Crashes with severity level C (possible injury) are not included.

Worksheet 1E – Summary Results for Rural Multilane Roadway Segments					
(1)	(2)	(3)	(4)		
Crash severity level	Predicted average crash frequency (crashes/year)	Roadway segment length (mi)	Crash rate (crashes/mi/year) (2)/(3)		
	(7) from Worksheet 1C (a) or (b)				
lotal	9.142	6.4	1.4		
Fatal and Injury (FI)	4.882	6.4	0.8		
Fatal and Injury ^a (FI ^a)	3.205	6.4	0.5		
Property Damage Only (PDO)	4.260	6.4	0.7		

Commont	Proposed Modified W4 Alternati				
Segment	Total Crashes	Fatal and Injury Crashes			
Rural Divided	9.298	4.958			
Suburban	1.497	0.447			
Intersection	Total Crashes	Fatal and Injury Crashes			
Eid Rd.	0.484	0.206			
Jacksha Rd.	0.455	0.191			
Old US-95 South	5 South 0.304				
Old US-95 North	0.511 0.1		0.511	0.511	0.193
Subtotal	12.550	6.091			

Proposed Modified W4 Alternative				
Total (Crashes/year)	12.6			
Fatal and Injury	6.1			
Property Damage Only	6.5			

Existing US-95			
Segment	Total Crashes	Fatal and Injury Crashes	
11	0.007	0.002	
12	0.021	0.007	
13	0.009	0.003	
14	0.021	0.007	
15	0.012	0.004	
16	0.026	0.008	
17	0.016	0.005	
18	0.053	0.017	
19	0.015	0.005	
20	0.101	0.033	
21	0.285	0.092	
22	0.557	0.179	
Intersection	Total Crashes	Fatal and Injury Crashes	
Zeitler Rd.	0.028	0.011	
Snow Rd.	0.017	0.007	
Skyview Dr.	0.009	0.004	
Clyde Rd. South	0.024	0.010	
Cameron Rd.	0.087	0.036	
Clyde Rd. North	0.129	0.053	
Subtotal	1.420	0.483	

Existing US-95 Loop				
Total (Crashes/year)	1.4			
Fatal and Injury	0.5			
Property Damage Only	0.9			

Proposed Modified W4 Alternative and Existing US-95 Loop				
Total (Crashes/year)	14.0			
Fatal and Injury	6.6			
Property Damage Only	7.4			

	Worksheet 1A – General Information and Input Da	ta for Rural Multilane Road	dway Segments		
Gen	eral Information		Location Information		
Analyst Agency or Company Date Performed	12/30/14 Jurisdiction		US-95, Thorncreek to Moscow Modified W4 Rural - Divided Latah Co, ID		
	Input Data	Analysis Year Base Conditions	2035 Site Conditions		
Roadway type (divided / undivided)		Undivided	Divided		
Length of segment, L (mi)			6.35		
AADT (veh/day)	AADT _{MAX} = 89,300 (veh/day)	-	7,915		
Lane width (ft)		12	12		
Shoulder width (ft) - right shoulder width for di	vided [if differ for directions of travel, use average width]	8	8		
Shoulder type - right shoulder type for divided		Paved	Paved		
Median width (ft) - for divided only		30	40		
Side Slopes - for undivided only		1:7 or flatter	Not Applicable		
Lighting (present/not present)		Not Present	Not Present		
Auto speed enforcement (present/not present)	Not Present	Not Present		
Calibration Factor, Cr		1.00	1.00		

Worksheet 1B (a) Crash Modification Factors for Rural Multilane Divided Roadway Segments						
(1)	(2)	(3)	(4)	(5)	(6)	
CMF for Lane Width	CMF for Right Shoulder Width	CMF for Median Width	CMF for Lighting	CMF for Automated Speed Enforcement	Combined CMF	
CMF 1rd	CMF 2rd	CMF 3rd	CMF 4rd	CMF 5rd	CMF comb	
from Equation 11-16	from Table 11-17	from Table 11-18	from Equation 11-17	from Section 11.7.2	(1)*(2)*(3)*(4)*(5	
1.00	1.00	0.99	1.00	1.00	0.99	

(1)	(2)		(3)	(4)	(5)	(6)	(7)	
Crash Severity Level	S	PF Coefficient	ts	N spf rd	Overdispersion	Combined CMFs	Calibration	Predicted average crash
	f	rom Table 11-5	5		Parameter, k	(6) from Worksheet	Factor, Cr	frequency, N predicted ra(d)
	а	b	С	from Equation 11-9	from Equation 11-10	1B (a)		(3)*(5)*(6)
Total	-9.025	1.049	1.549	9.392	0.033	0.99	1.00	9.298
Fatal and Injury (FI)	-8.837	0.958	1.687	5.008	0.029	0.99	1.00	4.958
Fatal and Injury ^a (Fi ^a)	-8.505	0.874	1.740	3.284	0.028	0.99	1.00	3.251
Property Damage Only (PDO)	-	-	-		-	-		(7) _{TOTAL} - (7) _{FI} 4.340

(1)	(2)	(3)	(4)	(5)	(6)	(7)	(8)	(9)
Collision Type	Proportion of Collision Type(TOTAL)	N predicted rs(d) (TOTAL) (crashes/year)	Proportion of Collision Type(FI)	N predicted rs(d) (Fi) (crashes/year)	Proportion of Collision Type (Fl ^a)	N predicted rs (FI ^a) (crashes/year)	Proportion of Collision Type (PDO)	N predicted rs(d) (PDO) (crashes/year)
	from Table 11-6	(7)TOTAL from Worksheet 1C (a)	from Table 11- 6	(7)⊧i from Worksheet 1C (a)	from Table 11-6	(7) _{FI} ^a from Worksheet 1C (a)	from Table 11-6	(7)PDO from Worksheet 1C (a)
Total	1.000	9.298	1.000	4.958	1.000	3.251	1.000	4.340
		(2)*(3) _{TOTAL}		(4)x(5) _{F1}		(6)*(7) _{F1} ^a		(8)*(9) PDO
Head-on collision	0.006	0.056	0.013	0.064	0.018	0.059	0.002	0.009
Sideswipe collision	0.043	0.400	0.027	0.134	0.022	0.072	0.053	0.230
Rear-end collision	0.116	1.079	0.163	0.808	0.114	0.371	0.088	0.382
Angle collision	0.043	0.400	0.048	0.238	0.045	0.146	0.041	0.178
Single-vehicle collision	0.768	7.141	0.727	3.604	0.778	2.529	0.792	3.438
Other collision	0.024	0.223	0.022	0.109	0.023	0.075	0.024	0.104

NOTE: * Using the KABCO scale, these include only KAB crashes. Crashes with severity level C (possible injury) are not included.

Worksheet 1E – Summary Results for Rural Multilane Roadway Segments					
(1)	(2)	(3)	(4)		
Crash severity level	Predicted average crash frequency (crashes/year)	Roadway segment length (mi)	Crash rate (crashes/mi/year) (2)/(3)		
	(7) from Worksheet 1C (a) or (b)	7			
Total	9.298	6.4	1.5		
Fatal and Injury (FI)	4.958	6.4	0.8		
Fatal and Injury ^a (FI ^a)	3.251	6.4	0.5		
Property Damage Only (PDO)	4.340	6.4	0.7		

Proposed Modified W4 Alternative				
Segment	Total Crashes	Fatal and Injury Crashes		
Rural Divided	9.457	5.035		
Suburban	1.522	0.454		
Intersection	Total Crashes	Fatal and Injury Crashes		
Eid Rd.	0.494	0.209		
Jacksha Rd.	0.464	0.195		
Old US-95 South	0.310	0.098		
Old US-95 North	0.521	0.196		
Subtotal	12.768	6.187		

Proposed Modified W4 Alternative					
Total (Crashes/year)	12.8				
Fatal and Injury	6.2				
Property Damage Only	6.6				

	Existing US-95			
Segment	Total Crashes	Fatal and Injury Crashes		
11	0.007	0.002		
12	0.021	0.007		
13	0.009	0.003		
14	0.021	0.007		
15	0.012	0.004		
16	0.026	0.009		
17	0.016	0.005		
18	0.053	0.017		
19	0.015	0.005		
20	0.102	0.033		
21	0.287	0.092		
22	0.563	0.181		
Intersection	Total Crashes	Fatal and Injury Crashes		
Zeitler Rd.	0.028	0.011		
Snow Rd.	0.017	0.007		
Skyview Dr.	0.009	0.004		
Clyde Rd. South	0.024	0.010		
Cameron Rd.	0.087	0.036		
Clyde Rd. North	0.129	0.054		
Subtotal	1.429	0.486		

Existing US-95 Loop				
Total (Crashes/year)	1.4			
Fatal and Injury	0.5			
Property Damage Only	0.9			

Proposed Modified W4 Alternative and Existing US-95 Loop					
Total (Crashes/year) 14.2					
Fatal and Injury	6.7				
Property Damage Only	7.5				

	Worksheet 1A General Information and Input Da	ta for Rural Multilane Road	way Segments	
Gene	al Information	Location Information		
Analyst Agency or Company	CJA, KJB ITD District 2	Roadway Roadway Section	US-95, Thorncreek to Moscow Modified W4 Rural - Divided	
Date Performed	12/30/14	Jurisdiction	Latah Co, ID	
		Analysis Year	2036	
	nput Data	Base Conditions	Site Conditions	
Roadway type (divided / undivided)		Undivided	Divided	
Length of segment, L (mi)			6.35	
AADT (veh/day)	AADT _{MAX} = 89,300 (veh/day)	-	8,044	
Lane width (ft)		12	12	
Shoulder width (ft) - right shoulder width for divid	ded [if differ for directions of travel, use average width]	8	8	
Shoulder type - right shoulder type for divided		Paved	Paved	
Median width (ft) - for divided only		30	40	
Side Slopes - for undivided only		1:7 or flatter	Not Applicable	
Lighting (present/not present)		Not Present	Not Present	
Auto speed enforcement (present/not present)		Not Present	Not Present	
Calibration Factor, Cr		1.00	1.00	

Worksheet 1B (a) Crash Modification Factors for Rural Multilane Divided Roadway Segments						
(1)	(2)	(3)	(4)	(5)	(6)	
CMF for Lane Width	CMF for Right Shoulder Width	CMF for Median Width	CMF for Lighting	CMF for Automated Speed Enforcement	Combined CMF	
CMF 1rd	CMF 2rd	CMF 3rd	CMF 4rd	CMF 5rd	CMF comb	
from Equation 11-16	from Table 11-17	from Table 11-18	from Equation 11-17	from Section 11.7.2	(1)*(2)*(3)*(4)*(5	
1.00	1.00	0.99	1.00	1.00	0.99	

(1)		(2)		(3)	(4)	(5)	(6)	(7)
Crash Severity Level	SPF Coefficients			N spf rd	Overdispersion	Combined CMFs	Calibration	Predicted average crash
	f	rom Table 11-	5		Parameter, k	(6) from Worksheet	Factor, Cr	frequency, N predicted raid)
	а	b	С	from Equation 11-9	from Equation 11-10	1B (a)		(3)*(5)*(6)
Total	-9.025	1.049	1.549	9.552	0.033	0.99	1.00	9.457
Fatal and Injury (FI)	-8.837	0.958	1.687	5.086	0.029	0.99	1.00	5.035
Fatal and Injury ^a (FI ^a)	-8.505	0.874	1.740	3.330	0.028	0.99	1.00	3.297
Property Damage Only (PDO)		-				_	-	(7) _{TOTAL} - (7) _{FI} 4.422

(1)	(2)	(3)	(4)	(5)	(6)	(7)	(8)	(9)
Collision Type	Proportion of Collision Type(TOTAL)	N predicted rs(d) (TOTAL) (crashes/year)	Proportion of Collision Type(FI)	N predicted rs(d) (Fi) (crashes/year)	Proportion of Collision Type (El ^a)	N predicted rs (FI [*]) (crashes/year)	Proportion of Collision Type (PDO)	N predicted rs(d) (PDO) (crashes/year)
	from Table	(7)TOTAL from Worksheet 1C	from Table 11-	(7)FI from Worksheet	from Table	(7) FI ^a from Worksheet	from Table	(7)PDO from Worksheet 1C
	11-6	(a)	6	1C (a)	11-6	1C (a)	11-6	(a)
Total	1.000	9.457	1.000	5.035	1.000	3.297	1.000	4.422
		(2)*(3) _{TOTAL}		(4)x(5) _{F1}		(6)*(7) _{FI} ^a		(8)*(9) PDO
Head-on collision	0.006	0.057	0.013	0.065	0.018	0.059	0.002	0.009
Sideswipe collision	0.043	0.407	0.027	0.136	0.022	0.073	0.053	0.234
Rear-end collision	0.116	1.097	0.163	0.821	0.114	0.376	0.088	0.389
Angle collision	0.043	0.407	0.048	0.242	0.045	0.148	0.041	0.181
Single-vehicle collision	0.768	7.263	0.727	3.660	0.778	2.565	0.792	3.502
Other collision	0.024	0.227	0.022	0.111	0.023	0.076	0.024	0.106

NOTE: * Using the KABCO scale, these include only KAB crashes. Crashes with severity level C (possible injury) are not included.

(1)	(2)	(3)	(4)	
Crash severity level	Predicted average crash frequency (crashes/year)	Roadway segment length (mi)	Crash rate (crashes/mi/year)	
· · · · · · · · · · · · · · · · · · ·	(7) from Worksheet 1C (a) or (b)		(2)/(3)	
Total	9.457	6.4	1.5	
Fatal and Injury (FI)	5.035	6.4	0.8	
Fatal and Injury ^a (FI ^a)	3.297	6.4	0.5	
Property Damage Only (PDO)	4.422	6.4	0.7	

US-95 THORNCREEK ROAD TO MOSCOW AASHTO HIGHWAY SAFETY MANUAL ANALYSIS ON ALTERNATIVES CARRIED FORWARD

DHP-NH-4110 (156) KEY # 09294 September 3, 2013

PREPARED BY DISTRICT 2 PROJECT DEVELOPMENT ENGINEER	
Curto J. armyn Curtos J. Arnzen, P.E.	
09/03/13 Date	



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Introduction

This Safety Analysis is a supporting document of the Environmental Impact Statement for US-95, Thorncreek to Moscow. This Safety Analysis will complete the following:

- Analyze the safety of the existing US-95 alignment,
- Quantify the safety benefit of the No Action Alternative and Alternatives E2, C3, and W4,
- And make an alternative recommendation based on safety.

This report replaces the safety analysis used for the Draft Environmental Impact Statement dated July 31, 2012. The report used for the DEIS was valid, but it was revised to address public comments received during the public comment period. It uses and reports updated crash data, and provides 20 years of predicted crash data starting in 2017, whereas, the previous safety analysis only provided crash predictions for 2017. The report provides additional analysis and information regarding predicted crashes on the remaining US-95 alignment that is proposed to be turned over to the North Latah Highway District primarily for local commuter traffic once an action alternative is constructed. The section that is proposed to be turned over to the North Latah Highway District primarily for local commuter traffic once an information regarding weather and wild animal related crashes.

As traffic volumes grow at an exponential rate, the number of predicted crashes per year increases. Selecting a safe alignment will result in a safety benefit every year after the construction of the new highway. With time, small differences in predicted crashes between the alternatives each year will grow to a significant safety benefit for an alternative that is predicted to have fewer crashes.

This report uses the First Edition (2010) of the American Association of State Highway and Transportation Officials (AASHTO) Highway Safety Manual (HSM) to analyze and quantify the safety benefits of each alternative. The HSM provides the most current and accepted knowledge and practices relating to safety management according to AASHTO and Transportation Research Board (TRB) Task forces.

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Summary

Safety is one of the key reasons for proposing the project. In fact, the Purpose and Need Statement in the Environmental Impact Statement is the following:

- Purpose The purpose of this project is to improve public safety and increase highway capacity on US 95 between Thorncreek Road and Moscow.
- Need Within the project limits, US95 does not meet current American Association of State Highway and Transportation Officials (AASHTO) Standards (widths, clear-zones, grades, and sight distance). Additional concerns include high accident locations and insufficient highway capacity.

The results of the calculation methods in the HSM show that Alternatives E2, C3, and W4 will be much safer than the No Action Alternative. The results of the calculation method also show Alternative E2 is the safest proposed alternative for total crashes, as well as total injury related crashes and fatalities. Table 1 shown below summarizes the findings of this safety analysis for 20 years of crashes starting in 2017. A 20 year crash forecast was used because the normal practice is to design a project using traffic volumes projected 20 years after the completion of the project. At this time, 2017 is the first year a safety benefit would be anticipated after completion of the project; however, a safety benefit will be realized every year after the project is completed. As traffic volumes grow at an exponential rate in the future, the safety benefit becomes much greater because traffic volumes are a factor in crash prediction equations used by the Highway Safety Manual. Supporting data and assumptions used to generate the table below are in Appendix E.

	Table 1: Predicted Crashes For Proposed Alternatives							
	Completio	on Year 2017	Crashes From 2017 Through 2036					
Alternative	Total Crashes	Fatal and Injury Crashes	Total Crashes	Fatal and Injury Crashes				
No Action	27.4	11.0	642.5	256.5				
E2	7.6	3.8	179.5	89.0				
C3	10.8	4.6	253.8	107.7				
W4	9.3	4.6	219.3	107.7				

In this safety analysis, the Idaho Transportation Department (ITD) predicted crashes on the remaining US-95 Loop for all of the action alternatives. The results of the calculation methods in the HSM show

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that Alternative E2 is still the safest predicted alternative after the crashes calculated on the remaining US-95 Loop are added to the total for all alternatives. The crash prediction results are shown in Table 2 below. Fewer total crashes and fewer fatal and injury crashes are predicted on Alternative E2 than Alternatives C3 and W4 even after crashes from the remaining US-95 Loop are considered in the calculated frequency. Alternative C3 has the highest predicted number of total crashes and Alternative W4 has the highest predicted number of fatal and injury crashes. The following table shows the crash predictions for each alternative after calculations for the remaining US-95 Loop are added. Supporting data and assumptions used to make crash predictions are in Appendix C and the crash prediction calculations used to generate the table are below in Appendix E.

	Completion Year 2017			017 Through 2036
Alternative	Total Crashes	Fatal and Injury Crashes	Total Crashes	Fatal and Injury Crashes
No Action	27.4	11.0	642.5	256.5
E2	9.2	4.4	213.9	100.7
C3	11.1	4.7	260.2	110.0
W4	10.5	5.1	246.2	116.9

Significance

Table 1 and Table 2 only report crash predictions for a 20 year period; however, once the fourlane highway between Thorncreek and Moscow is constructed, a safety benefit will be realized every year that US-95 between Thorncreek Road and Moscow is used by the traveling public for any of the action alternatives. Selecting an alternative based on safety will continue to benefit the traveling public every year after US-95 between Thorncreek Road and Moscow is constructed. Selecting an alternative based on safety will likely result in fewer fatalities and significantly reduced injuries over the course of its life.

Also, traffic is observed to grow at an exponential rate. Since crash predictions are a function of the volume of traffic, the number of predicted crashes increase every year. Future traffic growth magnifies the safety benefit predicted by a selected alternative.

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Economic Cost of Crashes

The ITD Office of Highway Safety uses the cost the FHWA establishes for preventing a fatality and other accidents as a basis for determining the cost of the other crash types. The National Highway Traffic Safety Administration (NHTSA) also did a study on the costs of crashes and determined who pays for the cost of crashes. The most significant point of this study is that society at large picks up nearly 75% of all crash costs incurred by individual motor vehicle crash victims. These costs are passed on to the general public through insurance premiums, taxes, direct out-of-pocket payments for goods and services, and increased charges for medical care. Economic values can be calculated for predicted crashes between Thorncreek Road and Moscow.

Using the average crash distribution rate of crashes in 2012, the economic cost of crashes in 2012, and a factor used to account for multiple injuries in accidents, the economic cost of crashes for the different proposed alternatives can be calculated. The economic cost for different accident types are published in the Idaho Transportation Department Office of Highway Safety's document titled Idaho Traffic Crashes 2012. The total economic cost of crashes for the proposed alternatives will show the significance relating to predicted crashes. For example, in 2012 the cost of a fatality is \$6,295,406 and the cost of a crash with property damage only is \$6,739. Table 3 shows the total estimated economic costs of all crashes on the different proposed alternatives between 2017 and 2036. The data and assumptions used to calculate the economic cost of crashes is found in Appendix D.

	Table 3: Total Economic Cost of Crashes on the Proposed Alternatives							
	Complet	Completion Year 2017		From 2017 Through 2036				
Alternative	Economic Cost	Difference From E2	Economic Cost	Difference From E2				
E2	\$1,100,000	\$0	\$26,000,000	\$0				
C3	\$1,400,000	\$300,000	\$32,000,000	\$6,000,000				
W4	\$1,400,000	\$300,000	\$32,000,000	\$6,000,000				
No Action Alternative	\$5,800,000	\$4,700,000	\$140,000,000	\$114,000,000				

Using 2012 crash costs and the economic cost of crashes from 2017 to 2036, the predicted economic cost of crashes on Alternative E2 is about \$6 million dollars less than Alternatives C3 and W4, and \$114 million dollars less than the No Action Alternative.

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Table 4, shown below, shows the total economic cost of all crashes on the different proposed alternatives including the remaining US-95 Loop that will remain after construction of the proposed alternative is complete. The data and assumptions used to calculate the total economic cost of crashes is in Appendix D.

	Table 4: Total Economic Cost of Crashes on the Proposed Alternatives Including the Remaining US-95 Loop from 2017 to 2036					
Completion Year 2017 From 2017 Through 2036						
Alternative	Economic Cost	Difference From E2	Economic Cost	Difference From E2		
E2	\$1,300,000	\$0	\$29,500,000	\$0		
C3	\$1,400,000	\$100,000	\$33,000,000	\$3,500,000		
W4	\$1,500,000	\$200,000	\$35,000,000	\$5,500,000		
No Action Alternative	\$5,800,000	\$4,500,000	\$140,000,000	\$110,500,000		

The estimated economic cost of crashes on Alternative E2 including the remaining US-95 Loop is about \$3.5 million dollars less than Alternative C3 including the remaining US-95 Loop, \$5.5 million dollars less than Alternative W4 including the remaining US-95 Loop, and \$110.5 million dollars less than the No Action Alternative.

The above-mentioned economic costs for crashes are for the first 20 years after construction of a proposed alternative; however, the economic savings due to a reduction of crashes is expected to continue over the entire lifetime that US-95 is being used if an action alternative is selected. Traffic volumes are predicted to increase exponentially, leading to more predicted crashes in the future. An increase in crashes will lead to a higher economic cost of crashes in the future and the safety benefit will continue to grow.

Safety Analysis of the Existing Alignment

Ten years of crash data on the existing alignment between MP 337.668 (Thorncreek Rd.) and MP 344.004 (Moscow) was analyzed in order to compare the safety of the existing alignment to the proposed alternatives. The crashes are shown in Appendix A.1 of this report. From January 1, 2003 through December 31, 2012, 253 crashes occurred or an average of 25.3 crashes per year.

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In addition to the high predicted number of total crashes, two of the District's top five Official High Crash Locations are located within this section of highway. Statewide, the two High Crash Locations in this section of roadway are ranked number 17 and 34. Appendix A.2 shows the list of High Crash Locations. The previous safety analysis used for the DEIS reported 3 High Crash Locations ranked 4, 6, and 16. High Crash Locations are based on three years of crash data and change annually depending upon the most recent crash data.

The crashes that have occurred on the existing alignment over the past 10 years appear to be random in nature and include head-on crashes, sideswipes, rear end turning, overturning, run off the road to the ditch and embankment, among other crash types. In the past 10 years, 6 fatalities have occurred in 5 crashes and 152 injuries have occurred in 253 crashes on US-95 between Thorncreek and Moscow. Two of the fatal crashes were head on collisions, one fatal crash was a sideswipe, one fatal crash was due to a motorist driving left of center into another car, and one was a pedestrian crash. The head-on crashes and sideswipe crashes are generally associated with passing maneuvers. The frequency of head-on, sideswipe, and driving left of center crash types is predicted to greatly decrease by replacing the 2-lane roadway with a new 4-lane roadway with a divided median. The US-95 project between the top of the Lewiston Hill and Thorncreek Road constructed a divided 4-lane roadway and has eliminated head-on crashes and sideswipes from cars traveling in the opposite direction since its completion in October 2007.

Approximately 40% of the existing crashes are from vehicles negotiating a curve. In the past 10 years, 19 crashes occurred with a motorist running off the road to the ditch, 20 crashes occurred with a motorist running off the road in an embankment area, and 89 crashes occurred with a motorist overturning a vehicle. The existing alignment does not meet AASHTO Standards for shoulder width, curve radius, sight distance, clear zone, and grade. Any action alternative will be designed to full AASHTO standards. The number of run off the road and overturning crashes is predicted to decrease if any action alternative is selected. The severity of the accidents is also predicted to decrease because the roadside clear zone will become more forgiving.

There are currently 66 at-grade intersections and approaches (public, commercial, residential, and field) in this 6.34 mile segment of US-95. Between 2003 and 2012, there were 26 crashes directly associated with private approaches, or intersections. The north end of the project is the

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most densely populated area. It has the highest number of access points and the highest number of intersection related crashes. The southern end of the project with its closely spaced approaches onto US-95, have also resulted in a high number of intersection related crashes. Currently, many approaches do not meet the ITD access control policy and contribute to intersection related conflicts. Twenty-one rear-ending crashes occurred on the existing alignment in the past 10 years. Rear ending accidents are generally associated with turning traffic to and from public roads and approaches to residencies, businesses, and industry. Any of the three proposed action alternatives would greatly reduce at grade intersections and approaches to US-95 and future approaches would not easily be granted because the access control would be purchased on the rural highway.

Currently, about 60% of the crashes on US-95 between Thorncreek Road and Moscow occur during inclement weather where the crash report lists snow, rain, fog, blowing snow, severe cross winds, or sleet/hail as the weather condition and has a road surface condition of wet, snow, ice, or slush. The number of crashes occurring during inclement weather is observed to be the greatest along curves with substandard radii. All existing alternatives will flatten curves to the AASHTO standard for radii and super-elevation, widen shoulders, widen clearzone, and construct a divided median, reducing the potential for weather related crashes.

There have been 32 wild animal crashes between Thorncreek Road and Moscow in the ten year period between January 2003 and December 2012. This is 13% of the total crashes; however, the severity of the crashes was very low, with 26 crashes being property damage only crashes, and 6 crashes being Type C Accidents (Possible Injury). The Idaho Department of Fish and Game have designated a portion of Thorncreek Road to Moscow as a low priority wildlife linkage area.

The economic cost crashes can be calculated for accidents between Thorncreek Road and Moscow based on the costs per accident type established by the NHTSA. The results of these costs are summarized in Table 5 Below:

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Table 5: Economic Cost of All Crashes Between Thorncreek and MoscowBetween 1/1/03 and 12/31/12					
Crash Type	Total Crashes	Total Injuries	Cost Per Injury or Crash (2012 Costs)	Total Cost	
Fatality	5	6	\$6,295,406	\$37,772,436	
Type A Accident (Serious)	18	27	\$313,516	\$8,464,932	
Type B Accident (Visible)	34	52	\$87,814	\$4,566,328	
Type C Accident (Possible)	44	74	\$58,209	\$4,307,466	
Property Damage Only	152	0	\$6,739	\$1,024,328	
Total	253			\$56,135,490	

From October 1, 2007, or the date the four lane divided highway from the Top of Lewiston Hill to Thorncreek Road (MP 323.36 to MP 337.668) was completed, to December 31, 2012, 27 injury crashes and no fatal crashes occurred on this new section of US-95, or 1.89 accidents per centerline mile. During the same time period on US-95 between Thorncreek Road and Moscow (MP 337.668 to MP 344.004), 55 injury and 3 fatal accidents occurred, or 9.2 injury crashes or fatal crashes per centerline mile.

During the public comment period, there were public comments to improve safety by reducing the speed limit on US-95 between Thorncreek Road and Moscow. However, reducing the speed limit would not make the roadway safer. An engineering speed study conducted in September 2012 concluded that the 85th percentile speed was 64 mph and that a 60 mph speed limit was adequate. Setting the posted speed limit at the 85th percentile speed is widely accepted and used by traffic professionals and the probability of crash occurrence is lowest for vehicles traveling at or slightly above the 85th percentile speed.

The conclusion of safety analysis of the existing alignment is that the existing crash data supports the need for the construction of an action alternative and reconstruction of US-95 between Thorncreek Road and Moscow with a four lane divided highway. The No Action Alternative is not acceptable because of the observed crash history of the existing alignment and the high economic cost of all crashes between Thorncreek Road and Moscow.

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Calculation Methodology for Action Alternatives

Standard Predictive Calculations

In order to calculate predicted number of crashes per year for proposed alternatives, Chapter 11-Predictive Method for Rural Multilane Highways and Chapter 12-Predictive Method for Urban and Suburban Arterials of the AASHTO Highway Safety Manual were followed. The Empirical Bayes method is not applicable since all three action alternatives are new and will be a different highway type than the existing facility.

The Highway Safety Manual calculates crashes using a base Safety Performance Function. The base Safety Performance Function is a regression equation that estimates the average crash frequency for a specific section of highway as a function of annual average daily traffic (AADT) and road length.

Once the base condition is calculated, Crash Modification Factors are applied to the base condition that either increase or decrease the predicted crash rate. Crash Modification Factors represent the relative change in crash frequency due to a change in one specific condition. An example of a Crash modification Factor might be a result of median width. The base condition for median width is 30 feet. Between Thorncreek and Moscow, the proposed median width is greater than the base condition, or 40 feet wide; therefore, a Crash Modification Factor of 0.99 is applied. This predicts that the 40 foot wide median will be 1% safer than the base condition of 30 feet.

After the Crash Modification Factors are applied to the base condition, a calibration factor may be applied to the overall crash data to account for local conditions. The calibration factor is multiplied by the overall crash costs after the Crash Modification Factors have been applied. At this time, the State of Idaho uses 1.0 as its calibration factor. The calibration factor may also be changed based on observed crash data. In the case of the Thorncreek to Moscow project, the proposed alternatives are new; therefore, existing data is not available and the calibration factor cannot be adjusted due to observed crash data.

Predictive Calculations on Proposed Alignments

Each of the three action alternatives has two different and distinct segments. One segment has characteristics of a rural multilane highway and the other segment has characteristics of a suburban arterial. Each segment within each alternative was modeled separately. Segments of

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highway that have a proposed 34 foot divided median (42' between the northbound and southbound travel lanes) and 65 mph speed limit were modeled as a rural divided multilane highway and segments of highway with five total lanes including a two-way left turn lane and a 45 mph speed limit were modeled as a suburban arterial. Typical sections for each proposed alternative are shown in Appendices C.1, C.2, and C.3. All public road intersections were also modeled with each proposed alternative.

In order to analyze the proposed alternatives equitably, all Crash Modification Factors and the predicted ADT for all proposed action alternatives are the same for the rural section of every action alternative and they are the same for the suburban sections of every action alternative. The Crash Modification Factors, the base conditions for the Crash Modification Factors, the input data, and other assumptions that affect the predicted number of crashes are shown on the spreadsheets shown within Appendix E. The primary factors that predict differences in safety between the action alternatives is the length of the rural section, length of the suburban section, and the number of county road approaches that intersect a proposed alternative.

Confidence intervals cannot be calculated for each of the proposed alternatives because some of the Crash Modification Factors do not have published standard deviation; however, all Crash Modification Factors used are widely accepted and published in the Highway Safety Manual. The confidence interval is nearly the same for all three proposed action alternatives because the Crash Modification Factors used for all action alternatives are identical within a specific highway type. The only slight differences between confidence intervals exist because the action alternatives have different lengths of rural and suburban highway sections.

Spreadsheets developed by Karen Dixon, PhD Civil Engineering, from Oregon State University were used for calculations and are shown in Appendix E of this report. Dr. Dixon was one of the authors of the AASHTO Highway Safety Manual.

Predictive Calculations on the Remaining US-95 Loop

As a result of public comments received during the DEIS comment period, ITD predicted crashes on the remaining US-95 Loop once one of the proposed action alternatives is constructed. Chapter 10 of the AASHTO Highway Safety Manual offers a method that will predict crashes on existing rural two-lane, two-way highways.

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The method used to predict crashes on rural two-lane, two-way highways is a multistep procedure that first splits a roadway section into numerous different sections or intersections based on roadway geometry, AADT, presence of a passing lane, or other factors. After the roadway has been divided into different segments, a Safety Performance Factor is calculated for each of the different segments. The Safety Performance Factor determines the predicted average crash frequency for a segment with certain base conditions, traffic volumes, and lengths. After the Safety Performance Factor is calculated, Crash Modification Factors are applied to measure the predicted variation in the number of crashes from the base condition. For example, the base condition for shoulder width is 6 feet. Since existing US-95 has a 2 foot wide shoulder, a Crash Modification Factor of 1.04 is applied to the Safety Performance Factor. The Crash Modification Factor increases the number of crashes by 4 % based on the reduced shoulder width. The Crash Modification Factors, the base conditions for the Crash Modification Factors, the input data, and other assumptions that affect the predicted number of crashes are shown on the spreadsheets shown within Appendix E.

A calibration factor of 1.0 was used on the remaining loop. The existing crash data set was not used to create a calibration factor, since the motorists that use the remaining loop will be different than the motorists that currently use US-95. Motorists that will use the remaining US-95 Loop if an action alternative is selected will primarily be commuters who have a destination along the remaining US-95 Loop, while the motorists who currently use US-95 will be a wide variety of motorists. Some of these motorists would be users who won't be as familiar with road conditions as commuters. A lower crash frequency would be expected with commuters and a modified calibration factor may not be appropriate. Also, since traffic volumes will be much smaller a calibration factor using existing crash data may not correlate to accurately predict crashes on the remaining US-95 Loop.

Wild Animal Crashes

To satisfy concerns about wild animal crashes, the wild animal crash rate was investigated between Thorncreek Road and Moscow and wild animal crash rates within ungulate crossing areas in Latah County identified by the Idaho Department of Fish and Game in Appendix B.1. Table 6, shown below, is a list of wild animal crashes within ungulate crossing areas that have been identified by the Idaho Department of Fish and Game.

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Table 6: Wild Animal Crash	es at Ungulate Crossing Areas on From 1/1/03 to 12/31/12	US-95 in Latah County					
Ungulate Crossing Area Total Wild Animal Crashes Wild Animal Crashes Per Yea							
Marsh Hill (MP 367.1 - 370.1)	27	2.7					
Crooks Hill (MP 356.0 - 359.0)	19	1.9					
Steakhouse Hill (MP 349.7 -352.7)	47	4.7					
Thorncreek to Moscow (MP 340.3-343.3)	17	1.7					

Currently, 17 of the 32 wild animal crashes on the existing alignment are between Thorncreek and Moscow occur within the identified ungulate crossing area.

Different wildlife technical reports indicate Alternative E2 may have more wild animal crash potential than the Alternatives C3 and W4 because 1.98 miles of E2 are within an ungulate impact area based on the results; however, greater sight distance on Alternative E2 may offset the wild animal crash potential. Sight distance will be greater on Alternative E2 because the length and radius of horizontal curvature is greater than the other action alternatives. Greater sight distance may reduce the crash potential of the wild animal crashes of Alternative E2 and offset the additional wild animal crash potential caused from Alternative E2 being in an ungulate impact area. Appendix B.2 shows the ungulate impact area in relationship to the alternatives.

The Highway Safety Manual Analysis Technique predicts some wild animal crashes within the base formulas; however, the wild animal crashes are not quantified within the formulas. The predicted crashes for each alternative generated using the HSM within this report include wild animal crashes.

A wildlife crash countermeasure that clears the roadside of trees and brush will be constructed. The crash countermeasure is predicted to reduce the total number of wild animal crashes to a rate that is similar to the number of wild animal crashes predicted by the base rate of the Highway Safety Manual. A report included in Appendix B.3 and titled "Methods to Reduce Traffic Crashes Involving Deer: What Works and What Does Not", shows a 50% reduction in animal crashes for railway clearing. This 50% reduction was achieved with the clearing of a 40

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to 60 meter strip for railway cars to avoid moose collisions. This report also acknowledges that roadside clearing is effective, but there is limited information supporting the extent of the wild animal crash reduction.

In July 2010, ITD District 6 completed a project to widen the roadside clear width from 30 feet to 60 feet from the roadway along US-20 between MP 369 and 375.5. For the ten years prior to the clearing project 37 wild animal crashes occurred or 3.7 crashes per year. Since the project only 1 wild animal crash has occurred, or about 0.4 crashes per year. This is about a 90% per year reduction in wild animal crashes so far. The data for this ITD project is shown in Appendix B.4. It should be noted that only 2 years and 6 months have passed since the completion of this project; however, the roadside clearing used on this project has substantially reduced wild animal crashes in the short time period.

For the proposed Thorncreek Road to Moscow Project, a minimum of 240' of Right-of-Way is estimated; however, in most areas the topography of the land will require a larger purchase of land that is estimated to be up to about 600' wide. The proposed Right-of-Way will be cleared of trees and brush providing a clear area that ranges from a minimum of 75' to maximum of about 330' from the edge of traveled way to the nearest possible brush or trees. The wide clear area is predicted to reduce the wild animal crash potential on all proposed alternatives.

It is difficult to pinpoint the amount of wild animal crashes that are expected and to quantify the difference in wild animal crashes on each of the action alternatives. ITD believes that there will not be a significant difference in wild animal crashes between the different alignments. The extra possible wild animal crash potential on a section of Alternative E2 may be offset by greater sight distance by the motorists and roadside clearing will greatly minimize the frequency of wild animal crashes.

While it is difficult to predict the number of wild animal crashes, we can estimate the severity of wild animal crashes. The severity of wild animal crashes is very low compared to other crash types. Because the severity of wild animal crashes is low, the current State Highway Safety Plan does not devote an emphasis area for wild animal crashes. As mentioned below in Table 7, about 91% of wild animal accidents are crashes resulting in property damage only. Of the 476 wild animal crashes along US-95 in District 2 during the past 10 years, no fatalities have occurred as a result of a wild animal crash and only 3 crashes (less than 1%) resulted in a

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serious injury. Wild Animal Crash Data used to generate Tables 7 and 8 are shown in Appendix B.5

Table 7: Wild Anim	nal Crashes aloi	ng US-95 in Distric Related Econ		003 Through 12/31	/2012 and Their
Crash Type	Number of Crashes	Percentage of Total	Total Number of Inuries	Cost of Crash Type (2012 Costs)	Total Cost
Fatality	0	0%	0	\$6,295,406	\$0
Type A Accident (Serious)	3	0.6%	3	\$313,516	\$940,548
Type B Accident (Visible)	11	2.3%	16	\$87,814	\$1,405,024
Type C Accident (Possible)	31	6.55%	39	\$58,209	\$2,270,151
Property Damage Only	431	90.55%	0	\$6,739	\$2,965,160
Total	476	100.0%	58		\$7,580,883

Table 8, shown below, shows that the total economic cost of wild animal crashes within the existing Thorncreek to Moscow Alignment from 1/1/03 to 12/31/12 is \$524,468. This cost is less than 1% of the total economic costs on the existing alignment between Thorncreek Road and Moscow during the same time period. All data used for prediction of wild animal crashes is based on crashes that have been reported to the police. Many wild animal crashes are not reported to the police because the result of the collision is not significant and does not include an injury or significant property damage. Unreported wild animal crashes are not a primary ITD safety concern, since they do not increase the number crashes with injury and the property damage is generally not significant.

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Table 8: Economic (Costs of Wild A	nimal Crashes Wit From 1/1/03 t	_	Thorncreek to Mo	scow Alignment
Crash Type	Number of Crashes	Percentage of Total	Number of Injuries	Cost of Crash	Total Cost
Fatality	0	0%	0	\$6,295,406	\$0
Type A Accident (Serious)	0	0%	0	\$313,516	\$0
Type B Accident (Visible)	0	0%	0	\$87,814	\$0
Type C Accident (Possible)	6	18.7%	6	\$58,209	\$349,254
Property Damage Only	26	81.3%	0	\$6,739	\$175,214
Total	32	100%			\$524,468

The Federal Highway Administration calculates the total economic cost of different crash types and estimates that property crashes resulting in property damage only are valued \$6,739 while fatalities are valued at \$6,295,406. Using these figures, 935 crashes causing property damage only are the equivalent of 1 fatality in terms of economic costs. Using this logic ITD is naturally more concerned about accident types that are likely to result in fatalities or severe accidents than accident types that generally result in property damage only.

The significance of potential additional wild animal crashes can be investigated by analyzing a hypothetical situation. If ITD was able to estimate the increased accident potential of Alternative E2 to be 1 extra wild animal accident per year, the significance of the extra crashes using the accident costs established by the FHWA and the percentages of accident types caused by wild animals can be calculated. For our 20 year crash study period, 20 additional wild animal crashes would result in an additional economic cost of \$310,000 for the 20 additional crashes. If \$310,000 is added to the estimated economic cost of crashes in Alternative E2, the total economic cost of crashes for Alternative E2 is still significantly less than the other action alternatives. In fact, 223 additional wild animal crashes in the next 20 years on Alternative E2 would be required to make the economic cost of accidents on Alternative E2 and its remaining US-95 Loop equivalent to Alternative C3 and its remaining loop. Two hundred twenty three additional wild animal crashes over the next 20 years on E2 is not a practical estimate of wild animal crashes given the fact that the roadside clearing countermeasure is being used, sight

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distance will be improved, and that no other location in Latah County has nearly as many wild animal crashes. All supporting calculations relating to these cost estimates are in Appendix D.

The conclusions about predicted wild animal crashes can be summarized in the following statements:

- Wild animal crash prediction on new alternatives is difficult to pinpoint.
- Alternative E2 may have greater wild animal crash potential than Alternatives C3 and W4 based on findings reported in the wildlife technical reports for Thorncreek to Moscow; however, greater sight distance on Alternative E2 may offset the increased wild animal crash potential. Since the greater sight distance may offset the increased wild animal crash potential on Alternative E2 and wild animal crash prediction is difficult, no factors increasing the number of wild animal crashes were applied to the crash predictions on Alternative E2.
- The roadside clearing crash countermeasure that will be used is predicted to greatly reduce the number of wild animal crashes and mitigate wild animal crash potential. Past data shows a 50% - 90% reduction in wild animal crashes when roadside clearing is used.
- The severity of wild animal crashes is very low compared to other crash types. Even if additional wild animal crashes were predicted on Alternative E2, the additional wild animal crashes would not offset the other safety benefits of Alternative E2 and Alternative E2 would still be significantly safer than the other action alternatives.

In conclusion, wild animal crashes should not be a dominant factor in selecting an alternative. Wild animal crashes have been observed to have low severity and low economic costs relative to the total amount of economic costs due to crashes and because it is predicted that the total number of wild animal crashes is not significantly greater for any of the alternatives. Alternative E2 may have more wild animal crash potential than Alternatives C3 and W4 because it is within an ungulate impact area; however, roadside clearing will reduce the wild animal crash potential. Wild animal crash rates are predicted to be similar to the wild animal crash rates that the base formulas of the HSM predict.

Crashes Relating to Unfavorable Weather Conditions

Approximately about 60% of the crashes on US-95 between Thorncreek Road and Moscow occur during inclement weather where the crash report lists snow, rain, fog, blowing snow,

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severe cross winds, or sleet/hail as the weather condition and has a road surface condition of wet, snow, ice, or slush. Therefore, the ITD commissioned Dr. Russell Qualls, Idaho State Climatologist and a Registered Professional Engineer, to study the weather patterns in the study area and make recommendations on proposed alternatives based on weather conditions. In Dr. Quall's report, he indicates that Alternative W4 would have colder temperatures and be more susceptible to frost; however, Alternative E2 and C3 would have greater precipitation than W4. Dr. Qualls suggested due to lack of a single, clearly superior alternative with regard to the impact of weather in the corridors, weather should not be a dominant factor in selecting one alternative over the other. For this report, all three alternatives are treated equally and no crash modification factors or calibration factors are applied to any of the alternatives for weather related crashes.

Crash Prediction Results for Proposed Alternatives

No Action Alternative

The existing alignment had 253 total crashes and 101 fatal and injury related crashes for the 10 year period from January 1, 2003 through December 31, 2012.

As AADTs between Thorncreek Road and Moscow grow and the two lane highway approaches its capacity, passing opportunities will decrease and crashes on US-95 are expected to increase. The frequency of crashes is predicted to increase at the same rate as the growth rate, or at 1.63% per year. Using a growth rate of 1.63% per year for crashes is reasonable because greater traffic volumes increase predicted crashes in HSM calculations. Using this method is a quick and easy way to predict future crashes by extrapolating existing crash data. The growth rate of predicted crashes used for this method compares reasonably well with the growth rate of predicted crashes on the action alternatives without completing extensive crash prediction calculations.

Between 2017 and 2036, the total number of crashes on the No Action Alternative is predicted to be 642.5 total crashes and 256.5 fatal and injury related crashes if no improvements are made. Increasing actual crash data for the existing alignment with a growth rate is a reasonable projection of crashes for the No Action Alternative. The predictions of crashes per year are shown in Appendix E.

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Alternative E2

Alternative E2 is predicted to have the fewest crashes of the three action alternatives and the No Action Alternative. Alternative E2 is the shortest alternative, has the fewest county road intersections, and has the fewest commercial and residential approaches. These factors all reduce the predicted crash rate. A grade separation is assumed at Eid Road due to the topography of the land and the turning movements in and out of the trailer park. Supporting data and assumptions used to make crash predictions are in Appendix C.1 and the crash prediction calculations used to generate the table below are in Appendix E. Table 9 shown below summarizes the crash data on Alternative E2 between 2017 and 2036.

Table 9: HSM Crash Results for Alternative E2						
······	Constructio	n Year 2017	Crashes From 20	017 Through 2036		
	Total Crashes	Fatal and Injury Crashes	Total Crashes	Fatal and Injury Crashes		
Rural Divided Multilane Segment	6.1	3.3	142.9	77.1		
Suburban Segment	0.9	0.3	22.1	6.6		
South Old US-95 Intersection	0.2	0.1	5.7	1.9		
North Old US-95 Intersection	0.4	0.1	8.8	3.4		
Total	7.6	3.8	179.5	89.0		

Crashes within the remaining US-95 Loop once Alternative E2 is constructed were also calculated. To calculate the crashes on the remaining US-95 Loop, Existing US-95 was separated into 23 different segments based on roadway geometry and other factors. The different segments and assumptions used for the calculations are shown in Appendix C.1. Crashes for all segments and intersections onto the remaining US-95 Loop were calculated and results are shown in Appendix E. More crashes are predicted on the remaining loop for E2 than the remaining loop for W4 and C3 because the remaining loop on E2 is longer, has a greater traffic volume, and has more intersections. Since the traffic volumes are much smaller on the remaining loop for the action alternatives, Alternative E2 and the remaining US-95 Loop still has a significant safety advantage over Alternatives C3 and W4 with their respective remaining loops.

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Table 10: HSM	Crash Results for	Alternative E2 and	d the Remaining US	-95 Loop
	Construction Year 2017		Crashes From 2017 Through 2036	
	Total Crashes	Fatal and Injury Crashes	Total Crashes	Fatal and Injury Crashes
Alternative E2	7.6	3.8	179.5	89.0
Remaining US-95 Loop	1.6	0.6	34.4	11.8
Total	9.2	4.4	213.9	100.7*

*Note: Differences between the total number and the sum of components are due to rounding. The actual numbers that have not been rounded can be found in Appendix E.

An estimate of the economic cost of all accidents on Alternative E2 can be calculated using the HSM Crash Results shown above, the economic costs of the different crash types reported in Idaho Traffic Crashes 2012, the average frequency of the different injury and fatal accidents on Idaho's Highways, and the average multiple car crash frequency.

The estimated economic cost of all crashes on E2 between 2017 and 2036 is about \$26,000,000 and the estimated economic cost of crashes on E2 and the remaining US-95 Remaining Loop between 2017 and 2036 is about \$29,500,000. Supporting data, assumptions, and calculations used to calculate the economic cost of crashes is shown in Appendix D.

Alternative C3

Alternative C3 is predicted to be the least safe action alternative in terms of total crashes and is tied with Alternative W4 with the most fatal and injury crashes. It has the longest five lane suburban section with a two-way left turning lane of the three action alternatives. Crashes are predicted at a rate of 3.4 crashes per centerline mile for the five lane suburban section while the rural four lane divided section has a predicted rate of 1.1 crashes per mile. Alternative C3 also has the most residential and commercial approaches of the three alternatives. The numerous residential and commercial approaches result in greater numbers of predicted crashes due to vehicles turning on and off of US-95. Five at-grade intersections at Eid Road, Clyde Road, Cameron Road, North Old US-95, and South Old US-95 must be constructed to accommodate local traffic and crashes associated with the additional county road intersections are predicted.

A grade separation is currently assumed at Zeitler Road. Supporting data and assumptions used to make crash predictions are in Appendix C.2 and the crash prediction calculations used to

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generate the table is in Appendix E. Table 11, shown below, summarizes the predicted crashes for Alternative C3.

	Table 11: HSM Crash Results for Alternative C3						
	Constructio	n Year 2017	Crashes From 2017 Through 2036				
	Total Crashes	Fatal and Injury Crashes	Total Crashes	Fatal and Injury Crashes			
Rural Divided Multilane Segment	4.9	2.7	115.2	62.1			
Suburban Segment	4.8	1.5	111.6	33.6			
South Old US-95 Intersection	0.3	0.1	8.2	3.5			
Eid Road Intersection	0.2	0.1	5.3	1.7			
Cameron Road Intersection	0.2	0.1	3.7	1.7			
Clyde Road Intersection North	0.2	0.1	5.6	2.8			
North Old US-95 Intersection	0.2	0.1	4.2	2.2			
Total	10.8	4.6*	253.8	107.7			

*Note: Differences between the total number and the sum of components are due to rounding. The actual numbers that have not been rounded can be found in Appendix E.

Crashes within the remaining US-95 Loop once Alternative C3 is constructed were also calculated. To calculate the crashes on the remaining US-95 Loop, Existing US-95 was separated into 10 different segments based on roadway geometry and other factors as shown in Appendix C.2. Crashes for all segments and intersections onto the remaining US-95 Loop were calculated. Fewer crashes are predicted on the remaining loop for C3 than the remaining loop for E2 and W4 because the remaining loop on C3 is shorter, has a smaller traffic volume, and has more intersections. Since the traffic volumes are much smaller on the remaining loop for the action alternatives, the safety benefits from having a shorter remaining loop with less traffic does not offset the safety advantage of Alternative E2, but it does make Alternative C3 safer than Alternative W4 in terms of fatal and injury crashes and total economic costs as shown below. The calculations are shown in Appendix E.

DHP-NH-4110 (156); Key No. 9294; Thorncreek to Moscow

September 3, 2013

Table 12: H	SM Crash Results	for Alternative C3	and Remaining US-9	5 Loop
	Constructio	on Year 2017	Crashes From 2017 Through 2036	
	Total Crashes	Fatal and Injury Crashes	Total Crashes	Fatal and Injury Crashes
Alternative C3	10.8	4.6	253.8	107.7
Remaining US-95 Loop	0.3	0.1	6.4	2.3
Total	11.1	4.7	260.2	110.0

An estimate of the economic cost of all accidents on Alternative C3 can be calculated using the HSM Crash Results shown above, the economic costs of the different crash types reported in Idaho Traffic Crashes 2012, the average frequency of the different injury and fatal accidents on Idaho's Highways, and the average multiple car crash frequency.

The estimated economic cost of crashes on C3 between 2017 and 2036 is calculated to be about \$32,000,000 and the estimated economic cost of crashes on C3 and the remaining US-95 Loop between 2017 and 2036 is calculated to be about \$33,000,000. The data, assumptions, and calculations for the economic cost of crashes is shown in Appendix D.

Alternative W4

Alternative W4 is predicted to have more total crashes, fatal crashes, and injury crashes than Alternative E2, but is predicted to have fewer total crashes than Alternative C3. Alternative W4 is tied with Alternative C3 in fatal and injury crashes. Alternative W4 is the longest proposed action alternative, and has four proposed county road intersections. A grade separation at Snow Road is assumed due to the topography of the land in relation to Snow Road.

Supporting data and assumptions used to make crash predictions are in Appendix C.3 and the crash prediction calculations used to generate the table is in Appendix E. Table 15, shown below, summarizes the predicted crashes for Alternative W4.

DHP-NH-4110 (156); Key No. 9294; Thorncreek to Moscow

September 3, 2013

	Table 13: HSM Crash Results for Alternative W4						
	Constructio	n Year 2017	Crashes From 2017 Through 2036				
	Total Crashes	Fatal and	Total Crashes	Fatal and Injury			
		Injury Crashes		Crashes			
Rural Divided Multilane	6.9	3.8	163.1	88.0			
Segment							
Suburban Segment	1.1	0.3	26.2	7.9			
Eid Road Intersection	0.3	0.1	8.2	3.5			
Jacksha Road	0.3	0.1	7.7	3.3			
Intersection							
South Old US-95	0.2	0.1	5.2	1.7			
Intersection							
North Old US-95	0.4	0.1	8.8	3.4			
Intersection							
Total	9.3*	4.6*	219.3	107.7			

*Note: Differences between the total number and the sum of components are due to rounding. The actual numbers that have not been rounded can be found in Appendix E.

Crashes within the remaining US-95 Loop if Alternative W4 is constructed were also calculated. To calculate the crashes on the remaining US-95 Loop, Existing US-95 was separated into 12 different segments based on roadway geometry and other factors as shown in Appendix C.3. Crashes for all segments and intersections onto the remaining US-95 Loop were calculated. Fewer crashes are predicted on the remaining loop of W4 than the remaining loop of E2 because the remaining loop on W4 is shorter, has a smaller traffic volume, and has fewer intersections. More crashes are predicted on the remaining loop of W4 than the remaining loop of C3, because the remaining loop on C3 is shorter and has fewer intersections. Since the traffic volumes and their predicted growth rates are much smaller on the remaining loop with less traffic does not offset the safety advantage of Alternative E2, but it does make Alternative C3 safer than Alternative W4 in terms of fatal and injury crashes as shown below. Calculations are shown in Appendix D.

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Table 14: HS	SM Crash Results	for Alternative W4	and Remaining US-9	95 Loop
	Constructio	on Year 2017	Crashes From 20	017 Through 2036
-	Total Crashes	Total Crashes Fatal and		Fatal and Injury
		Injury Crashes		Crashes
W4 Alternative	9.3	4.6	219.3	107.7
Remaining US-95 Loop	1.3	0.4	26.9	9.2
Total	10.5*	5.1*	246.2	116.9

*Note: Differences between the total number and the sum of components are due to rounding. The actual numbers that have not been rounded can be found in Appendix E.

An estimate of the economic cost of all accidents on Alternative W4 can be calculated using the HSM Crash Results shown above, the economic costs of the different crash types reported in Idaho Traffic Crashes 2012, the average frequency of the different injury and fatal accidents on Idaho's Highways, and the average multiple car crash frequency.

The estimated economic cost of crashes on Alternative W4 between 2017 and 2036 is calculated to be about \$32,000,000 and the estimated economic cost of crashes on Alternative W4 and the remaining US-95 Loop is calculated to be about \$35,000,000. Data, assumptions, and calculations used to create the total economic cost of crashes can be found in Appendix D.

Conclusion

The First Edition of the AASHTO Highway Safety Manual (2010) was used to calculate predicted crash rates for the three different alternatives carried forward on the Thorncreek to Moscow project. AASHTO and TRB Task forces recognize that that the Highway Safety Manual is the most accepted and current document that provides knowledge and practices relating to safety evaluation and management. The manual was developed as a tool for crash analysis and estimation. The following table summarizes the calculations based on the First Edition of the AASHTO Highway Safety Manual:

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Table 15: Predicted Crashes For Proposed Alternatives					
Completion Year 2017 Crashes From 2017 Through 2036					
Alternative	Total Crashes	Fatal and Injury Crashes	Total Crashes	Fatal and Injury Crashes	
No Action	27.4	11.0	642.5	256.5	
E2	7.6	3.8	179.5	89.0	
C3	10.8	4.6	253.8	107.7	
W4	9.3	4.6	219.3	107.7	

The ITD also calculated the crashes on each of the remaining US-95 Loops for the action alternatives. The following table shows the crash predictions for each alternative after calculations for the remaining US-95 Loop are added.

	Completio	on Year 2017	Crashes From 2	017 Through 2036
Alternative	Total Crashes	Fatal and Injury Crashes	Total Crashes	Fatal and Injury Crashes
No Action	27.4	11.0	642.5	256.5
E2	9.2	4.4	213.9	100.7
C3	11.1	4.7	260.2	110.0
W4	10.5	5.1	246.2	116.9

Calculations from the AASHTO Highway Safety Manual show that all alternatives are predicted to be safer than the No Action Alternative and eliminate two High Crash Locations. Selecting any action alternative is predicted to significantly reduce fatalities and the different crash types; however, selecting Alternative E2 will result in the greatest safety benefit. Calculations show that Alternative E2 is predicted to be safer than both Alternatives C3 and W4,

both in total crashes, and fatal and injury crashes. The following are the reasons that Alternative E2 is predicted to be the safest proposed alternative:

- Alternative E2 is the shortest alternative.
- Alternative E2's suburban section is 1.18 miles shorter than Alternative C3's suburban section.

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- Alternative E2 has the fewest public road intersections.
- Alternative E2 has the fewest residential and commercial approaches.

Alternative C3 is not safer than Alternative E2 primarily because of the following:

- Alternative C3 has the longest 5 lane Suburban Section. The suburban section is 1.18 miles longer than the suburban section of Alternative E2. The suburban section generates more crashes than the rural section because the travel lanes are closer together and not separated by a median and because the five lane section has more turning movements from commercial and residential approaches that cause more crashes.
- Alternative C3 has the most residential and commercial approaches.
- Alternative C3 has the most public road intersections.

Alternative W4 is not safer than Alternative E2 primarily because of the following:

- Alternative W4 is the longest alternative and is longer than Alternative E2 by 0.84 miles.
- Alternative W4 has more county road intersections than Alternative E2.

All calculations in this report are for a 20 year design period from the time the construction of Thorncreek to Moscow is expected to be complete. Selecting the safest alignment will result in a safety benefit every year after the construction of the new highway. With time, relatively small differences in predicted crashes between the alternatives each year will grow to a significant safety benefit for an alternative that is predicted to have fewer crashes. Alternative E2 has 18.7 fewer predicted fatal and injury crashes than Alternatives C3 and W4 in the first 20 years and this difference in predicted crashes will continue to grow with time. It is not unreasonable to predict that lives will be saved and numerous injuries will be prevented over the entire life of US-95 Thorncreek to Moscow by selecting Alternative E2 using these crash predictions and the average fatality rates in Idaho.

From a safety perspective, Alternative E2 satisfies the Purpose and Need Statement to a greater extent than Alternatives C3 and W4 and is the recommended alternative based on safety because it has the lowest predicted crash rate. The reason it has the lowest predicted crash rate is because it is the shortest alternative, has the fewest public road intersections, and has the fewest approaches. Selecting Alternative E2 will result in a safety benefit every year for the entire life of the highway and will likely save lives and prevent injuries.

Appendix A.1

Crash Data

- Thorncreek Road to Moscow Crash Data From 1/03 through 12/12
- Thorncreek Road to Moscow Injury Classification Data From 1/03 through 12/12
 - Lewiston Hill to Thorncreek Road Crash Data From 10/07 through 12/12

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Property Dmg Report	10/16/2008	Thursday	0	0	Dark, No Street Lights	Wet	Cloudy	None	None	None	On Roadway	Nonjunction	Animal - Domestic	Descending S	Negotiating Curve	Pickup/Van/ Panel/SUV	338.100	20
Property Dmg Report	11/22/2004	Monday	0	0	Day	Dry	Cloudy	None	None	Inattention	Right Shoulder	Nonjunction	Overturn	Descending S	Going Straight	Car I	338.100	19
B Injury Accident	8/19/2003	Tuesday		0	Dark, No Street Lights	Dry	Clear	None	Speed Too Fast For Conditions	None	Left Shoulder	Nonjunction		Ascending	Negotiating Curve As	Car	338.100	18
B Injury Accidemt	2/22/2007		0	0	-				None	None	Outside Right-Of- Way	Nonjunction	6	Descending N	Going Straight De	Pickup/Van/ Panel/SUV	338.100	13
B Injury Accident	2/22/2007	Thursday	2		Dark, No Street Lights	Snow	Cloudy	None	None	None		Nonjunction	Side Swipe Opposite	Descending S	Going Straight De	Car	338.100	1
Property Dmg Report	12/4/2005		0	•			-		Following Too Close	Inattention	On Roadway	Nonjunction	Rear-End	Ascending N	Going Straight As		338.100	16
Property Dmg Report	12/4/2005	Sunday	0	0	Day		Cloudy	None	None	None		Nonjunction	Rear-End	Ascending N	Going Straight As	Pickup/Van/ Panel/SUV	338.100	16
Property Drng Report	9/15/2008	Monday	0	•	Dark, No Street Lights	Dīv	Clear	None	None	None	On Roadway	Nonjunction	Animal - Wild	Descending S	Negotiating Curve De	Car	338.056	5
Property Dmg Report	11/28/2011	Monday	•	0	Dawn or Dusk	Ē	Cloudy	None	None	Speed Too Fast For Conditions	Private Property	Nonjunction	Overturn	Ascending N	Going Straight As	Pickup	338.040	14
C Injury Accident	4/6/2010	Tuesday	1	0	Dark, No Street Lights	Ice	Cloudy	None	None	Speed Too Fast For Conditions	Roadside or Sidewalk	Nonjunction	Overturn	Ascending N	Going Straight As	Pickup/Van/ Panel/SUV	338.038	13
Property Dmg Report	2/25/2012	Saturday	•	0	Day	Snow	Blowing	None	None	Speed Too Fast For Conditions	Right Shoulder	Nonjunction	Ditch	Ascending N	Going Straight As	Pickup	338.019	5
Fatal Accident	1/7/2011		0	0					None	None		Nonjunction	Angle	Descending S	Going Straight De	Car	338.012	
Fatal Accident	1/7/2011	Friday	4	1	Dark, No Street Lights	Snow	Snow	None	Drug Impaired	Drove Left of Center	On Roadway	Nonjunction	Angle	Descending N	Going Straight Dev	Car	338.012	Ħ
C Injury Accident	11/20/2012	Tuesday	1	•	Dark, No Street Lights	Wet	Rain	None	None	Animal(s) in Roadway	On Roadway	Nonjunction	Animal - Wild	Descending S	Going Straight Des	Car	338.004	10
C Injury Accident	9/30/2012	Sunday	-	0	Day	Dry	Clear	None	Failed to Maintain Lane	Alcohol Impaired	Outside Right-Of- Way	Nonjunction	Overturn	Ascending N	Going Straight As		337.998	و.
Property Dmg Report	4/16/2010		0	•					None	None	On Roadway	Nonjunction	Animal - Wild	Descending S	Going Straight Des	Pickup/Van/ Panel/SUV	337.973	•
Property Dmg Report	4/16/2010	Friday	0	•	Dark, No Street Lights	Dıy	Clear	None	None	None	On Roadway	Nonjunction	Animal - Wild	Descending S	Going Straight Des	Car	337.973	
B Injury Accident	8/17/2012		0	•					None	None	Roadside or Sidewalk	Nonjunction	Tree	Ascending N	Going Straight Asc	Car	337.941	7
B Injury Accident	8/17/2012		0	0					None	None		Nonjunction	Head-On	Ascending N	Going Straight Asc	Car	337.941	7
B Injury Accident	8/17/2012	Friday	ω	0	Day	Dry	Clear	None	Failed to Maintain Lane	Asleep, Drowsy, Fatigued	On Roadway	Nonjunction	Overturn	Ascending S	Going Straight Asc	<	337.941	-
B Injury Accident	12/1/2010	Wednesday	4	0	Day	Wet	Cloudy	None	Overcorrected	Asleep, Drowsy, Fatigued	Roadside or Sidewalk	Nonjunction	Overturn	Descending S	Going Straight Des	Pickup/Van/ Panel/SUV	337.900	6
C Injury Accident	9/4/2008		0	0					Fallowing Tap Close	Inattention	On Roadway	Driveway/Alley/P arking Lot Related	Rear-End	Descending S	Going Straight Des	Pickup/Van/ Panel/SUV	337.897	v
C Injury Accident	9/4/2008	Thursday	1	0	Day	Dry	Clear	None	None	None		Driveway/Alley/P arking Lot Related	Rear-End	Descending S	Slowing in Traffic Des	Gr	337.897	U
Property Dmg Report	9/27/2008	Saturday	•	0	Day	Dη	Clear	None	Inattention	None	Roadside or Sidewalk	Nonjunction	Fence	Ascending N	Going Straight Asc	Car	337.800	4
A Injury Accident	1/30/2007	Tuesday	2	•	Day	Wet	Fog	None	Inattention	Tire Defect	Roadside or Sidewalk	Nonjunction	Ditch	Ascending N	Going Straight Asc		337.800	ω
Property Dmg Report	3/28/2008	Friday	0	0	Dark, No Street Lights	lce	Cloudy	None	None	None	Roadside or Sidewalk	Nonjunction	Overturn	Descending S	Going Straight Des	Pickup/Van/ Panel/SUV	337.700	2
Property Drng Report	11/30/2005	Wednesday	0	•	Day	Slush	Snow	None	None	Other	Right Shoulder	Nonjunction	Overturn	Ascending N	Going Straight Asc	Pickup/Van/ Panel/SUV	337.689	4
Severity	AccidentDate	Day	Injuries	Fatalities Injuries	Light	Surface	Weather	Road	Contributing Circumstance 2	Contributing Circumstance 1	Event Relation To Road	Junction	Event 1	Lane	Driver Action Dir	Milepost Vehicle Type D	Milepost	*

All Accidents on 03-33 between

Fatalities 6

Total Crashes 253

Injuries 152

47	46	45	4	43	42	41	8	39	38	37	36	35	35	34	34	33	32	31	30	8	29	28	27	26	25	24	23	22	22	22	12	#
338.700	338,600	338.600	338.500	338.500	338.500	338.400	338.400	338.400	338.300	338.300	00E'8EE	338.300	338.300	338.200	338.200	338.200	338.200	338.200	338.200	338.200	338.200	338.200	338.200	338.130	338.100	338.100	338.100	338.100	338.100	338.100	338,100	Milepost
Pickup/Van/ Panel/SUV	Pickup	Car	Pickup	Pickup/Van/ Panel/SUV	Pickup/Van/ Panel/SUV	SUV/Crossov er	SUV/Crossov er	Gar	Car	Pickup/Van/ Panel/SUV	Pickup/Van/ Panel/SUV	Pickup/Van/ Panel/SUV	Pickup/Van/ Panel/SUV	Pickup	Car	SUV/Crossov er	Car	Car	Pickup/Van/ Panel/SUV	Pickup/Van/ Panel/SUV	Pickup/Van/ Panel/SUV	Pickup/Van/ Panel/SUV	Pickup/Van/ Panel/SUV	Car	Car	Car	Car	Car	Car	Pickup/Van/ Panel/SUV	Pickup/Van/ Panel/SUV	Vehicle Type
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Nonjunction	Nonjunction	Nonjunction	Nonjunction	Nonjunction	Nonjunction	Nonjunction	Nonjunction	Nonjunction	Nonjunction	Nonjunction	Nonjunction	Nonjunction	Nonjunction	Nonjunction	Nonjunction	Nonjunction	Nonjunction	Nonjunction	Nonjunction	Nonjunction	Nonjunction	Nonjunction	Nonjunction	Nonjunction	Nonjunction	Nonjunction	Nonjunction	Nonjunction	Nonjunction	Nonjunction	Nonjunction	Junction
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Nonjunction	Nonjunction	Nonjunction	Nonjunction	Nonjunction	Nonjunction	Nonjunction	Nonjunction	Nanjunction	Nonjunction	Nonjunction	Nonjunction	Nonjunction	Nonjunction	Nonjunction	Nonjunction	Nonjunction	Nonjunction	Nonjunction	Nonjunction	Nonjunction	Nonjunction	Nonjunction	Nonjunction	Nonjunction	Nonjunction	Nonjunction	Nonjunction	Nonjunction	Junction
Left Shoulder	Outside Right-Of- Way	Outside Right-Of- Way	Left Shoulder	Roadside or Sidewalk	On Roadway	Left Shoulder	Right Shoulder	Right Shoulder	Roadside or Sidewalk	Left Shoulder	Right Shoulder	Right Shoulder	Right Shoulder	Left Shoulder	Roadside or Sidewalk		On Roadway	Left Shoulder	Right Shoulder	Outside Right-Of- Way		On Roadway	Left Shoulder	Right Shoulder	Outside Right-Of- Way	Right Shoulder	Left Shoulder	Roadside or Sidewalk	Event Relation To Road
Speed Too Fast For Conditions	f- Speed Too Fast For Conditions	None	Speed Too Fast For Conditions	Inattention	Wrong Side or Wrong Way	Other	Speed Too Fast For Conditions	Inattention	Speed Too Fast For Conditions	None	Alcohol Impaired	Other	Speed Too Fast For Conditions	None	Speed Too Fast For Conditions	None	None	None	None	Alcohol Impaired	None	Drove Left of Center	None	Speed Too Fast For Conditions	Speed Too Fast For Conditions	Other	Speed Too Fast For Conditions	Speed Too Fast For Conditions	Contributing Circumstance 1
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11/16/2011	2/11/2012	11/8/2009	12/31/2003	3/24/2005	3/22/2012	3/22/2012	11/8/2012	9/15/2012	12/9/2011	12/4/2010	11/21/2010	1/9/2010	1/10/2009	12/26/2004	1/7/2004	1/6/2004	1/6/2004	2/2/2003	12/1/2007	4/13/2007	3/26/2007	3/26/2007	11/18/2006	12/10/2003	3/1/2012	4/24/2003	2/22/2003	2/6/2008	AccidentDate
Property Dmg Report	Property Dmg Report	Property Dmg Report	Property Dmg Report	C Injury Accident	Property Dmg Report	Property Dmg Report	Property Dmg Report	C Injury Accident	Property Dmg Report	Property Dmg Report	B Injury Accident	Property Dmg Report	Property Dmg Report	B Injury Accident	B Injury Accident	Property Dmg Report	Property Dmg Report	Property Dmg Report	B Injury Accident	Property Dmg Report	Property Dmg Report	Property Dmg Report	Property Dmg Report	Property Dmg Report	Property Dmg Report	A Injury Accident	Property Dmg Report	C Injury Accident	Severity

Property Dmg Report	10/31/2005		•	0					None	None		Nonjunction	N Rear-End	Ascending	Stopped in Traffic	Pickup/Van/	340.300	125
Property Dmg Report	10/31/2005	Monday		0	Day	Wet	Cloudy	None	None	None	On Roadway	Nonjunction	N Rear-End	Ascending	Going Straight		340.300	125
B Injury Accident	1/24/2007		-	•		+			None	None		Nonjunction	N Head-On	Ascending	Going Straight	Pickup/Van/ Panel/SUV	340.250	124
B Injury Accident	1/24/2007	Wednesday	2	0	Day	Dry	Clear	None	Drove Left of Center	Distracted IN or ON Vehicle	On Roadway	Nonjunction	S Head-On	Ascending	Negotiating Curve	Car	340.250	124
B Injury Accident	5/18/2012	Friday	-	0	Day	Dıy	Clear	None	None	Animal(s) in Roadway	On Roadway	Nonjunction	S Ditch	Descending	Avoiding Obstacle	Саг	340.200	123
C Injury Accident	12/14/2007	Friday	ы	0	Dawn or Dusk	lce	Cloudy	None	Inattention	Drove Left of Center	Roadside or Sidewalk	Nonjunction	N Overturn	Descending	Going Straight		340.100	122
Property Dmg Report	5/26/2009	Tuesday	0	•	Day	Dry	Clear	None	None	None	On Roadway	Nonjunction	S Other	Descending	Going Straight	-	340.027	121
Property Dmg Report	3/26/2004	Friday	0	0	Day	Dη	Cloudy	None	None	None	On Roadway	Nonjunction	N Other Object Not Fixed	Ascending	Going Straight		340.000	120
B Injury Accident	11/12/2008	Wednesday	2	0	Dark, No Street Lights	Wet	Rain	None	None	Other	Off Roadway- Location Unknown	Nonjunction	N Overturn	Descending	Going Straight	Pickup/Van/ Panel/SUV	339.900	119
Property Dmg Report	4/9/2011	Saturday	0	•	Dark, No Street Lights	Dry	Clear	None	None	None	On Roadway	Nonjunction	N Animal - Wild	Ascending	Going Straight	Pickup	339.850	118
C Injury Accident	10/1/2005	Saturday	1	0	Dark, No Street Lights	Wet	Cloudy	None	Other	None	On Roadway	Nonjunction	N Animal - Wild	Ascending	Going Straight	G	339.800	117
Property Dmg Report	12/12/2004	Sunday	0	0	Day	ice	Clear	None	None	None	Left Shoulder	Nonjunction	S Embankment	Ascending	Turning Left	Pickup/Van/ Panel/SUV	339.731	116
Property Dmg Report	6/5/2005	Sunday	0	0	Дау	Wet	Rain	Other	None	None	Right Shoulder	Nonjunction	S Traffic Sign Support	Descending	Going Straight	Pickup/Van/ Panel/SUV	339.700	115
Property Drng Report	2/2/2010		0	0					None	None		Intersection Related	N Rear-End Turning	Ascending	ight	Car	339.620	114
Property Dmg Report	2/2/2010		0			1			None	None		Intersection Related	N Rear-End	Ascending N	Slowing in Traffic	Pickup/Van/ Panel/SUV	339.620	114
Property Dmg Report	2/2/2010	Tuesday	0	•	Day	Dıy	Cloudy	None	None	Inattention	On Roadway	Intersection Related	N Rear-End	Ascending N	Going Straight	Pickup/Van/ Panel/SUV	339.620	114
A Injury Accident	1/30/2008	Wednesday	2	0	Day	Ice	Clear	None	None	None	Roadside or Sidewalk	Nonjunction	V Embankment	Descending	Going Straight	Pickup/Van/ Panel/SUV	339.620	113
C Injury Accident	12/13/2007	Thursday	-	•	Day	łce	Clear	None	None	None	Roadside or Sidewalk	Nonjunction		Ascending N	Going Straight	Car	339.620	112
C Injury Accident	5/31/2007		•	0					None	None		In Intersection	Same 5 Direction Turning	Descending S	Turning Left	Pickup/Van/ Panel/SUV	339.620	111
C Injury Accident	5/31/2007	Thursday	4	•	Day	Dry	Clear	None	None	Inattention	On Roadway	In Intersection		Descending S	Passing	Pickup/Van/ Panel/SUV	339.620	Ë
Property Dmg Report	10/27/2011		•	0					None	Improper Overtaking	Roadside or Sidewalk	Nonjunction	Same Direction Turning	Ascending	Passing	Pickup	339.600	110
Property Dmg Report	10/27/2011	Thursday	•	•	Day	Dry	Clear	None	None	Failed to Signal		Nonjunction		Ascending N	Turning Left	SUV/Crossov er	339.600	110
Property Dmg Report	1/8/2011	Saturday	•	0	Day	lce	Clear	None	None	Speed Too Fast For Conditions	Right Shoulder	Nonjunction	Overturn	Descending S	Going Straight	Pickup	339.500	109
Property Dmg Report	8/15/2009	Saturday	•	•	Dark, Street Lights Off	Dry	Cloudy	None	None	Other	On Roadway	Nonjunction	Animal - Wild	Descending N	ight	Pickup/Van/ Panel/SUV	339.500	108
Property Dmg Report	12/21/2008	Sunday	•	•	Dark, No Street	Snow	Snow	None	None	None	Roadside or Sidewalk	Nonjunction	Overturn	Descending N	'n	Pickup/Van/ Panel/SUV	339.500	107
Property Dmg Report	1/14/2008	Monday	•	•	Dark, No Street Lights	Snow	Snow	None	Speed Too Fast For Conditions	None	Roadside or Sidewalk	Nonjunction	Embankment	Ascending S	Negotiating /	ŝ	339.500	106
Property Dmg Report	11/19/2005	Saturday	•	0	Dark, No Street Lights	Dry	Clear	None	None	None	On Roadway	Nonjunction	Animal - Wild	Ascending N	Going Straight	Car	339.500	105
C Injury Accident	8/4/2005	Thursday	4	0	Dawn or Dusk	Dry	Clear	Loose Gravel/Seal Coat	None	None	Left Shoulder	Nonjunction	Overtum	Ascending S	Negotiating /	Car	339.500	104
B Injury Accident	10/16/2004	Saturday	4	0	Day	Wet	Cloudy	None	None	Speed Too Fast For Conditions	Right Shoulder	Nonjunction	Ditch	Descending S	ight	Pickup/Van/ Panel/SUV	339,400	103
Property Drng Repart	5/10/2005	Tuesday	•	•	Day	Wet	Cloudy	None	Other Vehicle Defect	Drove Left of Center	Left Shoulder	Nonjunction	Overturn	Descending N	Negotiating D	Car	339.400	102
Severity	AccidentDate	Day /	njurles	Fatalities Injuries	Light	Surface	Weather	Road Condition	Contributing Circumstance 2		Event Relation To Road	Junction	Event 1	Lane	Driver Action	Vehicle Type	Milepost	*

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341.112	341.100	341.100	341.100	341.100	341.100	341.100	341.100	341.046	341.023	341.009	341.009	341.009	341.009	341.009	341.009	341.009	341.009	341.001	341.000	340,996	340.994	340.981	340.981	340.976	340.955	340.900	340.900	340,900	340.900	340.900	340.900	Milepost
Pickup/Van/ Panel/SUV	Car	Pickup/Van/ Panel/SUV		Pickup/Van/ Panel/SUV		Pickup/Van/ Panel/SUV	Pickup/Van/ Panel/SUV	Pickup/Van/ Panel/SUV	Pickup/Van/ Panel/SUV	SUV/Crossov er	Pickup/Van/ Panel/SUV	Car	Pickup/Van/ Panel/SUV	Car	Pickup/Van/ Panel/SUV	Pickup/Van/ Panel/SUV	Car	Car	Car	Car	Pickup/Van/ Panel/SUV	Pickup/Van/ Panel/SUV	Pickup/Van/ Panel/SUV	Car	Pickup/Van/ Panel/SUV	Car	SUV/Crossov er	Tractor - 1 Trailer	Pickup/Van/ Panel/SUV	Car	Pickup/Van/ Panel/SUV	Vehicle Type
Negotiating Curve	Going Straight	Going Straight	Negotiating Curve	Going Straight	Negotiating Curve	Going Straight	Going Straight	Goi	Negotiating Curve	R	Ne	Negotiating Curve	Parked Vehicle	Going Straight	Turning Left	Turning Left	Going Straight	Negotiating Curve	Negotiating Curve	Negotiating Curve	Negotiating Curve	Going Straight	Negotiating Curve	Negotiating	Negotiating	Negotiating Curve	Going Straight	Going Straight	Negotiating	Negotiating Curve	Negotiating Curve	Driver Action
Ascending	Descending	Descending	Ascending	Ascending	Ascending	Ascending	Ascending	Descending	Descending	Ascending	Ascending	Descending	Descending	Descending	Descending	Descending	Descending	Ascending	Descending	Ascending	Ascending	Ascending	Descending	Ascending	Descending	Ascending	Descending	Descending	Descending	Ascending	Ascending	Lane Direction
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Overturn	Head-On	Head-On	Tree	Tree	Embankment	Embankment	Overturn	Overturn	Embankment	Overturn	Overturn	Parked Car	Parked Car	Rear-End	Rear-End	Rear-End	Rear-End	Embankment	Delineator Post	Ditch	Animal - Wild	Overturn	Overturn	Tree	Embankment	Embankment	Overturn	Overturn	Animal - Wild	Embankment	Overturn	Event 1
Nonjunction	Nonjunction	Nonjunction	Nonjunction	Nonjunction	Nonjunction	Nonjunction	Nonjunction	Intersection Related	Nonjunction	Nonjunction	Nonjunction	Intersection Related	Intersection Related	Intersection Related	Intersection Related	In Intersection	Intersection Related	Nonjunction	Nonjunction	Nonjunction	Nonjunction	Nonjunction	Nonjunction	Nonjunction	Nonjunction	Nonjunction	Nonjunction	Nonjunction	Nonjunction	Nonjunction	Nonjunction	Junction
Outside Right-Of- Way		On Roadway	Roadside or Sidewalk	Left Shoulder	Right Shoulder	Roadside or Sidewalk	Left Shoulder	Right Shoulder	Roadside or Sidewalk	Roadside or Sidewalk	Right Shoulder	On Roadway		On Roadway			On Roadway	Left Shoulder	Left Shoulder	Off Roadway- Location Unknown	On Roadway	Roadside or Sidewalk	Outside Right-Of- Way	Left Shoulder	Left Shoulder	Left Shoulder	Roadside or Sidewalk	Right Shoulder	On Roadway	Left Shoulder	Left Shoulder	Event Relation To Road
f- Speed Too Fast For Conditions	None	Speed Too Fast For Conditions	Speed Too Fast For Conditions	None	None	None	Speed Too Fast For Conditions	Following Too Clase	Distracted IN or ON Vehicle	Speed Too Fast For Conditions	Speed Too Fast For Conditions	None	Drove Left of Center	Vision Obstruction	None	None	None	Alcohol Impaired	Alcohol Impaired	Other	None	Speed Too Fast For Conditions		Exceeded Posted Speed	Overcorrected	Alcohol Impaired	Drug Impaired	Other	None	None	Overcorrected	Contributing
None	None	or Drove Left of Center	None	Speed Too Fast For Conditions	None	Inattention	None	None	Inattention	Failed to Maintain	None	None	None	None	None	None	Following Too Close		Speed Too Fast For Conditions	None	None	None	None	Alcohol Impaired	Drove Left of Center	None	Asleep, Drowsy, Fatigued	Inattention	None	None	Drove Left of Center	Contributing Circumstance 2
None		None		Other	None	None	None	None	None	None	None		None		None		None	None	None	None	None	None	None	None	None	None	None	None	None	None	Loose Gravel/Seal Coat	Road Condition
Cloudy		Snow	Cloudy	Snow	Cloudy	Clear	Sieet/Hail	Cloudy	Clear	Snow	Snow		Snow		Cloudy		Clear	Rain	Clear	Rain	Clear	Cloudy	Snow	Cloudy	Cloudy	Clear	Clear	Cloudy	Cloudy	Fog	Fog	Weather
Ĩc		Snow	ice.	ice i	Ice	Ice	ice.	Dry	Dry	Snow	lce		Snow		Wet		Dry	Wet	Ice	Wet	Dry	lce	Slush	Dry	ĪCe	Dγ	Wet	Dγ	Dry	Ice	<u>e</u>	Surface
Dark, No Street Lights		Dark, No Street Lights	Dawn or Dusk	Dark, No Street Lights	Dark, Street Lights Off	Dawn or Dusk	Dark, Street Lights Off	Day	Day	Dawn or Dusk	Dark, No Street		Dark, No Street Lights		Dark, No Street Lights		Day	Dark, No Street Lights	Dark, No Street	Day	Day	Day	Dark, No Street Lights	Dawn or Dusk	Day	Dark, No Street Lights	Day	Dark, No Street Lights	Day	Day	Day	Light
0	0	0	0	•	0	0	0	0	•	•	0	•	0	0	0	0	0	0	0	0	0	٥	0	0	0	•	0	0	0	0	0	Fatalities Injuries
1		ω	1	•	1	1	0	0	1	•	•	0	0	•	ч	0	2	1	•	0	0	0	0	1	2	0	2	ч	•	•	•	Injuries
Monday		Sunday	Sunday	Saturday	Friday	Wednesday	Thursday	Friday	Friday	Tuesday	Saturday		Saturday		Sunday		Monday	Saturday	Saturday	Sunday	Monday	Friday	Monday	Wednesday	Tuesday	Saturday	Thursday	Monday	Monday	Thursday	Sunday	Day
11/27/2006	11/21/2010	11/21/2010	1/3/2010	1/24/2009	4/4/2003	1/30/2008	12/21/2006	3/16/2007	9/11/2009	12/18/2012	1/24/2009	12/13/2008	12/13/2008	3/2/2003	3/2/2003	5/14/2007	5/14/2007	2/18/2012	12/8/2012	5/16/2004	7/17/2006	3/28/2008	2/3/2003	5/28/2008	11/25/2003	8/18/2012	12/15/2011	11/10/2008	4/14/2008	12/13/2007	12/4/2005	AccidentDate
A Injury Accident	B Injury Accident	B Injury Accident	B Injury Accident	Property Dmg Report	C Injury Accident	C Injury Accident	Property Dmg Report	Property Dmg Report	B Injury Accident	Property Drng Report	Property Dmg Report	Property Dmg Report	Property Dmg Report	C Injury Accident	C Injury Accident	A Injury Accident	A Injury Accident	A Injury Accident	Property Dmg Report	Property Dmg Report	Property Dmg Report	Property Dmg Report	Property Dmg Report	A Injury Accident	B Injury Accident	Property Dmg Report	A Injury Accident	C Injury Accident	Property Drng Report	Property Dmg Report	Property Dmg Report	Severity

9/11/2010 C Injury Accident	-	0	0					Inattention	Alcohol Impaired	On Roadway	Nonjunction	N Opposite	Descending		Car	341.847	197
9/11/2010	Saturday	1	•	Dark, No Street Lights	Dry	Clear	None	None	None		Nonjunction	S Side Swipe	Descending		Pickup/Van/ Panel/SUV	341.847	197
3/30/2012		0	0					None	None	On Roadway	Nonjunction	N Other Object	Ascending	-	Pickup	341.800	196
3/30/2012	Friday	0	0	Day	Wet	Severe Cross Winds	None	None	Other	On Roadway	Nonjunction	S Cargo Loss/Shift	Ascending	Going Straight	Pickup	341.800	196
4/18/2008	Friday	1	0	Dark, No Street Lights	Dry	Clear	None	None	None	Right Shoulder	Nonjunction	S Overturn	Descending	Going Straight D	Pickup/Van/ Panel/SUV	341.800	195
10/4/2003 Property Dmg Report	Saturday	0	0	Dark, No Street Lights	Dry	Clear	None	None	Alcohol Impaired	Left Shoulder	Nonjunction	S Overturn	Ascending	Going Straight	Pickup/Van/ Panel/SUV	341.800	194
6/10/2003 Property Dmg Report	Tuesday	•	0	Dark, No Street Lights	Dry	Cloudy	None	None	None	On Roadway	Nonjunction	S Animal - Wild	Descending	Going Straight D	Pickup/Van/ Panel/SUV	341.800	193
5/2/2007	Wednesday	ч	٩	Day	DIY	Clear	None	None	Asleep, Drowsy, Fatigued	Left Shoulder	Nonjunction	N Overturn	Descending	Going Straight D	Car	341.800	192
10/30/2004	Saturday	4	0	Day	Wet	Cloudy	None	Speed Too Fast For Conditions	Inattention	Outside Right-Of- Way	Nonjunction	N Overturn	Ascending	Negotiating Curve	Car	341.700	191
3/2/2012 B Injury Accident	Friday	1	0	Day	Ice	Cloudy	None	Inattention	Speed Too Fast For Conditions	Right Shoulder	Nonjunction	S Overturn	Descending	Passing D	SUV/Crossov er	341.500	190
3/18/2004 Property Dmg Report		0	0					None	Other	On Roadway	Nonjunction	S Other Object Not Fixed	Ascending	Going Straight	Pickup/Van/ Panel/SUV	341.500	189
3/18/2004 Property Dmg Report	Thursday	0	0	Day	Dry	Severe Cross Winds	None	None	None	On Roadway	Nonjunction	N Other Object Not Fixed	Ascending	Going Straight A	Pickup/Van/ Panel/SUV	341.500	189
3/10/2009 Property Dmg Report		0	0					None	None		Nonjunction	S Head-On Turning	Descending	Negotiating D Curve	Pickup/Van/ Panel/SUV	341.481	188
3/10/2009 Property Dmg Report	Tuesday	0	0	Day	Slush	Clear	None	None	Inattention	On Roadway	Nonjunction	N Head-On Turning	Descending	Turning Left D	Car	341,481	188
7/7/2005 Property Drng Report	Thursday	0	0	Dawn or Dusk	Dry	Clear	None	None	Speed Too Fast For Conditions	Left Shoulder	Nonjunction	S Ditch	Ascending	Going Straight 🛛 A	Car	341.400	187
1/23/2009 Property Dmg Report		0	0					None	None	On Roadway	Nonjunction	N Animal - Wild	Descending	Going Straight D	Car	341.335	186
1/23/2009 Property Dmg Report	Friday	o	0	Dark, No Street Lights	Wet	Cloudy	None	None	None	On Roadway	Nonjunction	S Animal - Wild	Descending	Going Straight D	Car	341.335	186
11/22/2010 B Injury Accident		0	•					None	Following Top Close	On Roadway	Nonjunction	S Rear-End	Descending	Going Straight D	Pickup/Van/ Panel/SUV	341.332	185
11/22/2010 B Injury Accident		0	0					None	None	On Roadway	Nonjunction	S Side Swipe Opposite	Descending	Going Straight D	Pickup/Van/ Panel/SUV	341.332	185
11/22/2010 B Injury Accident	Monday	2	0	Dark, No Street Lights	Ice	Cloudy	None	Following Too Close	Vision Obstruction		Nonjunction	N Side Swipe Opposite	Descending	Going Straight D	Car	341.332	185
7/8/2003 C Injury Accident		0	o					None	None		In Intersection	N Rear-End	Ascending	Slowing in A	Truck - 3+ Axle	341.317	184
7/8/2003 C Injury Accident	Tuesday	1	0	Day	Dıy	Clear	None	None	None	On Roadway	In Intersection	N Rear-End	Ascending	Passing A	Car	341.317	184
10/4/2007 C Injury Accident		0	0					None	Drove Left of Center	On Roadway	In Intersection	E Angle	Ascending	Turning Right A	Pickup/Van/ Panel/SUV	341.317	183
10/4/2007 C injury Accident		0	0					None	None		In Intersection	4 Angle	Ascending N	Going Straight A	Car	341.317	183
10/4/2007 C Injury Accident	Thursday	ω	o	Dark, No Street Lights	Dıy	Clear	None	None	None		In Intersection	Angle	Ascending N	Going Straight A	Car	341.317	183
10/22/2005 B Injury Accident	Saturday	1	0	Dark, No Street Lights	υγ	Clear	None	None	None	Right Shoulder	Intersection Related	Utility Pole	Descending S	Going Straight D	Pickup/Van/ Panel/SUV	341.317	182
12/24/2012 Property Dmg Report	Monday	0	0	Dark, No Street Lights	Ice	Blowing	None	Failed to Maintain	Speed Too Fast For Conditions	Roadside or Sidewalk	Nonjunction		Ascending N	Going Straight A		341.300	181
11/27/2010 Property Dmg Report	Saturday	Ģ	0	Day	Ice	Snow	None	None	Speed Too Fast For Conditions	Right Shoulder	Nonjunction	S Traffic Sign Support	Descending	ing Buli	Pickup/Van/ Panel/SUV	341.300	180
9/30/2011		0	0					None	None		Nonjunction	V Pedestrian	Descending W	Wałk/Ride with Traffic NO Bike Du Lane	Pedestrian	341.200	179
9/30/2011	Friday	0	1					None	None	On Roadway	Nonjunction	S Pedestrian	Descending	Going Straight Du	Pickup	341.200	179
6/30/2010 Property Dmg Report	Wednesday	٥	0	Day	Dry	Clear	None	None	None	On Roadway	Nonjunction	Animal - Wild	Descending S	Going Straight D	Car	341.200	178
AccidentDate	Day A	Injuries	Fatalities	Light	Surface	Weather	Road	Contributing Circumstance 2	Contributing Circumstance 1	Event Relation To Road	Junction	Event 1	Lane Direction	Driver Action	Vehicle Type	Milepost	7

1		0	0		-	-		None	None		Nonjunction	N Rear-End	Ascending	Going Straight	Gir	342.400	217
Monday		0	0	Lights	Snow	Snow	None	or Inattention	Speed Too Fast For Conditions	On Roadway	Nonjunction	N Rear-End	Ascending	Going Straight	Pickup	342.400	217
Monday			0	Dawn or Dusk	Ice	Cloudy	None	None	Speed Too Fast For Conditions	Roadside or Sidewalk	Nonjunction	N Overturn	Ascending	Going Straight	Pickup/Van/ Panel/SUV	342.400	216
Tuesday		0	•	Dawn or Dusk	Ice	Cloudy	None	None		Outside Right-Of- Way	Nonjunction	N Overturn	Ascending	Negotiating Curve	Pickup/Van/ Panel/SUV	342.317	215
Wednesday				Day	s Dry	Severe Cross Winds	None	None		Off Roadway- Location Unknown	Nanjunction	N Overturn	Ascending	Going Straight	Motorcycle	342.300	214
Friday		0	0	Dark, No Street Lights	Snow	Cloudy	None	Drove Left of Center	Speed Too Fast For Conditions	Roadside or Sidewalk	Nonjunction	S Ditch	Ascending	Going Straight	Pickup/Van/ Panel/SUV	342.200	213
Monday	· · · · · · · · · · · · · · · · · · ·	2	•	Day	Dıy	Clear	None	Overcorrected	Distracted IN or ON Vehicle	Off Roadway- Location Unknown	Nonjunction	s Overturn	Descending	Going Straight	Car	342.200	212
	1 -	0	0					Following Too Close	None	P Right Shoulder	At Driveway/Alley/P arking Lot	S Rear-End Turning	Descending	Going Straight	Pickup/Van/ Panel/SUV	342.200	Z11
Tuesday		0	0	Dark, No Street Lights	Wet	Cloudy	None	None	None		At Driveway/Alley/P arking Lot	S Rear-End Turning	Descending	Turning Right	Pickup/Van/ Panel/SUV	342.200	211
	1	0	•					None	Other	On Roadway	Nonjunction	S Cargo Loss/Shift	Ascending	Going Straight	Pickup/Van/ Panel/SUV	342.100	210
Saturday	Sat	•	-	Day	Dry	Clear	None	None	None	On Roadway	Nonjunction	N Other	Ascending	Going Straight	Pickup/Van/ Panel/SUV	342.100	210
	-	0	0					None	None	<u> </u>	Driveway/Alley/P arking Lot Related	N Overturn	Ascending	Going Straight	Pickup/Van/ Panel/SUV	342.100	209
		-	o			-		Inattention	None	P On Roadway	Driveway/Alley/P arking Lot Related	5 Head-On	Ascending	Going Straight	Pickup/Van/ Panel/SUV	342.100	209
ay	Friday	ω	0	Day	Dη	Clear	None	None	None	P On Roadway	Driveway/Alley/P arking Lot Related	S Rear-End	Ascending S	Stopped in Traffic	Pickup/Van/ Panel/SUV	342.100	209
		•	0					None	Νοπε		Driveway/Alley/P arking Lot Related	Rear-End Turning	Descending S	Stopped in Traffic	Pickup/Van/ Panel/SUV	342.030	208
, ,	Sunday	4	0	Day	Dry	Cloudy	None	None	Inattention	o On Roadway	Driveway/Alley/P arking Lot Related	Rear-End Turning	Descending S	Going Straight	Car	342.030	208
		0	0					Distracted IN or ON Vehicle	Inattention	On Roadway	Nonjunction	Rear-End	Descending S	ight	G	342.028	207
l I		0	0					None	None	On Roadway	Nonjunction	5 Rear-End	Descending S	Stopped in	SUV/Crossov er	342.028	207
	Friday	4	0	Day	Dry	Clear	None	None	None		Nonjunction	S Rear-End	Descending S	eft	Pickup	342.028	207
1	Thursday	•	0	Dark, No Street Lights	Dη	Clear	None	None	None	On Roadway	Nonjunction	Ą	Descending S	Negotiating	Pickup/Van/ Panel/SUV	342.000	206
		•	•					None	None	On Roadway	Nonjunction	Rear-End Turning	Descending S	Going Straight	Pickup/Van/ Panel/SUV	342.000	205
	Friday	-	0	Day	Wet	Rain	None	None	Failed to Signal	-	At Driveway/Alley/P arking Lot	Rear-End Turning	Descending S	eft	Саг	342.000	205
٩	Wednesday	0	0	Dark, No Street Lights	Dry	Clear	None	None	None	On Roadway	Nonjunction	Animal - Wild	Ascending N	'ng	Car	341.900	204
	Sunday	0	0	Day	Wet	Rain	None	None	None	Roadside or Sidewalk	Nonjunction	Embankment	Descending S	'ng	Pickup/Van/ Panel/SUV	341.900	203
	Friday	0	0	Dawn or Dusk	Ice	Clear	None	None	Speed Too Fast For Conditions	der	Nonjunction	Overturn	Descending S	Negotiating Curve	Pickup/Van/ Panel/SUV	341.900	202
<	Saturday	<u>ь</u>	0	Dark, No Street Lights	Dry	Clear	None	None	Asleep, Drowsy, Fatigued	Off Roadway- Location Unknown	Nonjunction	l Tree	Descending N		ភ្ន	341.900	201
2	Monday	0	0	Dark, No Street Lights	Dry	Clear	None	None	None	On Roadway	Nonjunction	Animal - Wild	Ascending N	Going Straight	Pickup/Van/ Panel/SUV	341.900	200
Å	Saturday	-	•	Dark, No Street Lights	Dry	Clear	None	None	Asleep, Drowsy, Fatigued	Left Shoulder	Nonjunction	Overturn	Ascending 5	ight	Car	341.900	199
		•	•					None	None		Nonjunction	Head-On	Descending S	Negotiating Curve	Car	341.899	198
<	Tuesday	ъ	1	Day	Dry	Clear	None	Drove Left of Center	Asleep, Drowsy, Fatigued	On Roadway	Nonjunction	I Head-On	Descending N		Car	341.899	198
	Day	Injuries	Fatalities Injuries	Light	Surface	Weather	Road Condition	Contributing Circumstance 2		Event Relation To Road	Junction	Event 1	Lane Direction	Driver Action	Vehicle Type	Milepost	*
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	Saturdav 5/	0	0	Day	Dry	Clear	None	Inattention	Failed to Yield	On Roadway	Nonjunction	Same S Direction Turning	Descending	U-Turn	Car	343.200	240
	4,	0	0					None	None		Driveway/Alley/P arking Lot Related	S Side Swipe Same		Turning Left	Car	343.100	239
	Tuesday 4/	0	0	Daγ	Wet	Cloudy	None	Failed to Yield	Inattention	o On Roadway	Driveway/Alley/P arking Lot Related	Side Swipe Same	Descending S	Going Straight	Pickup/Van/ Panel/SUV	343.100	239
-	Sunday 7/	1	0	Day	Dry	Clear	None	None	None	On Roadway	Nonjunction	Animal - Wild	Ascending N	Going Straight	Car	343.100	238
1/30/2003 Property Dmg Report	Thursday 1/	•	0	Dark, No Street Lights	Wet	Rain	None	None	None	On Roadway	Nonjunction	N Animal - Wild	Ascending N	Going Straight	Car	343.100	237
2/16/2006 Property Dmg Report	Thursday Z/	0	0	Dark, No Street Lights	Ice	Cloudy	None	None	None	Outside Right-Of- Way	Nonjunction		Descending S	Going Straight	Pickup/Van/ Panel/SUV	343.095	236
12/6/2011 Property Dmg Report	1	0	0				-	None	Inattention	On Roadway	Nonjunction	N Side Swipe Opposite	Descending N	ight	Tractor - 1 Trailer	343.007	235
12/6/2011 Property Dmg Report	1:	0	0					None	None		Nonjunction		Descending S	Stopped in Traffic	Car	343.007	235
12/6/2011 Property Dmg Report	Tuesday 12	0	0	Dark, No Street Lights	Dry	Clear	None	None	None		Nonjunction	Side Swipe	Descending S	Stopped in Traffic	Van - 1 to 8 seats	343.007	235
11/22/2010 Property Dmg Report	11	•	0					None	Failed to Yield	On Roadway	Intersection Related	E Angle Turning	Descending E	Turning Left	Pickup/Van/ Panel/SUV	343.000	234
11/22/2010 Property Dmg Report	Monday 11,	•	0	Dark, No Street Lights	lce	Snow	None	None	None		Intersection Related	S Angle Turning	Descending S	Going Straight	Car	343.000	234
2/10/2009 B Injury Accident	Tuesday 2/	14	0	Day	Dry	Clear	None	None	None	Left Shoulder	Nonjunction	S Embankment	Ascending S	Going Straight	Pickup/Van/ Panei/SUV	342.996	233
3/27/2004 Property Dmg Report	Saturday 3/	•	0	Dark, No Street Lights	Dıγ	Cloudy	None	None	Alcohol Impaired	Outside Right-Of- Way	Nonjunction	0	Descending S	Going Straight	ស្ន	342.968	232
4/11/2011 Property Dmg Report	4/	-	0					None	None		Nonjunction	Side Swipe	Descending S	Merging	Truck With Trailer	342.905	231
4/11/2011 Property Dmg Report	Monday 4/	•	0	Day	Wet	Rain	None	Failed to Yield	Inattention	On Roadway	Nonjunction	Side Swipe Same	Descending S	Merging	SUV/Crassov er	342.905	231
7/13/2005 Property Dmg Report	Wednesday 7/	0	0	Dark, No Street Lights	Dry	Clear	None	None	None	On Roadway	Nonjunction	b	Descending S	Going Straight	Car	342.857	230
10/17/2006 C Injury Accident	10	•	0					Other Vehicle Defect	Inattention	On Roadway	Nonjunction	N Rear-End	Ascending N	Going Straight	Car	342.801	229
10/17/2006 C Injury Accident	Tuesday 10,	1	•	Dark, No Street Lights	Dry	Clear	None	None	None		Nonjunction	N Rear-End	Ascending N	Going Straight	Car	342.801	523
1/6/2005 Property Dmg Report	Thursday 1,	•	0	Dark, No Street Lights	Snow	Snow	None	Speed Too Fast For Conditions	Tire Defect	Right Shoulder	Nonjunction	S Overturn	Descending S	Going Straight	Pickup/Van/ Panel/SUV	342.800	228
12/22/2007 Property Dmg Report	Saturday 12,	0	•	Dawn or Dusk	Snow	Snow	None	None	Other	Roadside or Sidewalk	Nonjunction	S Embankment	Descending S	Going Straight	Car	342.800	227
12/18/2012 C Injury Accident	Tuesday 12,	1	0	Dark, No Street Lights	Ge	Snow	High/Low Shoulder	r Failed to Maintain Lane	Speed Too Fast For Conditions	Left Shoulder	Nonjunction	N Overturn	Ascending N	light	SUV/Crossov er	342.700	226
1/23/2012 Property Dmg Report	Monday 1/	•	0	Day	Ice	Clear	None	None	Other	Private Property	Nonjunction	V Fence	Ascending N	Negotiating Curve	Pickup	342.700	225
9/1/2004 Property Dmg Report	Wednesday 9,	0	0	Dark, No Street	Dry	Clear	None	None	None	On Roadway	Nonjunction	5 Animal - Wild	Descending S	Going Straight	Car	342.700	224
2/10/2007 B Injury Accident	Saturday 2/	2	0	Dark, No Street Lights	Ice	Cloudy	None	Speed Too Fast For Conditions	Overcorrected	Left Shoulder	Nonjunction	VOverturn	Descending N	Negotiating Curve	Pickup/Van/ Panel/SUV	342.700	223
12/8/2012 Property Dmg Report	Saturday 12	0	0	Day	Slush	Cloudy	None	r Failed to Maintain	Speed Too Fast For Conditions	Right Shoulder	Nonjunction	5 Tree	Descending S	Passing	Car	342.600	222
12/22/2003 B Injury Accident	Monday 12,	1	0	Dark, No Street Lights	Īce	Clear	None	r Distracted IN or ON Vehicle	Speed Too Fast For Conditions	Roadside or Sidewalk	Nonjunction	Överturn	Descending	Negotiating Curve	Pickup/Van/ Panel/SUV	342.600	221
2/5/2011 Property Dmg Report	Saturday 2/	•	0	Dark, No Street Lights	Ice	Clear	None	None	Spec	Outside Right-Of- Way	Nonjunction	-	Ascending S	69	SUV/Crossov er	342.500	220
12/2/2007 Property Dmg Report	12	•	•					None	Speed Too Fast For Conditions	On Roadway	Nonjunction	Side Swipe Opposite	Ascending S	Avoiding Obstacle	Pickup/Van/ Panel/SUV	342.500	219
12/2/2007 Property Dmg Report	Sunday 12	0	0	Day	Snow	Rain	None	None	Vision Obstruction		Nonjunction		Ascending N	Going Straight	G	342.500	219
3/2/2007 Property Dmg Report	Friday 3,	0	0	Day	Dıy	Clear	None	Fatigued	Overcorrected	Roadside or Sidewalk	Nonjunction	Embankment	Ascending S	Going Straight	Gar	342.500	218
AccidentDate Severity	Day Acc	Injuries	Fatalities Injuries	Light	Surface	Weather	Road Condition	Contributing Circumstance 2	Contributing Circumstance 1	Event Relation To Road	Junction	Event 1	Lane		Vehicle Type Driver Action	Milepost	#

L						$\left \right $			Note	Indition		-	N Same	Ascending	Turning Left	Car	344.004	253
5 Property Dmg Report	8/24/2005		•	0					Nore	Instruction	-	Ξ	Side Swipe	_	-+-	Iraller		
5 Property Dmg Report	y 8/24/2005	Wednesday	0	0	Day	Dry	Clear	None	Inattention	Drove Left of Center	On Roadway	Intersection	N Side Swipe	Ascending	Turning Right	Truck With	344.004	253
<u> </u>	1/20/2012		0	0		+			None	None	On Roadway	g Nonjunction	N Angle Turning	Ascending	Going Straight	Car	343.985	252
	1/20/2012	Friday	0	•	Day	Slush	Cloudy	None	Speed Too Fast For Conditions	Failed to Yield		g Nonjunction	E Angle Turning	Ascending E	Turning Left	Car	343.985	252
-	1/11/2004	Sunday	•	0	Day	lce	Clear	None	-	Speed Too Fast For Conditions	Right Shoulder	Nonjunction	5 Overturn	Descending S	Going Straight	Pickup/Van/ Panel/SUV	343,981	251
	1/13/2012	Friday	-	0	Lights	Dıy	Clear	None	None	Vision Obstruction	Parking Lot Access Rd	Nonjunction	Ditch	Descending	Turning Left	Car	343.900	250
-	5/28/2004		0	0	Dark No Street	+			None	None		Driveway/Alley/P arking Lot Related	Side Swipe Same	Ascending	Stopped in Traffic	Car	343.800	249
4 Property Drng Report	5/28/2004		0	0	+	+-			None	None		Driveway/Alley/P arking Lot Related	J Side Swipe Same	Ascending N	Stopped in Traffic	Pickup/Van/ Panel/SUV	343.800	249
Property Dmg Report	5/28/2004	Friday	0	0	Day	Wet	Cloudy	None	None	Inattention	On Roadway	Driveway/Alley/P arking Lot Related	Side Swipe Same	Ascending N	Going Straight	Tractor - 1 Trailer	343.800	249
2 C Injury Accident	1/23/2012	Monday	4	•	Dark, No Street Lights	<u>.</u>	Clear	None	None	Speed Too Fast For Conditions	Roadside or Sidewalk	Nonjunction	Overturn	Ascending	Going Straight	Pickup	343.700	248
B Injury Accident	9/10/2006	Sunday	1	٥	Day	Div	Clear	None	None	Inattention	Outside Right-Of- Way	Nonjunction		Descending S	Going Straight D	Car	343.616	247
C Injury Accident	1/9/2010		0	0					Improper Lane Change	Improper Overtaking	On Roadway	Nonjunction	Sic	Descending S	Going Straight D	Pickup/Van/ Panel/SUV	343.500	246
C Injury Accident	1/9/2010	Saturday	1	0	Day	lce	Sleet/Hail	None	None	Speed Too Fast For Conditions		Nonjunction	Side Swipe Same	Descending S	Going Straight D	Car	343.500	246
	2/26/2003		0	0					Inattention	None	On Roadway	At Driveway/Alley/P arking Lot	Rear-End	Descending S	Going Straight D	Pickup/Van/ Panel/SUV	343.500	245
Property Dmg Report	2/26/2003	Wednesday	0	0	Day	Dry	Clear	None	None	None		Driveway/Alley/P arking Lot Related	Rear-End	Descending S	Turning Left D	Pickup/Van/ Panel/SUV	343.500	245
C Injury Accident	4007/CT /2	Apuns		•	Day	Snow	Cloudy	None	Overcorrected	Inattention	Sidewalk	Nonjunction	Utility Pale	Descending S	Going Straight D		343.481	244
+	10/30/2005	Sunday	1		Day	Dry	Cloudy	None	None	Other	On Roadway	Nonjunction	Overturn	Descending S	Going Straight D	Pickup/Van/ Panel/SUV	343.400	243
+	12/13/2012		0	•					None	None		Driveway/Alley/P arking Lot Related	Rear-End	Descending S	Slowing in Traffic	ស្ន	343.300	242
A Injury Accident	12/13/2012		0	•					None	None	On Roadway	Driveway/Alley/P arking Lot Related	Rear-End	Descending S	Slowing in Traffic De	Car	343.300	242
A Injury Accident	12/13/2012	Thursday	s	0	Dark, No Street Lights	Dry	Cloudy	None	Following Too Close	Inattention	On Roadway	Driveway/Alley/P arking Lot Related	Rear-End	Descending S	Going Straight De	Pickup	343.300	242
Property Dmg Report	5/10/2006		0	0					Following Too Close	None	On Roadway	At Driveway/Alley/P arking Lot	Rear-End	Descending S	Going Straight De	Pickup/Van/ Panel/SUV	343,300	241
Property Dmg Report	5/10/2006	Wednesday	•	0	Day	Dη	Clear	None	None	None		At Driveway/Alley/P arking Lot	Rear-End	Descending S	Turning Left De	Pickup/Van/ Panel/SUV	343.300	241
a Severity	Accidentinate	Uay	Injuries	Fatalities Injuries	Light	Surface	Weather	Condition	Circumstance 2	Circumstance 1	Road	Junction	Event 1	Lane	Driver Action D	Milepost Vehicle Type	Milepost	#
								Prad										

Injury Type	2003	2004	2005	2006	2007	2008	2009	2010	2011	2012	Total
Dead	2	0	0	1	0	0	0	1	2	0	6
Incapacitating	1	1	1	8	5	3	0	3	3	2	27
Non- Incapacitating	5	4	5	2	12	3	3	11	1	6	52
Possible	6	4	7	1	13	8	5	9	5	15	73
Unknown	0	0	0	0	0	1	0	0	0	0	1

Accident Injury Classification for US-95 between Thorncreek Road and Moscow (MP337.668 and 344.004) between 1/1/03 and 12/31/12

*From Data provided by ITD Headquarters Office of Highway Safety on Webcars.

Indext Chromatismer Constrained Name Analy input dip Name Constrained Name Constrained Name				-					None	None		Related	S Rear-End	Descending	Going Straight		326.300	
Image Conveniences			,	+ ,	LIBING	+	+		:			Intersection	Τ			Panel/SUV		
Next Conversion Conversion </td <td></td> <td></td> <td>0</td> <td></td> <td>Dark, No Street</td> <td></td> <td>Clear</td> <td>None</td> <td>Inattention</td> <td>None</td> <td>On Roadway</td> <td>Intersection</td> <td>S Rear-End</td> <td>Descending</td> <td>Going Straight</td> <td></td> <td>326.300</td> <td>26</td>			0		Dark, No Street		Clear	None	Inattention	None	On Roadway	Intersection	S Rear-End	Descending	Going Straight		326.300	26
Instant Conversions of the sector series of the sector series of the sector sect	+-		•						None	None	On Roadway	Nonjunction	E Head-On	Ascending	Going Straight	Pickup	326.111	
Not Chronic Stand 7 Condition 7			0	0	Day	Dry	Clear	None		Asleep, Drowsy, Fatigued	On Roadway	Nonjunction	W Overturn	Ascending	Going Straight	-	326.111	25
Next Crosswert Construct of Construct construct of Construct construct of Construct constr			0	•	:				None	None	On Roadway	Intersection Related	N Rear-End	Ascending	Going Straight	_	326.100	
Node Crometerio Crometerio Construction Number Algorithy inpaired Construction Space Explaine Space Explaine Space S			۰. ۱	0		<u> </u>	Cloud	None	Failed to Yield	Inattention		Intersection Related	N Overturn	Ascending	Turning Left	Pickup/Van/ Panel/SUV	326.100	24
Nome Commentation 2 Commentation 2 </td <td><u> </u></td> <td>+</td> <td>•</td> <td></td> <td></td> <td></td> <td>Cloud</td> <td>None</td> <td>Overcorrected</td> <td>Inattention</td> <td>Roadside or Sidewalk</td> <td>Nonjunction</td> <td>N Overturn</td> <td>Descending</td> <td>Negotiating</td> <td>ស្ន</td> <td>326.016</td> <td>23</td>	<u> </u>	+	•				Cloud	None	Overcorrected	Inattention	Roadside or Sidewalk	Nonjunction	N Overturn	Descending	Negotiating	ស្ន	326.016	23
Name Croumstance 1 Condition Winder Name Condition Winder Name According on the state			•		Dark, No Street	-		High/Lov Shoulde	None		Right Shoulder	Nonjunction	S Embankment	Descending	Going Straight	Car	326.000	22
Name Circumstance 1 Circumstance 2 Condition Windler Nume	-	+ -	•		_			None	None	Inattention	Left Shoulder	Nonjunction	N Jackknifed	Ascending	Negotiating Curve	Tractor - 2 Trailers	325.987	12
Name Circumstance 1 Circumstance 2 Condition Wather Summe Final Res Intrust Intrust <td></td> <td>+</td> <td>•</td> <td>0</td> <td></td> <td></td> <td>Snow</td> <td>None</td> <td></td> <td>Speed Too Fast Fe Conditions</td> <td>Roadside or Sidewalk</td> <td>Nonjunction</td> <td>N Traffic Sign Support</td> <td>Ascending</td> <td>Going Straight</td> <td>Car</td> <td>325.986</td> <td>20</td>		+	•	0			Snow	None		Speed Too Fast Fe Conditions	Roadside or Sidewalk	Nonjunction	N Traffic Sign Support	Ascending	Going Straight	Car	325.986	20
None Circumstance 1 Circumstance 2 Condition Weetler Suffree Light Failure Nume Nume <td></td> <td></td> <td>-</td> <td></td> <td>Dark, No Street Lights</td> <td>┢─</td> <td>Clear</td> <td>None</td> <td></td> <td>Speed Too Fast Fc Conditions</td> <td>Roadside or Sidewalk</td> <td>Nonjunction</td> <td>S Overturn</td> <td>Descending</td> <td>Negotiating Curve</td> <td>Car</td> <td>325.900</td> <td>19</td>			-		Dark, No Street Lights	┢─	Clear	None		Speed Too Fast Fc Conditions	Roadside or Sidewalk	Nonjunction	S Overturn	Descending	Negotiating Curve	Car	325.900	19
None Circumstance 1 Circumstance 2 Condition Weether Suffrage Light Failure 1 Unite 1 <thunit 1<="" th=""> Unite 1<td>-</td><td>-</td><td>1</td><td></td><td>Day</td><td></td><td>Cloud)</td><td>None</td><td></td><td>Alcohol Impaired</td><td>Roadside or Sidewalk</td><td>Nonjunction</td><td>S Overturn</td><td>Descending</td><td>Negotiating Curve</td><td>Gr</td><td>325.800</td><td>18</td></thunit>	-	-	1		Day		Cloud)	None		Alcohol Impaired	Roadside or Sidewalk	Nonjunction	S Overturn	Descending	Negotiating Curve	Gr	325.800	18
None Circumstance 1. Circumstance 2. Condition Weether Survee Failures Intention Failures Intention Failures Intention Pailures Intention Pailures	-	-	0	•	Day	Dıy	Clear	None	_	Failed to Maintai	Left Shoulder	Nonjunction	S Ditch	Ascending	Going Straight	G	325.500	17
Nand Circumstance 1 Circumstance 2 Condition Weather Surface Light Feature 5 Input e Sector 100	-		0		Dark, No Street	lce	Clear				Right Shoulder	Nonjunction	N Ditch	Ascending	Going Straight	Pickup	325.200	16
Nand Circumstance 1 Pathelie Failable Fa	-		D	0	Dawn or Dusk	-	Cloud)	None			Private Property	Nonjunction	N Fence	Ascending	Negotiating	Pickup	324.995	15
NaedCircumstance 1Circumstance 2ConditionWeatherSurfaceLightFatilitesInpuresDay	+	+	1		Dark, No Street Lights		Cloudy	None	Inattention	Asleep, Drowsy, Fatigued	Roadside or Sidewalk	Nonjunction	N Overturn	Ascending	Negotiating	Car	324.900	14
NoneCircumstance 1Circumstance 2ConditionWeatherSurfaceLightFatalitiesInjuriesJayAccommunesMedianNoneLipol ImpairedNoneCloudyDryDay01Monday $2/16/2009$ On RoadwaySpeed To Fast ForNoneNoneNoneSonwSnowDay00Wednasday $1/2/29/2010$ Ueft ShoulderDistracted IN OrNoneNoneNoneNoneCloudyDryDay00Wednasday $1/2/29/2010$ Ueft ShoulderDistracted IN OrNoneNoneNoneNoneCloudyDryDay00U $1/2/29/2010$ Ueft ShoulderDistracted IN OrNoneNoneNoneCloudyDryDay00U $1/2/29/2010$ NoneDistracted IN OrNoneNoneNoneCloudyDryDay001Monday $1/2/29/2010$ NoneDistracted IN OrNoneNoneNoneCloudyDryDay01Monday $1/2/29/2010$ NoneNoneNoneNoneNoneNoneCloudyDryDark, No Street00Saturday $1/2/2010$ NoneNoneNoneNoneNoneNoneCloudyDryDay01Wednasday $2/12/2010$ NoneNoneNoneNoneNoneNoneCloudyDryDay01		<u> </u>	0		Dark, No Street Lights	Dry	Clear	None	None	None	On Roadway	Nonjunction	N Animal - Wild	Ascending	Going Straight	Pickup/Van/ Panel/SUV	324.800	13
NoadCircumstance 1Circumstance 2ConditionWeatherStraceLightFatalitiesInpuresDayAccomentanceMedianNoneAlcohol ImpairedNoneCloudyDryDay01Monday $2/16/2009$ On RoadwaySpeed Too Fast ForNoneNoneSnowSnowDay00Wednasday $1/2/29/2010$ I eff. ShoulderDistracted IN orNoneNoneNoneNoneCloudyDryDay00Wednasday $1/2/29/2010$ I eff. ShoulderDistracted IN orNoneNoneNoneNoneCloudyDryDay01Monday $1/2/29/2010$ I eff. ShoulderDistracted IN orNoneNoneNoneCloudyDryDark, No Street001Monday $1/2/29/2010$ I eff. ShoulderSpeed Too Fast ForNoneNoneNoneCloudyDryDark, No Street00Saturday $1/2/29/2010$ I sidewalkConditionsNoneNoneNoneNoneCloudyDryDark, No Street00Saturday $1/2/29/2010$ I sidewalkSpeed Too Fast ForNoneNoneNoneCloudyDryDark, No Street00Friday $1/2/20209$ I eff. ShoulderSpeed Too Fast ForNoneNoneNoneCloudyDryDay01Wednesday $2/16/2011$ I sidewalkAlex, Dirotavi	-	-	0		Day		Cloudy	None		Speed Too Fast Fo Conditions	Median	Nonjunction	N Embankment	Ascending	Passing	Car	324.800	12
Read Circumstance 1 Circumstance 2 Condition Weather Surface User Fatalities Injuries Day Fatalities Injuries Day Accomentations Median None Accolorinpaired None Cloudy Dry Day 0 1 Monday $2/16/2009$ On Roadway Speed Too Fast For None None Snow Snow Day 0 0 Wednasday $1/29/2010$ On Roadway Speed Too Fast For None None Cloudy Dry Day 0 0 Wednasday $1/29/2010$ Unf Soulder Distracted II Nor None None Cloudy Dry Day 0 0 $1/2/29/2010$ None Distracted II Nor None None Cloudy Dry Day 0 1 Monday $1/2/29/2010$ Median None None Cloudy Dry Dark, No Street 0 0 Saturday $1/2/21/2008$ <t< td=""><td></td><td>1</td><td>0</td><td></td><td>Dark, No Street Lights</td><td>8</td><td>Fog</td><td>None</td><td></td><td></td><td>Right Shoulder</td><td>Nonjunction</td><td>N Traffic Sign Support</td><td>Ascending</td><td>Going Straight</td><td>Car</td><td>324.750</td><td>11</td></t<>		1	0		Dark, No Street Lights	8	Fog	None			Right Shoulder	Nonjunction	N Traffic Sign Support	Ascending	Going Straight	Car	324.750	11
RoadCircumstance 1Circumstance 2ConditionWeatherSurfaceLightFatalitiesInjuriesDayFatalitiesInjuriesDayFatalitiesInjuriesAccomentanceMedianNoneLiphtNoneAlcohol ImpairedNoneCloudyDayDay01Monday $2/16/2009$ On RoadwaySpeed To Fast For On RoadwaySpeed To Fast For On WehideNoneNoneNoneCloudyDayDay00Uednasday $1/2/29/2010$ On RoadwaySpeed To Fast For ON WehideNoneNoneCloudyDryDayDay01Monday $1/2/29/2010$ Ueft ShoulderDistracted IN or ON RoadwayNoneNoneCloudyDryDayDo1Monday $1/2/29/2010$ No RoadwaySpeed Too Fast For On RoadwayNoneNonePoor PavementSnowCloudyDryDark, No Street UghtsDDSaturday $1/2/13/2008$ No RoadwayNoneNoneNoneClearDryDark, No Street UghtsDOSaturday $7/2/2010$ No RoadwayNoneNoneNoneCloudyDryDark, No Street UghtsDOFriday $2/16/2011$ Roadide or SteretSpeed Too Fast For Roadide orNoneNoneCloudyDryDark, No Street Dark, No StreetDOFriday $2/16/2011$ Roadide or SteretSpeed Too Fast For <br< td=""><td></td><td></td><td></td><td></td><td></td><td>Snow</td><td>Snow</td><td></td><td></td><td>Inattention</td><td>Left Shoulder</td><td>Nonjunction</td><td>N Cross Median</td><td>Ascending</td><td>Changing Lanes</td><td>Саг</td><td>324.700</td><td>10</td></br<>						Snow	Snow			Inattention	Left Shoulder	Nonjunction	N Cross Median	Ascending	Changing Lanes	Саг	324.700	10
RoadCircumstance 1Circumstance 2ConditionWeatherSurfaceLightFatalitiesInpuriesDayCatalitiesInpuriesDayCatalitiesInpuriesDayCatalitiesInpuriesDayCatalitiesInpuriesDayCatalitiesConcentorereMedianToo Slow for TafficNoneAlcohol ImpairedNoneCloudyDnyDay01Monday2/16/2009On RoadwaySpeed To FafficNoneNoneSnowSnowDay00Used nasday1/2/29/2010On RoadwaySpeed Too Fast For ConditionsNoneNoneCloudyDnyDay001Monday10/11/2010On RoadwaySpeed Too Fast For ConditionsNoneNoneCloudyDnyDark, No Street00Sunday10/25/2009MedianSpeed Too Fast For ConditionsNonePoor PavementSonwIceDark, No Street00Saturday12/12/2010On RoadwayNoneNoneNoneCloudyDnyDark, No Street00Friday12/12/2010MedianSpeed Too Fast For ConditionsNoneNoneCloudyDnyDark, No Street00Saturday12/12/2010On RoadwayNoneNoneNoneNoneCloudyDnyDark, No Street00Friday12/12/2010SidewalkSpeed Too Fast For SidewalkNoneNone </td <td>+</td> <td><u> </u></td> <td>1</td> <td>0</td> <td>Day</td> <td></td> <td>Cloudy</td> <td>None</td> <td>None</td> <td>None</td> <td>Median</td> <td>Nonjunction</td> <td>s Overturn</td> <td>Descending</td> <td>Negotiating</td> <td>Pickup/Van/ Panel/SUV</td> <td>324.700</td> <td>9</td>	+	<u> </u>	1	0	Day		Cloudy	None	None	None	Median	Nonjunction	s Overturn	Descending	Negotiating	Pickup/Van/ Panel/SUV	324.700	9
Road Circumstance 1 Circumstance 2 Condition Weather Surface Light Fatalities Inpuries Paccenture Median None Alcohol Impaired None Cloudy Dry Day 0 1 Monday $2/16/2009$ Median Traffic None Snow Snow Day 0 0 Wednasday $2/16/2009$ On Roadway Speed Too Fast For None None Snow Day 0 0 Wednasday $1/29/2010$ On Roadway Speed Too Fast For None None Cloudy Dry Day 0 0 $1/29/2010$ Left Shoulder Distracted IN or Overcorrected None Cloudy Dry Day 0 1 Monday $10/11/2010$ None None None None Cloudy Dry Dark, No Street 0 0 Saturday $10/21/2010$ No Roadway None None None None			1	0	Day		Cloudy	None	Inattention	Asleep, Drowsy, Fatigued	Right Shoulder	Nonjunction	N Overturn	Ascending	Going Straight	Car	324.523	8
Road Circumstance 1 Circumstance 2 Condition Weather Stratee Light Fatalities Inpuries Day Accommunes <	-		0		Dark, No Street Lights	lce	Clear	None		Speed Too Fast Fo Conditions	Roadside or Sidewalk	Nonjunction	S Overturn	Descending	Negotiating	SUV/Crossov er	324.500	7
Road Circumstance 1 Circumstance 2 Condition Weather Stratee Light Fatalities Inpuries Day Accommune Accommu	-	-	0		Dark, No Street Lights	Dry		None	None	None	On Roadway	Nonjunction	N Animal - Wild	Ascending	Going Straight	Car	324.200	6
Road Circumstance 1 Circumstance 2 Condition Weather Surface Light Fatalities Injuries Day Accommuter Median None Alcohol Impaired None Cloudy Dry Day 0 1 Monday 2/16/2009 Median Too Slow for Taffic None None Cloudy Dry Day 0 0 Wednasday 2/16/2009 On Roadway Speed Too Fast For On Vehicle None Snow Snow Day 0 0 Wednasday 12/29/2010 Left Shoulder Distracted IN or ON Vehicle Overcorrected None Cloudy Dry Dark, NStreet 0 1 Monday 10/12/2009 On Roadway None None Cloudy Dry Dark, NStreet 0 1 10/12/2009	<u> </u>	<u> </u>	•	0	Dark, No Street Lights		<u> </u>	Poor Pavement Markings		Speed Too Fast Fo Conditions	Median	Nonjunction	N Overturn	Ascending	Going Straight	Pickup/Van/ Panel/SUV	324.200	σ
Road Circumstance 1 Circumstance 2 Condition Weather Surface Light Fatalities Injuries Day Accenture Median None Alcohol Impaired Wone Cloudy Dry Day 0 1 Monday 2/16/2009 Transfer None Snow Snow Snow Day 0 0 Weansaday 12/29/2010 On Roadway Speed Too Fast For On Vehicle None Snow Snow Day 0 0 12/29/2010 Left Shoulder Distracted IN on Overcorrected None Cloudy Dry Day 0 1 Monday 12/29/2010		+	•	•	Dark, No Street Lights	-	Cloudy	None	None	None	On Roadway	Nonjunction	N Animal - Wild	Ascending	Negotiating	Pickup/Van/ Panel/SUV	324.100	4
Road Circumstance 1 Circumstance 2 Condition Weather Surface Light Fatalities Injuries Day Accomputer Median None Alcohol Impaired None Cloudy Dry Day 0 1 Monday 2/16/2009 Too Slow for None Cloudy Snow Snow Day 0 0 Wednesday 12/29/2010 Tor Solw for None Snow Snow Day 0 0 Wednesday 12/29/2010 On Roadway Speed Too Fast For None 0 0 12/29/2010	-		-	0	Day	Dry	Cloudy	None	Overcorrected	Distracted IN or ON Vehicle	Left Shoulder	Nonjunction	N Ditch	Ascending	Avoiding	Car	324.010	ω
Road Circumstance 1 Circumstance 2 Condition Weather Surface Light Fatalities Injuries Day Accoentuate		12	•	0				-		Speed Too Fast Fo Conditions	On Roadway	Nonjunction	S Rear-End	Descending	Going Straight	Tractor - 1 Trailer	323.900	
Road Circumstance 1 Circumstance 2 Condition Weather Surface Light Fatalities Injuries Day Accedenciate Saturation	_		•	0	Day	Snow	Snow		None	Too Slow for Traffic		Nonjunction	S Rear-End	Descending	Going Straight	Саг	323.900	2
Circumstance 1 Circumstance 2 Condition Weather Surface Light Fatalities Injuries Day AccidentUate	-	<u> </u>		0	Day	Dry	Cloudy		Alcohol Impaired	None	Median	Nonjunction	S Overturn	gri	7			1
Road	identDate Severity		Injuries	Fatalities		Surface	Weather	Road Condition	Contributing Circumstance 2	Contributing Circumstance 1	Event Relation Tc Road	Junction F	Most Harmful Event	Lane Direction	Driver Action	Vehicle Type	Milepost	Accident #

All Crashes on US-95 Between the Lewiston Hill and Thorncreek Road (323.36 to 337.668) from 10/01/07 to 12/31/12

Total Crashes: 71 Total Fatalities: 0 Total Injuries: 37

002.200 20
Ascending N
Overturn Nonjunction
Off Roadway-
3/7/2009

B Injury Accident	7/16/2011		•	0					Failed to Yield	Inattention	On Roadway	In Intersection	S Non-Contact Unit	Descending	Turning Left	Car	337.180	
B Injury Accident	7/16/2011	Saturday	-	0	Day	Div	Cloudy	None	None	None	Roadside or Sidewalk	In Intersection	5 Overturn	Descending	Negotiating		337.180	71
C Injury Accident	1/7/2009	Wednesday	2	0	Lights	Wet	Cloudy	None	None	None	On Roadway	In Intersection	S Animal - Wild	Descending	Negotiating	Pickup/Van/ Panel/SUV	337.180	70
Property Dmg Report	10/21/2008			0					Inattention	None	On Roadway	In Intersection	Same Direction	Descending	Going Straight	Car	336.981	
Property Drng Report	10/21/2008	Tuesday	0	0	Day	Dry	Clear	None	None	None		In Intersection	Same Direction	Descending	Going Straight	Car	336.981	69
Property Dmg Report	1/9/2009	Friday	0	•	Lights	Dry	Clear	None	None	None	On Roadway	Nonjunction	5 Animal - Wild	Descending S	Going Straight	Pickup/Van/ Panel/SUV	336.600	68
Linheith mills inshare	6007 /c/c	Idesoay	c	c	Day	DIV	Cloudy	None	None	None	On Roadway	Nonjunction	5 Animal - Wild	Descending S	Going Straight	Car	335.300	67
C Injury Accident	12/9/2007	Sunday	ы ш	1	Lights	lœ	Snow	None	None	Speed Too Fast For Conditions	Roadside or Sidewalk	Nonjunction	V Overturn	Ascending N	Going Straight	Pickup/Van/ Panel/SUV	334.800	66
C Injury Accident	1/4/2010	Monday	-	•	Lights	Wet	Cloudy	None	None	Inattention	Outside Right-Of- Way	Nonjunction	4 Overturn	Ascending N	Going Straight	Car	334.732	65
B Injury Accident	10/1/2007	Monday	1	0	Day	Dry	Cloudy	None	Overcorrected	Inattention	Left Shoulder	in intersection	Overturn	Ascending N	Negotiating	Pickup/Van/ Panel/SUV	334.621	64
C Injury Accident	2/28/2011	Monday	2	0	Lights	lce	Snow	None	None	Speed Too Fast For Conditions	Outside Right-Of- Way	Nonjunction	Overturn	Descending N	Going Straight	Pickup	334.500	63
Property Dmg Report	8/7/2010	Saturday	0	0	Lights	Diy	Clear	None	None	None	On Roadway	Nonjunction	Animal - Wild	Ascending N	Going Straight	Pickup/Van/ Panel/SUV	334,500	62
Property Umg Report		Saturday	0	0	Lights	Snow	Cloudy	None	None	Speed Too Fast For Conditions	Median	Nonjunction	Overturn	Ascending N	Going Straight	Pickup	334.200	61
Property Drng Report	_		0	0	Dark No Street				None	None		In Intersection	Side Swipe Same	Descending S	Passing	Pickup/Van/ Panel/SUV	334.160	
Property Ding Report	12/5/2009	Saturday	•	0	Day	Snow	Cloudy	None	Improper Turn	Inattention	On Roadway	In Intersection	Side Swipe Same	Descending S	Turning Left	Car	334.160	60
Property Dmg Report		Friday	0	0	Lights	lce	Snow	None	None	Speed Too Fast For Conditions	Median	Nonjunction	Overturn	Descending S	Going Straight	Pickup/Van/ Panel/SUV	334.017	59
Property Dmg Report	9/30/2009	Wednesday	0	0	Dawn or Dusk	Ру	Cloudy	None	None	Vision Obstruction	On Roadway	Nonjunction	Animal - Wild	Descending S	Going Straight	Pickup/Van/ Panel/SUV	334.003	58
C Injury Accident	12/30/2010	Thursday	-	•	Day	Ice	Cloudy	None	None	Other	Roadside or Sidewalk	Nonjunction	0	Descending S	Going Straight	Pickup/Van/ Panel/SUV	333.400	57
Property Dmg Report	2/15/2011		0	0					None	None		Nonjunction	ŝ	Descending S	Going Straight	Pickup	333.200	
Property Dmg Report	2/15/2011	Tuesday	•	0	Day	Snow	Cloudy	None	None	Drove Left of Center	On Roadway	Nonjunction	Side Swipe Same	Descending S	Going Straight	Pickup	333.200	56

Appendix A.2

Official High Crash Location List for District 2

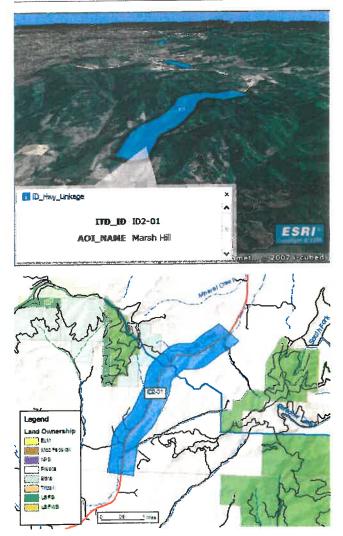


Idaho Transportation Department Office of Highway Safety Cluster Summary Non-Interstate District Report

	9			Cluster Summar	ry Non-Int	erstate Dis	strict Report			Rate
District	Rank	Route	Segment Code	& Milepost Range	Length	County	City	Frequenc Rank	Severit Rank	Multiplie Rank
<u>2</u>	8.5	95	001540	344.568 - 344.767	0.199	Latah	Moscow	24	35	41
	12	12	001910	33,325 - 33.825	0.500	Nez Perce		42.5	37	33
	17.5	95	001540	340.620 - 341.620	1.000	Latah		52	10	104
	28.5	95	001540	282,601 - 283,101	0,500	Lewis		98.5	28	83
	34	95	001540	337.668 - 339.620	1.952	Latah		60	41	106
	37	12	001910	123.508 - 127.008	1,500	Idaho		159	32	30
	38	8	001870	17.980 - 18.480	0.500	Latah		72.5	86	9
	65	95	001540	368,736 - 369,236	0.500	Latah		140.5	46	96
	69	95	001540	303.581 - 304.081	0.500	Nez Perce		38	121	53
	76	8	001870	9.812 - 10.312	0.500	Latah		98.5	58	139
	82	12	001910	21.640 - 22.640	1.000	Nez Perce		163.5	6	191.5
	83,5	3	001800	15,050 - 15,550	0.500	Latah		140.5	95	40
	101	12	001910	36.818 - 37.818	1.000	Nez Perce		98.5	104	128.5
	119	66	002530	.000992	0.992	Latah		221	100	54
	132	95	001540	349,863 - 351,863	2.000	Latah		85.5	129	172
	133.5	12	001910	30.825 - 31.825	1.000	Nez Perce		163.5	81	191.5
	133.5	12	001890	.503972	0.469	Nez Perce	Lewiston	84	119	195
	141	6	001840	100.550 - 101.050	0.500	Latah		193	168	15
	146	8	001870	7.942 - 8.800	0.858	Latah		122	108	214
	147	3	001800	26.439 - 26.842	0.403	Latah		158	169	59
	148	3	001800	16.550 - 17.050	0.500	Latah		193	148	67
	154	13	001960	11.269 - 11.769	0.500	Idaho		193	178	16
	155	95	001540	355.930 - 356.430	0.500	Latah		72.5	176	142
	158	95	001540	190.626 - 191.126	0.500	Idaho		193	115	150
	178.5	8	001870	5.800 - 6.300	0.500	Latah		140.5	133	221
	180	12	001910	18.450 - 18.950	0,500	Nez Perce		140.5	158	173
	189	95	001540	280.101 - 280.601	0.500	Lewis		140.5	187	140
	191	12	001910	99.508 - 100.508	1.000	Idaho		226	131	170.5
	200	95	001540	367.736 - 368.236	0.500	Latah		140.5	223.5	96
	202	162		5.427 - 6.427	1.000	Idaho		226	208	50
	203	12		46.893 - 47.393	0.500	Clearwater		140.5	218	117.5
	205	12		28.825 - 29.325	0.500	Nez Perce		140.5	209	165
	210	95		196.189 - 196.689	0.500	Idaho		193	189	156
	212	95		370,736 - 371,236	0.500) Latah		193	205.5	161.5
	225	12		138.008 - 139.008	1.000) Idaho		226		170.5
	226	6		2.748 - 3.248	0.500) Latah		193	213	178
	227	3		6.000 - 8.000	1,000) Latah	Juliaetta	218.5	211	202.5
	221	5		-						

Appendix B.1

Wildlife Crossing Areas on US-95 in Latah County Identified by Idaho Fish and Game



ITD2_ID: ID2-01

AOI_NAME: Marsh Hill

PRIORITY: Moderate

SPECIES: mule deer/ elk/ moose/ black bear/ small mammals

MIG_POP:

LOC_POP: Yes

SCALE:

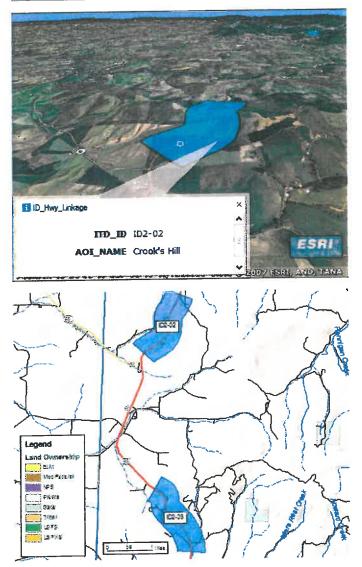
HWY_MORT:

SEASON: Spring, Summer, Fall, Winter

ATTRACT:

AGENCIES:

ADDITIONAL COMMENTS: Not a high kill area.Herd of elk by rest area.



ITD2_ID: ID2-02

AOI_NAME: Crook's Hill

PRIORITY: Low

SPECIES: mule deer/ elk/ moose/ small mammals

MIG_POP:

LOC_POP:

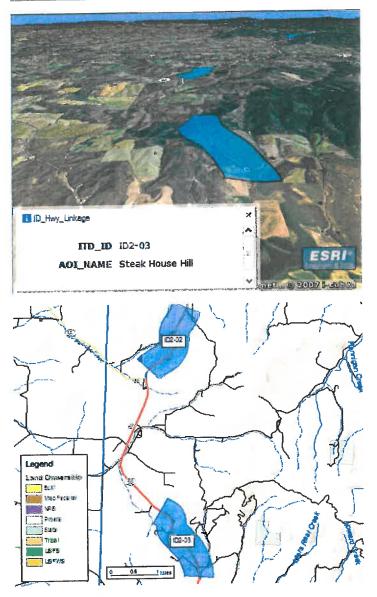
SCALE:

HWY_MORT:

SEASON:

ATTRACT:

AGENCIES:



ITD2_ID: ID2-03

AOI_NAME: Steak House Hill

PRIORITY: Moderate

SPECIES: mule deer/ elk/ moose/ small mammals

MIG_POP:

LOC_POP:

SCALE:

HWY_MORT:

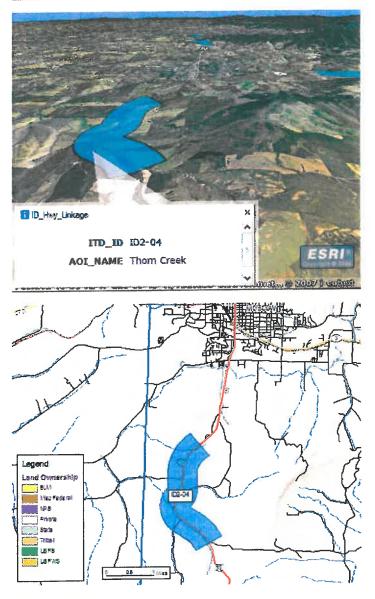
SEASON:

ATTRACT:

AGENCIES:

ADDITIONAL COMMENTS:

High kill area. Potential highway safety issue.



ITD2_ID: ID2-04

AOI_NAME: Thorn Creek

PRIORITY: Low

SPECIES: mule deer/ elk/ moose/ short-eared owls/ small mammals

MIG_POP:

LOC_POP:

SCALE:

HWY_MORT:

SEASON:

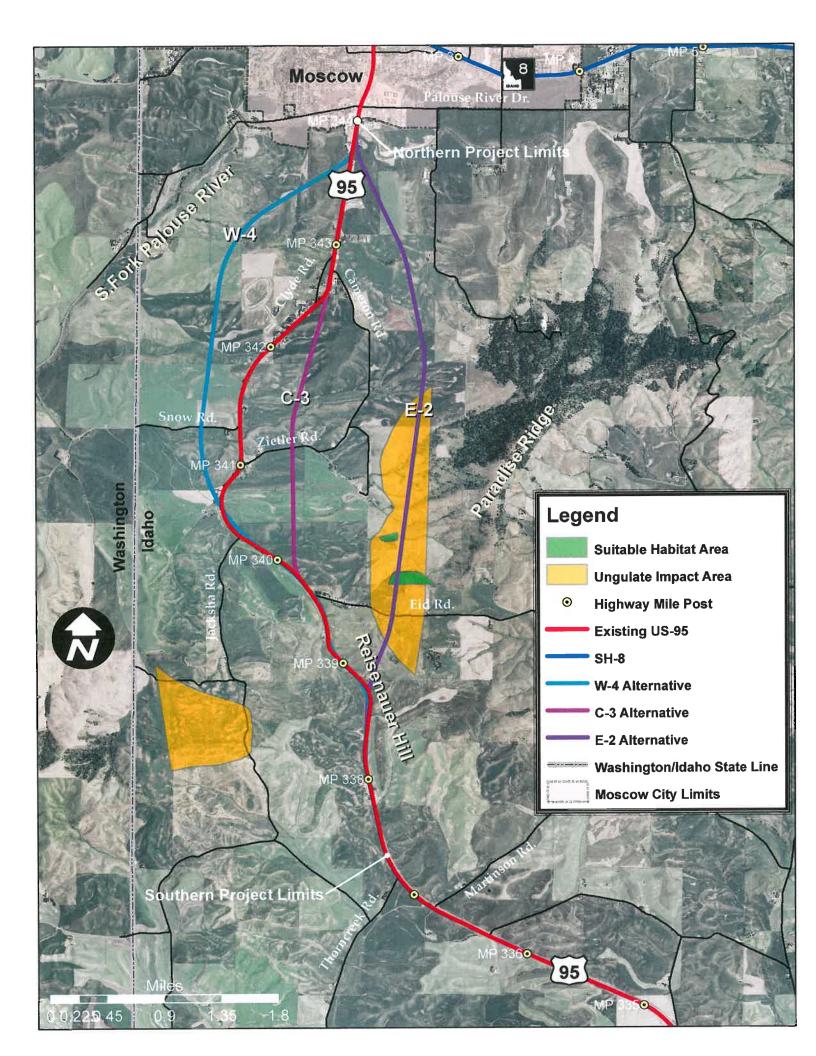
ATTRACT: water/riparian

AGENCIES:

ADDITIONAL COMMENTS:

Moose population increasing in this area. Private ponds act as an attractant. Plans to make hwy wider and relocate.

Appendix B.2 Ungulate Impact Area



Appendix B.3

Methods to Reduce Traffic Crashes Involving Deer: What Works and What Does Not

Methods to Reduce Traffic Crashes Involving Deer: What Works and What Does Not

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October 2003

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ABSTRACT

More than 1.5 million traffic crashes involving deer are estimated to occur each year in the United States. These crashes produce at least \$1.1 billion in vehicle damage and about 150 fatalities annually. Deer-related crashes are increasing as both deer populations and vehicular travel increase. Many methods have been used in attempts to reduce deer crashes, often with little scientific foundation and limited evaluation. This paper summarizes the methods and reviews the evidence of their effectiveness and the situations in which each may be useful. The only widely accepted method with solid evidence of effectiveness is well-designed and maintained fencing, combined with underpasses or overpasses as appropriate. Herd reduction is controversial but can be effective. Deer whistles appear useless. Roadside reflectors appear to have little long-term effect, although additional well-designed evaluations are needed before firm conclusions can be drawn. Both temporary passive signs and active signs appear promising in specific situations, but considerable research is required to evaluate long-term driver response and to improve and test deer detection technology for active signs. Other methods using advanced technology require substantial additional research and evaluation.

INTRODUCTION

Deer and motor vehicles do not share the nation's highways gracefully or safely. Although precise data are not available, the best estimates suggest that more than 1.5 million deer-vehicle crashes (DVCs) in the United States in 2002 produced at least \$1.1 billion in vehicle damage, about 150 human fatalities, and at least 1.5 million dead deer (Conover et al., 1995; DeerCrash, 2003; Williams, 2003a). These numbers are rising every year as both the number of deer and the amount of motor vehicle travel continue to increase.

Many methods have been proposed and implemented in attempts to reduce DVCs. Few have been documented or evaluated well. This summary reviews the methods and evidence of their effectiveness. For the methods with solid evidence we discuss conditions most appropriate for their use. For promising methods we suggest additional research. Finally, we provide data collection and reporting recommendations that, if implemented, will help to understand the DVC problem more clearly and evaluate DVC control methods more accurately.

Deer Population and Crash Trends

Deer inhabit all of the United States, including Hawaii, where they have escaped from captivity. White-tailed deer are common east of the Rocky Mountains, especially in northeastern, southeastern, and midwestern states; mule deer are found from the Rocky Mountains west, with smaller populations of black-tailed deer in some locations. In southern areas, white-tailed deer usually occupy fixed range areas year-round. In northern areas with deep snow, white-tailed deer may travel many miles between summer

ranges and winter deer yards. These movements depend somewhat on winter severity and spring greenup. Mule deer have regular migratory routes between summer and winter ranges.

Deer population totals are difficult to estimate, but there is abundant evidence that deer populations have increased over the past century. McCabe and McCabe (1997) estimated a North American white-tailed population of 24-33 million in 1500, before European settlement began, which dropped below 2 million by 1900 and then rose to 16-17 million by 1997. Other estimates placed the total U.S. deer population at 25-30 million by the end of the twentieth century; for example, Knapp (2001) estimated more than 27 million deer. Knox (1997) estimated that Virginia's deer population increased from about 25,000 in 1923 to about 900,000 in 1994.

Nationwide DVC counts also are difficult to estimate, but there is strong evidence that they are increasing. Most state crash data files record crashes with animals but do not distinguish deer from other animals such as moose, elk, horses, and cattle. The National Highway Traffic Safety Administration (NHTSA) Fatality Analysis Reporting System, a census of all fatal traffic crashes, shows an average of 154 fatal crashes involving animals in the four years 1998-2001, compared with an average of 111 in the four years 1992-95, an increase of 39 percent. NHTSA's General Estimates System estimates about 274,000 total police-reported crashes with animals annually in 2000-01 compared with 222,000 in 1992-93, an increase of 24 percent (Williams, 2003a). Data from states that distinguish deer from other animals suggest that most animal crashes involve deer: 99.7 percent in Michigan (Highway Safety Information System (HSIS), 1995), more than 90 percent in Minnesota (HSIS, 1995), and 93 percent in Pennsylvania (Williams, 2003a).

DVCs increased by 54 percent in Pennsylvania from 1994 to 2000 (Williams, 2003a), by 51 percent in Iowa from 1990 to 1997 (Hubbard et al., 2000), and by 69 percent in five states combined (Illinois, Maine, Michigan, Minnesota, and Utah) from 1985 to 1991 (HSIS, 1995). In 1999, 16 percent of all reported traffic crashes in Wisconsin were DVCs, up from 5 percent in 1978 (DVCR Working Group, 2000). The number of DVC claims at a major automobile insurance company rose 21 percent from 1998 to 2001 (Williams, 2003b).

Many DVCs are not reported to police. In a small telephone survey in New York, Decker et al. (1990) found that police were notified of about half, and insurance companies of less than half, of the DVCs. Taking the police underreporting into account, Conover et al. (1995) estimated that about 1.5 million DVCs occurred annually in the mid-1990s. The reported crashes alone produced more than \$1.1 billion in vehicle damage (in 1993 dollars); the unreported crashes added additional vehicle damage costs. More recently, an estimated 131,500 DVCs occurred in 2000 in the five upper midwest states of Illinois, Iowa, Michigan, Minnesota, and Wisconsin, producing 23 deaths, 4,650 injuries, and \$222 million in vehicle damage (DeerCrash, 2003).

DVCs are seasonal. White-tailed deer DVCs peak in October and November during the breeding season, with a secondary peak in May and June as yearling deer disperse from their birth ranges (Allen and McCullough, 1976 (Michigan data); Decker et al., 1990 (New York data); Puglisi et al., 1974 (Pennsylvania data); HSIS, 1995 (data for five states combined)). Mule deer DVCs are most frequent during the spring and fall migrations (Messmer et al., 2000). DVCs occur predominantly in darkness, on high-speed, two-lane, rural roads (HSIS, 1995; Williams, 2003a), especially when forest cover is close to the roadway (Finder et al., 1999).

Study Approach

We reviewed both published studies and other information obtained from highway safety, motor vehicle insurance, and natural resources sources. Three review studies were especially useful: Danielson and Hubbard (1998), DeerCrash (2003), and Putman (1997). The DeerCrash website (deercrash.com) contains an extensive bibliography and periodically updates summaries of information on specific methods. Studies involving animals other than deer were not reviewed systematically but were included when appropriate.

Three general strategies to reduce DVCs are to modify driver behavior, modify deer behavior, or reduce the number of deer. Each can be attempted in several ways. We summarize the theoretical basis and supporting evidence for each method and assess the available evaluation studies. We did not conduct a formal meta-analysis with specific criteria to define high-quality studies. Rather, we give more weight to methods with evidence from studies with sound designs, controls for potentially confounding influences, adequate sample sizes, and consideration of how the method's effectiveness may change over time.

METHODS TO AFFECT DRIVER BEHAVIOR

Three methods to affect driver behavior are to increase driver awareness of deer and the possibility of DVCs, improve the visibility of deer on or approaching roadways, and reduce driving speeds so drivers have more time to avoid crashes.

General Education

General education consists of efforts to provide information about DVC dangers so drivers will watch more carefully for deer and drive more slowly. Typical methods include news stories and public awareness campaigns in peak DVC seasons. About half the states use some form of general education (Romin and Bissonette, 1993; Sullivan and Messmer, 2003).

None of the general education campaigns has been evaluated. In other traffic safety areas such as impaired driving and occupant protection, stand-alone general education campaigns have not been effective in modifying driver behavior (O'Neill, 2001; Williams, 1994). Campaigns can be effective

when they present new information that directly affects drivers and that is reinforced by something drivers can observe. For example, publicity announcing increased enforcement of a safety belt use law can be effective when the publicity is followed with extensive law enforcement presence. It is unlikely that DVC general education is useful unless it provides information on very specific and time-sensitive situations, such as the beginning of mule deer migration across a short road segment. In these situations, either temporary passive or active signs may be more effective than general campaigns.

Signs

Roadside signs attempt to warn drivers of specific locations and even times when deer may be present. Passive signs have a fixed message at all times, though they may use lights or animation to attract attention. Active signs are lighted when deer are detected on or near the roadway.

Passive signs: Roadway signs warning drivers of deer-crossing locations are used in almost all states (Romin and Bissonette, 1993; Sullivan and Messmer, 2003). Most are passive: fixed signs in fixed locations, with the same message in words or pictures at all times and in all seasons, usually a standard yellow diamond sign with the figure of a deer, as specified in the *Manual of Uniform Traffic Control Devices*.

No studies have evaluated the effectiveness of standard deer warning signs in increasing driver awareness of deer, in reducing driving speeds, or in reducing DVCs. Because passive signs are used so frequently at locations where deer are present only occasionally, drivers probably ignore them (Putman 1997, Sullivan and Messmer, 2003).

Lighted and animated signs: Three methods have been used to attempt to increase the effect of deer warning signs. The first is to make the signs more visible with lights, flags, or even a lighted and animated figure of a deer. In a small study of lighted and animated signs, Pojar et al. (1975) found a slight effect on vehicle speeds but no effect on DVCs.

Temporary passive signs: The second method, used on roads crossed by mule deer migration corridors, installs or uncovers passive signs only during migration periods. Messmer et al. (2000) used large warning signs with battery-powered flashing amber lights at the ends of a two-mile and a four-mile roadway section, together with smaller flashing signs at each milepost within the two sections. Travel speeds during three migration periods when the signs were displayed and activated dropped about 8 mph from pre-migration levels, and DVCs dropped by 50 percent in the spring and 70 percent in the fall migration compared with three previous years. In a more extensive study of the same technique, using a more powerful research design, Sullivan et al. (preprint) placed similar temporary lighted signs on five roadway sections in three states with an adjacent section, separated by a buffer section, as a control. DVCs were about 50 percent lower in signed than in control sections across all sites. Vehicle speeds also were lower in signed sections.

Active signs: The final method uses signs that are activated only when deer are detected near the roadway. Detection methods include infrared light (in Minnesota), radar (Wyoming), laser (Washington), radio frequency beams parallel to the roadway (Indiana), and heat detection cameras (British Columbia). In Washington, radio collars have been attached to 8 elk in a herd of 80 near a segment of Highway 101. Flashing "elk warning" signs are activated when any of the collared elk come within one-quarter mile of the roadway (DeerCrash, 2003).

The only evaluation of these methods to date is a small study of a segment of U.S. 30 in Nugget Canyon, Wyoming (Gordon et al., 2001). An eight-foot fence was erected along both sides of the roadway, with a 300-foot gap through which migrating deer could cross. Two deer detection systems were used: infrared heat sensors, and geophones that detect ground vibrations combined with infrared light beams that detect motion across the beam. Both systems detected almost all deer (very few false negatives). The heat sensor system also was activated by birds and snow (more than 50 percent false positives), while the combined geophone and infrared system had no false positives. Vehicle speeds dropped by about 4 mph when the "deer on road when lights are flashing" sign was lighted, regardless of whether the sign was triggered by a deer, a false positive, or remotely by a researcher. DVC data were not collected, and it is unclear whether the observed speed reduction would be large enough to affect DVCs.

In summary, standard passive signs, although low-cost and low-maintenance, are unlikely to have any effect, though no evaluations substantiate this conclusion. The one study of lighted signs showed no effect on DVCs. Initial results are encouraging for temporary passive signs used in defined mule deer migratory corridors during migratory periods, which can vary from year to year. More testing is needed before the potential of active signs can be evaluated accurately. The two main issues are to refine detection technology to minimize false positives and false negatives and to determine the effects of these signs on driver behavior and DVCs.

Deer Visibility

The sooner a driver sees a deer on or approaching a roadway, the better the chance of avoiding a crash. Deer visibility can be improved through roadway lighting, roadside clearing, or methods to enhance drivers' nighttime vision.

Roadway lighting: Roadway lighting is commonly used to improve driver vision in urban areas, freeway interchanges, and other potentially dangerous locations. Because most DVCs occur at night, roadway lighting is an obvious potential countermeasure. In the only study of the effect of roadway lighting on DVCs, Reed and Woodard (1981) studied a single three-quarter-mile section in Colorado using a one week on/one week off design. The lighting did not affect overall deer crossings or driving speeds, and the study was too small to detect an effect on DVCs.

Roadway lighting is expensive. Only two states reported using lighting to control DVCs (Romin and Bissonette, 1996). It is unlikely to be useful except in very specialized situations.

Roadside clearing: A broad clear roadside area allows drivers to see deer that may enter the road and reduces forage that may attract deer close to the roadway. Finder et al. (1999) found that the most important landscape or topographical feature predicting high DVC sites in Illinois was the distance between the roadway and forest cover. In a study in Norway, Jaren et al. (1991) found that a clear 20-30 meter strip reduced crashes between railway trains and moose by more than 50 percent. Putman (1997) and Bruinderink and Hazebroek (1996) recommend reducing forage near the roadside. Roadside clearing raises many issues beyond DVC control, such as the costs of acquiring roadside right-of-way and of maintaining a clear area, the potential safety benefits if trees adjacent to the roadway are removed, and the aesthetics of cleared areas along secondary roads.

Infrared detection from vehicles: A potential long-term strategy to improve drivers' night vision is to equip vehicles with infrared technology that can detect deer and other heat-emitting objects and transmit information to drivers on heads-up displays. These systems have been introduced recently in Cadillacs (General Motors, 2000) and as aftermarket equipment for heavy trucks (Bendix, 2002), but their effects on DVCs have not been evaluated. Any strategy involving vehicle modifications requires many years to implement in the majority of the vehicle fleet.

Speed Limits

An approach often suggested to reduce traffic crashes in many situations is to attempt to reduce travel speeds through lower speed limits. Unfortunately, lower speed limits do not necessarily produce lower travel speeds (Transportation Research Board, 1998). The only study to evaluate the effects of speed limit changes on wildlife crashes involved short road segments in the highly regulated environment of Jasper National Park. Bertwistle (1999) compared sheep and elk crashes for eight years before and eight years after the speed limit was reduced from 90 to 70 km/h on three highway segments of 2.5 km, 4 km, and 9 km. He found that sheep crashes *increased* on these segments and decreased on adjoining segments where the speed limit remained at 90 km/h. Elk crashes increased on the speed-limit-reduction segments and increased more on the unchanged segments. No travel speed data were collected to measure the direct effect of the speed limit change. Bertwistle notes that differences in sheep and elk behavior likely explain the crash result differences.

Speed limit reductions together with deer warning signs may be useful in very specific locations with high deer populations or migration routes. However, unless speed limits are actively enforced, they are unlikely to affect travel speeds significantly, and perhaps not even then. Although seven states reported reducing speed limits in an attempt to control DVCs (Romin and Bissonette, 1996), the effects of these speed limit reductions have not been evaluated.

METHODS TO AFFECT DEER BEHAVIOR

Deer behavior management strategies attempt to either physically block deer from the roadway or make the roadway less attractive to deer by appealing to their senses of sight, sound, or smell.

Physical Control

Fencing: Fencing provides a physical barrier that attempts to prevent deer from entering the roadway. Every review of DVC control methods during the past 20 years has concluded that properly designed and maintained fencing, used together with appropriate underpasses, overpasses, and one-way deer gates, is the most effective method for reducing DVCs both in the United States (Danielson and Hubbard, 1998; Reed et al., 1979) and in Europe (Bruinderink and Hazebroek, 1996; Putman, 1997; Staines et al., 2001). State wildlife administrators agree, while state highway administrators rank fencing second to reducing deer herd size (Sullivan and Messmer, 2003). In 1992, 11 states had erected fencing to reduce DVCs (Romin and Bissonette, 1996). Crashes with moose were reduced by 80 percent after about 1,300 km of main roads in Sweden were fenced (Lavsund and Sandegren, 1991).

Aside from herd reduction, fencing is the only DVC method that unquestionably is effective if applied properly. Fencing that is sufficiently high, strong, long, and well-anchored with no gaps or tunnels will prevent deer from crossing a fenced road section. The issues with fencing involve the details and side effects.

- Physical characteristics: Fencing must be sufficiently high and long. Several studies have found 2.4 m (7.8 ft) fencing effective (Ward, 1982 (in Wyoming); Reed et al., 1982 (in Colorado); Ludwig and Bremicker, 1983 (in Minnesota)). White-tailed deer will jump a 2.2 m (7.4 ft) fence in search of food (Bellis and Graves, 1978). Fencing must extend far enough along a roadway to discourage deer from detouring around the ends of the fenced section. The necessary length depends on deer movement patterns. After one year's experience, Ward (1982) extended a fenced section from 6.7 to 7.8 miles and reduced end runs substantially. Electric fencing, currently being studied in Michigan, may provide an effective alternative to chain-link fencing (DVCR Working Group, 2000). Curtis et al. (1994) summarized the characteristics and effectiveness of various fencing types used to prevent deer from damaging crops.
- *Maintenance:* Regular checks are required to repair tunnels and breaks caused by erosion, animals, falling trees, and people. Deer regularly test a fence and are quick to pass through any breaks or gaps (Ward, 1982). Deer can crawl though openings less than 10 inches high under a fence (Bellis and Graves, 1978; Falk et al., 1978).
- *Effect on deer movements:* Fencing design should consider deer movement patterns and provide safe passage routes, as appropriate, through underpasses or other methods.

- Escape routes: Deer that manage to enter a fenced roadway need some way to escape. One-way
 gates have been found generally successful (Reed et al., 1974; Ward, 1982; Ludwig and
 Bremicker, 1983).
- *Costs:* Effective fencing is costly to construct and maintain. Iowa recently estimated construction costs for 8 ft chain-link fence on one side of a roadway at \$42,000 per mile (Danielson and Hubbard, 1998).
- *Other effects:* Roadway fencing or more substantial physical barriers may have other benefits such as reducing noise in adjacent properties or preventing pedestrian access to high-speed roads. Fencing and barriers may have positive or negative aesthetic implications.

Underpasses and overpasses: Deer underpasses, and more rarely used overpasses, allow deer to cross a roadway without encountering vehicles. Deer sometimes use underpasses or overpasses created when highways cross rivers or tunnel through ridges. Seven states report using underpasses specifically to allow deer crossings (Romin and Bissonette, 1996). Olbrich (1984) noted 824 under- and overpasses for animals on 823 km of federal highway in West Germany. To be effective, fencing or other barriers are required to channel deer to underpasses and overpasses.

Ward (1982) describes how a system of fencing and six underpasses was used along 7.8 miles of interstate highway crossing a mule deer migration route. The system did not disrupt deer movement and virtually eliminated DVCs. Other studies consider whether and how underpasses and overpasses are used rather than how they affect DVCs. Deer can be reluctant to use them, even when highly motivated to move along a migration route or to forage (Reed et al., 1975). Deer can remain wary or frightened even after several years of experience with the same underpass (Reed, 1981). Ward (1982) placed forage in underpasses to attract deer.

Factors affecting the use of underpasses and overpasses include their locations in relation to natural deer paths, size (wide openings and short lengths), design (earth floors), visual appearance (exit clearly visible from entrance, light walls and ceiling), and woody cover at the entrances (Danielson and Hubbard, 1998; Hartmann, 2003; Putman, 1997). In particular, some studies propose a minimum acceptable underpass "openness factor" of entrance area divided by underpass length (Putman, 1997).

Fencing and underpasses have been used to assist various species. Hartmann (2003) summarizes several case studies of underpass and overpass use by elk, bear, panther, mountain goats, and even salamanders. Singer and Doherty (1985) describe an underpass construction for mountain goats that directed almost all goats under rather than across the highway. Foster and Humphrey (1995) review other useful studies.

Underpasses and overpasses are expensive when included in original highway construction. Adding them to an existing highway is even more expensive. At-grade crosswalks: Crosswalks may provide a middle ground between a fully separated underpass or overpass and uncontrolled crossings marked only with signs. In the only study to date, Lehnert and Bissonette (1997) installed nine crosswalks on about 13 miles of two-lane and 4 miles of divided four-lane highways in Utah, with similar adjacent roads used as controls. At each crosswalk, fencing and landscaping directed deer to the crosswalk area. Because fencing was not permitted on the highway shoulder, the deer were channeled to the highway on a dirt path bordered by cobblestones. A similar path bordered by cobblestones crossed the divided highway's median strip. White painted cattleguard lines bounded the path across the highway surface. One-way gates in the fencing near the crosswalks allowed deer that moved beyond the crosswalk area to leave the roadway. Passive signs warned drivers to expect deer in the crosswalk areas.

The crosswalks appeared to decrease DVCs by about 40 percent, although the small sample size precluded any definitive conclusions. The crosswalk design of cobblestones and cattleguard stripes directed many, but not all, deer across the road as intended. Although drivers may have been more alert for deer at crosswalk areas, fewer than 5 percent responded to crosswalk signs by slowing down or turning on their high-beam headlights.

Crosswalks may be worth additional study to determine if design improvements can contain deer more effectively and if active signs that detect deer in the crosswalk area can improve driver awareness and actions.

Crosswalks, underpasses, and overpasses are more likely to be effective for western mule deer than eastern white-tails. Mule deer have defined migratory routes across highways, so DVCs are confined to relatively few locations where these expensive control methods can be justified. In contrast, white-tailed deer crashes occur throughout substantial lengths of two-lane, rural roads (Maine Department of Transportation, 2002). Further, DVCs occur most frequently in the fall breeding season, when antlered males are chasing females. At these times, crosswalks or other methods short of the complete physical control provided by substantial fences are unlikely to keep deer off the highway.

Sensory Control

Reflectors: Reflectors, used in Europe and some areas of the United States for more than 30 years, are the most contentious DVC control method. They have strong advocates, strong opponents, and conflicting results from more than 10 studies. The most commonly used and most frequently evaluated system, manufactured by Swareflex, consists of reflectors installed on posts at regular intervals along the roadway. Light from vehicle headlights is reflected to form a continuous "visual fence" of red, blue-green, or white light that deer are expected not to cross. Red reflectors form a visual barrier that humans cannot detect, so that it does not distract drivers. In 1992, 22 states reported using reflectors (Romin and Bissonette, 1997).

The basic behavioral questions about reflectors are whether deer can see light in the wavelengths used, whether deer are reluctant to cross such light beams, and whether deer become habituated to light beams over time. Zacks (1986) studied the effect of red and white light from Swareflex reflectors on penned white-tailed deer. He found no evidence that a beam of red or white light produced by reflectors from a static source, as opposed to a moving vehicle, affected deer behavior. Ujvari et al. (1998) exposed fallow deer in a large forested area to light from WEGU reflectors (a design similar to Swareflex) during a period of 15 nights. They found the proportion of deer that did not react to the reflected light increased over time: on the first night, 99 percent of the deer fled from low-intensity reflected light, while on the final three nights about 40 percent were completely indifferent to higher intensity light.

DeerCrash (2003) describes and summarizes 10 studies that attempt to evaluate the effect of roadside reflectors on DVCs using different study designs. The overall results are at best ambiguous.

- Four studies used designs that alternately cover and uncover the reflectors along a roadway segment. One found reflectors effective and three did not.
- Four studies used before/after designs. One found reflectors effective, one did not, and two had inconclusive results.
- Two studies used treatment/control designs. One found that reflectors were effective at some sites but not at others and the other study found no effect.

The best study in terms of its design, size, and power is Reeve and Anderson (1993), who used a cover/uncover design with control segments for three years on a 24.1 km segment of U.S. 30 in Wyoming that crosses a major mule deer migration route. They recorded 126 DVCs when the reflectors were uncovered, 64 when covered, and 147 on control segments. They concluded that the reflectors had no effect on DVCs.

Schafer and Penland (1985) provide the most positive site-specific evidence of effectiveness. They studied four roadway sections totaling 3.68 km in Washington during three years, in an area populated largely by white-tailed deer. They also used a cover/uncover design but with no control segments. They recorded 52 DVCs when reflectors were covered and only 6 when uncovered, concluding that the reflectors were highly effective.

Pafko and Kovach (1996) summarize results from a larger but less controlled application in Minnesota. Reflectors were installed at 16 road segments totaling 16.35 miles, four segments each in coniferous forest, prairie farmland, central hardwood, and metropolitan hardwood habitats. Average annual DVC counts on these segments for several years before and seven years after installation show 79 to 90 percent reductions in DVCs in the three rural habitats from pre-installation DVC averages of 98 to 214. In the metropolitan habitat, DVCs increased by 87 percent from a pre-installation average of 11.8.

These three examples illustrate the difficulties of drawing definitive conclusions from even the best studies. The very substantial reductions from high DVC totals found by Pafko and Kovach (1996)

suggest significant effects even though their simple before/after design does not control for other factors that may influence DVCs and their DVC counts may not be completely accurate. However, the authors note that estimated statewide deer populations were increasing during the study, DVCs did not decrease substantially on other roads, and the reductions appeared stable for several years. The increase in metropolitan areas may be due to small sample sizes, traffic volume increases, or reflector ineffectiveness on heavily traveled roads. Reeve and Anderson (1993) and Schafer and Penland (1985) reach very different conclusions from similar studies. Schafer and Penland had a considerably smaller study, with no control area, in an area populated largely by whitetails, while Reeve and Penland's study was on a mule deer migratory route.

If reflectors are effective, they offer obvious advantages. They are cheaper to install and maintain than physical barriers created with fencing and underpasses, though their cost is not insignificant — an estimated \$8,000 to \$10,000 per mile for installation (Danielson and Hubbard, 1998) plus annual maintenance to repair or replace damaged reflectors. Reflectors form a barrier only when vehicle headlights are present, so they allow deer to cross roads freely during daylight hours. However, the evaluations to date leave many questions unanswered. There appears to be no solid behavioral evidence that deer are reluctant to cross a light beam produced by reflectors. Do deer cross a beam at will, as suggested by Zacks (1986)? Do deer become habituated to such a beam, as found by Ujvari et al. (1998)? Are reflectors effective on high-volume roadways where there are few breaks in traffic to permit deer to cross? Are they effective on migratory routes or low-volume roads through established range areas where deer move freely?

Simple metal mirrors to reflect vehicle headlights as white light flashes also have been installed in a manner similar to reflectors. It appears that deer rapidly become accustomed to them, and they corrode quickly (Gilbert, 1982; Putman, 1997). Lavsund and Sandegren (1991) concluded from a large experiment that mirrors had no effect whatsoever on moose crashes in Sweden.

Flagging: An early attempt to influence deer behavior through sight was based on the observation that white-tailed deer raise their tails as a warning sign to other deer. Graves and Bellis (1978) placed rear-view silhouette models of deer with raised tails along a highway. These deer flag models did not affect deer movements (see also DeerCrash, 2003).

Whistles: Deer warning whistles have been available to the public for more than 20 years. A typical whistle is attached to a vehicle and produces ultrasonic noise in the range of 16-20 kHz when vehicle speed exceeds about 30 mph (DeerCrash, 2003). Whistles are based on the presumption that deer can hear and will be warned away from noise in this range. Twenty states reported using whistles in 1992 (Romin and Bissonette, 1997), although state wildlife agency and transportation department administrators ranked whistle effectiveness lowest of all common methods (Sullivan and Messmer, 2003).

Romin and Dalton (1992) conducted the only high-quality study of whistle effects. They drove past 150 groups of deer at distances up to 100 meters and a speed of 65 km/h, observing deer behavioral responses. Two common brands of whistles had no effect on deer behavior, even when deer were within 10 meters of the road. Romin and Dalton were unaware of any research demonstrating that deer are frightened by sound in the range produced by whistles. In a review of the effects of sound on animals and birds of many species, Bomford and O'Brien (1990) concluded that sounds of the type produced by whistles (steady noise rather than specific alarm or distress signals) may influence movements in the short term but that mammals and birds become accustomed to these sounds after long or frequent exposure.

Several less scientific reports and considerable anecdotal evidence either support or deny the effectiveness of whistles. For example, Cline (1989) reported on a one-year test of whistles attached to 42 Michigan State Police vehicles in five locations; 43 vehicles in five other locations served as controls. There were 14 DVCs involving police vehicles in the test locations and 5 in the control locations during the prior year; during the experimental year, there were 5 DVCs in each location. Based on these results, Cline concluded that the whistles were effective.

Roadside whistles, as opposed to vehicle-mounted whistles, are being tested in Saskatchewan (Beaupré, 2002). A series of noisemaking devices together with vehicle detection sensors was mounted along a 5 km section of highway. When the sensors detect a vehicle, the device warns deer with either sound or light signals.

In summary, there is no firm evidence that whistles are effective and considerable evidence that they are not. In the only high-quality study (Romin and Dalton, 1992), deer were not affected by whistles. It is unclear whether deer can hear whistles, whether whistle noise is covered by traffic noise, or whether deer become accustomed to whistle noise over time. In the absence of any solid studies that whistles are effective, they cannot be recommended.

Repellents: Chemical and biological substances attempt to repel deer in two ways. Contact repellents with unpleasant tastes applied to a food source seek to reduce or eliminate feeding. Area repellants with unpleasant smells, such as predator urine, seek to prevent deer from entering or crossing an area.

Several studies, summarized in El Hani and Conover (1995) and DeerCrash (2003), evaluated the effectiveness of various repellents on the feeding patterns of white-tailed and mule deer. Some repellents reduced feeding, but none completely stopped deer from feeding or entering an area. The studies also showed that deer habituate to repellents and will not be deterred by them if sufficiently hungry. No study in the United States has evaluated the effects of repellents in reducing DVCs, and repellents are not used systematically in any state to control DVCs (Romin and Bissonette, 1996). Putman (1997) reported that repellent "scent fences" have been studied in Germany, with mixed results. Early results from a repellent "odor fence" installed along 53 km of roadway in British Columbia, using posts and boxes every 0.25 km,

reportedly showed a 36 percent DVC reduction from the prior 10 years, and a test of four different repellents along 16 km of roadway on Vancouver Island began in 1999 (DVCR Working Group, 2000).

Repellents are most likely to hinder deer movements when applied in conjunction with fences or other physical barriers (Curtis et al., 1994). Jordan and Richmond (1992) demonstrated that an electric fence treated with repellents was more effective in deterring deer from feeding on apples than an electric fence alone, although repellent effectiveness decreased significantly after several weeks. The combination of repellents and fences has proved useful for home gardens and agricultural fields (Curtis et al., 1994) but would be expensive to install and maintain along highways.

Intercept feeding: In certain locations, deer regularly cross roadways to feed. Wood and Wolfe (1988) studied three such road sections in Utah for two years. On the treatment portion of each section, they established and maintained feeding stations more than 1,200 feet away from the roadway. They found lower DVCs in some, but not all, treatment areas. They noted that a feeding program has continuing costs, may make deer dependent on the food provided, and may attract more deer to the roadside. They concluded that intercept feeding may be useful only temporarily in specific situations.

Salt alternatives: Some authors suggest that deer may be attracted to roadways by salt applied to melt ice in the winter and that other deicing substances should be used instead (Feldhamer et al., 1986; DeerCrash, 2003). However, no studies have investigated the issue.

METHODS TO AFFECT DEER POPULATIONS

If there were no deer, or no deer near highways, there would be no DVCs. Deer herd reduction has long been considered an appropriate strategy for reducing DVCs as well as crop and garden losses caused by deer (DeNicola et al., 2000). State transportation department administrators rated herd management as potentially the most effective DVC control strategy, while state wildlife administrators rated it second only to fencing (Sullivan and Messmer, 2003).

The only herd reduction strategy that would completely eliminate DVCs would be to eliminate all deer, which the general public would not accept. Indeed, even in a high DVC area, only a minority of the public wished to reduce the deer population (Stout et al., 1993). In a survey of 10 randomly selected large metropolitan areas, 63 percent of respondents wanted no change in the number of deer in their neighborhoods, 27 percent wanted more deer, and only 10 percent wanted fewer deer (Conover, 1997).

Two reports document how local deer herd management policies can affect DVCs. In 1972, Princeton, New Jersey, passed a no-firearms-discharge ordinance. DVCs then increased by 436 percent in 10 years, from 33 in 1972 to 144 in 1982, compared with no statistically significant change in two adjoining townships where firearms hunting continued to be allowed (Kuser, 1995). Princeton then tried to reduce DVCs and other deer-related problems with deer whistles, reflectors, and increased bowhunting, but DVCs continued to rise, to 167 in 1991 and 227 in 1992. Irondequoit, New York, began a selective deer culling and bowhunting program in 1993. About 125 deer were removed in each of the next eight years. DVCs dropped from 227 in 1992 to about 100 annually in the late 1990s (Eckler, 2001).

Although herd reduction can be controversial, common sense and expert opinion agree that substantial and continued herd reductions will reduce DVCs (Danielson and Hubbard 1998; DVCR Working Group, 2000). But many questions remain, including the effectiveness of herd reductions over a large area on DVCs, the amount of herd reduction necessary to reduce DVCs substantially, how deer range and migration patterns influence the effect of herd reductions on DVCs, and how to design costeffective herd reduction programs (Brown et al., 2000). Wisconsin and other states are pursuing aggressive deer herd reduction programs (DVCR Working Group, 2000). Data from these programs may help address these questions.

SUMMARY AND CONCLUSIONS

Effective Methods with Solid Scientific Evidence

Fencing, combined with underpasses and overpasses as appropriate, is the only broadly accepted method that is theoretically sound and proven to be effective. Fencing is expensive to construct and maintain, and even the best fencing will not prevent all deer from entering a roadway.

Promising Methods Where More Information Is Needed

Herd reduction is unquestionably effective in reducing DVCs if the deer population in a specific area is reduced by a substantial amount. More research is needed on the minimum area needed for herd reduction to have a substantial effect and on the expected impact of a given amount of herd reduction on DVCs. A herd reduction strategy should be part of an overall wildlife management program that balances the costs and benefits of maintaining wildlife populations.

Roadside clearing may be effective, although there is very limited information supporting it. Roadside clearing must be part of a broader strategy of roadway design and maintenance.

Both temporary passive signs and active signs appear promising in specific situations, but considerable research is required to evaluate long-term driver response and to improve and test deer detection technology for active signs.

At-grade crossings for deer, perhaps combined with active signs, offer a long-shot chance at providing greater safety than uncontrolled crossings marked only with passive signs. At-grade crossings are most promising for highways crossing mule deer migration routes in western states.

Infrared driver vision technology in vehicles may be effective in the future. Its development and implementation will depend on its usefulness in improving driver night vision overall, not on its effect on DVCs.

Methods With Limited Demonstrated Effectiveness

Although reflectors have been studied fairly often, most studies were not designed or conducted well. The balance of the available evidence is that reflectors have little long-term effect, especially for white-tailed deer in suburban areas. Additional high-quality studies would be useful to investigate deer response and habituation to light beams and the effectiveness of reflectors when implemented.

Roadside lighting and intercept feeding may have limited effectiveness in specialized situations. Both methods are costly and have side effects that must be considered carefully.

Deer repellents can have limited effectiveness in modifying deer feeding and movement patterns. It is unlikely that repellents will be useful in roadway applications.

Methods that Appear Ineffective Based on Available Evidence

General education, passive signs, and lower speed limits appear ineffective in influencing driver behavior and reducing DVCs. The lack of good studies proving their ineffectiveness probably results from the unwillingness of funding organizations to allocate resources to study methods that are so unpromising.

Ineffective Methods with Evidence from Controlled or Experimental Situations

Deer whistles and deer flagging signs are not effective.

DISCUSSION AND RECOMMENDATIONS

Previous reviews of DVC control methods (Reed et al., 1979; Bruinderink and Hazebroek, 1996; Putman, 1997; Danielson and Hubbard, 1998; Staines et al., 2001) reached conclusions similar to ours, as did a review of moose-vehicle crashes in Sweden (Lavsund and Sandegren, 1991). There is no quick, cheap method to reduce DVCs. Fencing and herd reduction programs can be effective if they are designed and maintained well, but they are neither cheap nor quick.

DVC control must be part of an overall environmental strategy that balances the competing needs of humans and wildlife. For example, there is a trend in suburban areas to preserve or create green space and wildlife corridors (Houck, 1990). These areas must be carefully planned and coordinated by transportation, natural resource, and urban planning agencies to avoid attracting more deer and increasing DVCs.

Data Collection and Reporting

States should identify crashes involving deer on their state crash report forms and crash data files rather than aggregating crashes involving all animals. Without this, it is difficult to track DVC totals, trends, and patterns. States also should record precise DVC locations, as Maine does (Maine Department of Transportation, 2002), using GIS or other methods, to identify areas with high DVC frequencies. This

information is critical in deciding where fencing, herd reduction, active signs, or other DVC control methods are needed.

Research

Research is needed in the following areas.

- *Herd reduction:* minimum geographic area needed to be effective, effect of different amounts of herd reduction on DVCs in various settings
- Active signs: improved deer detection technology, long-term driver response
- Temporary passive signs and at-grade crossings: additional field trials under varying circumstances
- Reflectors: deer response and habituation, effect of reflector systems as implemented
- Intensive general education: effects of intensive driver awareness programs for DVCs in targeted communities
- Integrated DVC program: effects of coordinated program including signs, roadside clearing, and general education in specific high DVC locations
- Data: multi-state survey of DVC reporting to police, insurance companies, and wildlife agencies

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Appendix B.4

Roadside Clearing Crash Modification Factor

- Wild Animal Crash Data on US-20 From MP 369 to 375.5
 Between 7/1/2000 and 3/9/2013
 - Photographs of US-20 Before, During, and After the Tree Clearing Project During July 2010

Wild Animal Related Crashes on US-20 from MP 369 to 375.5 from 7/14/2000 to 8/11/2012

Total Crashes 38

Fatalities 0

Injuries 7

38	37	36	35	34	33	32	31	30	29	28	+	╀	┢	╎	╋	╈	╉	22	╉	-+	+	-		+	-	+	+	┽	+	╉	-	╉	+	╉	-+	+	2 36	1 36	# M
375.011	375.000	375.000	375.000	375.000	374.800	374.037	374.000	374.000	374.000	374.000	373.900	-	+	+	+-	╀	_	372.002	╇	-+	-	-	_	371.132	-+	_	371.000	370 998	370.500	-+	_		-+	-+	_	_	-	_	Milepost
Pickup/Van/Panel/SUV	Pickup/Van/Panel/SUV	Pickup/Van/Panel/SUV	Tractor - 1 Trailer	Motor Home	Pickup/Van/Panel/SUV	Motor Home	Car	Car	Pickup/Van/Panel/SUV	Car	Pickup/van/Panel/suv	Pickup/Van/Panel/SUV	Pickup/Van/Panel/SUV	PICKUP/ Vall/ Fallel/ SOV	Pickup/vaii/Paiiei/30v	Pickup/ Vari/Fariel/30V	hickup/varijtanci/suv	Dickup/van/Danel/SUV	Pickun/Van/Panel/SUV	Pickup/Van/Panel/SUV	Pickup/Van/Panel/SUV	Pickup/Van/Panel/SUV	Pickup/Van/Panel/SUV	Car	Tractor - 1 Trailer	Pickup/Van/Panel/SUV	Tractor - 1 Trailer	Car	Tractor - 1 Trailer	Tractor - 1 Trailer	Pickup/Van/Panel/SUV	Motorcycle	Pickup/Van/Panel/SUV	Pickup/Van/Panel/SUV	Car	Pickup/Van/Panel/SUV	Pickup/Van/Panel/SUV	Pickup/Van/Panel/SUV	Vehicle Type
Going Straight	Going Straight	Going Straight	Going Straight	Going Straight	Going Straight	Going Straight	Coing Straight	Going Straight	Coing Straight	Consocasht	Going Straight	Coing Straight	Going Straight	Going Straight	Going Straight	Going Straight	Going Straight	Going Straight	Going Straight	Going Straight	Going Straight	Going Straight	Going Straight	Going Straight	Going Straight	Going Straight	Going Straight	Going Straight	Going Straight	Going Straight	Going Straight	Going Straight	Going Straight	Going Straight	Driver Action				
Descending	Ascending	Ascending	Ascending	Descending	Ascending	Ascending	Ascending	Ascending	Ascending	Descending	Decrending	Ascending	Accending	Deconding	Ascending	Descending	Ascending	Descending	Ascending	Ascending	Descending	Descending	Descending	Descending	Ascending	Descending	Descending	Descending	Ascending	Descending	Ascending	Ascending	Ascending	Descending	Ascending	Descending	Descending	Ascending	Lane Direction
Animal - Wild	Animai - wild	Animai - Wild	Animai - Wild	Allitudi - vvild	Animal - Wild	Animal Wild	Animal - Wild	Animal - Wild	Animal - Wild	Animal - Wild	Animal - Wild	Animal - Wild	Animal - Wild	Animal - Wild	Animal - Wild	Animal - Wild	Animal - Wild	Animal - Wild	Animal - Wild	Animal - Wild	Animal - Wild	Animal - Wild	Animal - Wild	Animal - Wild	Animal - Wild	Animal - Wild	Animal - Wild	Animal - Wild	Animal - Wild	Animal - Wild	Animal - Wild	Animal - Wild	Animai - Wild	Animal - Wild	Event 1				
Nonjunction	Nonjunction	Nonjunction	Nonjunction	Nonjunction	Nonjulicuon	Nonjunction	Noniunction	Nonjunction	Noniunction	Noniunction	Noniunction	Noniunction	Noniunction	Noniunction	Noniunction	Noniunction	Nonjunction	Nonjunction	Nanjunction	Nonjunction	Nonjunction	Nonjunction	Nonjunction	Nonjunction	Nonjunction	Nonjunction	Nonjunction	Nonjunction	Nonjunction	Nonjunction	Nonjunction	Nonjunction	Nonjunction	Nonjunction	Nonjunction	Nonjunction	Nonjunction	Nonjunction	Intersection
On Roadway	Un Roadway	Un Roadway	Un Roadway	On Roadway	On Boodway	On Boodway	On Roadway	On Roadway	On Roadway	On Roadway	On Roadway	On Roadway	On Roadway	On Roadwav	On Roadway	On Roadway	On Roadway	On Roadway	On Roadway	On Roadway	On Roadway	On Roadway	On Roadway	On Roadway	On Roadway	On Roadway	On Roadway	On Roadway	On Roadway	On Roadway	On Roadway	On Roadway	On Roadway	On Roadway	On Roadway	On Roadway	On Roadway	On Roadway	Roadway
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Dain, no street abits	Dark No Street Lights	Dav	Dark. No Street Lights	Dav		Day	Dark, No Street Lights	Day	Dark, No Street Lights	Dark, No Street Lights	Dark, No Street Lights	Day	Dark, No Street Lights	Dark, No Street Lights	Dark, No Street Lights	Day	Uay	Dark, No Street Lights	Dawn or Dusk	Dawn or Dusk	Dark, No Street Lights	Dark, No Street Lights	Dawn or Dusk	Dark, No Street Lights	Day	Day	Dark, No Street Lights	Day	Dark, No Street Lights	-									
	9/7/2004	10/22/2004	10/10/2004	6/16/2003	8/14/2005	9/1/2008	8/29/2001	6/8/2009	8/7/2002	12/30/2005	10/25/2000	8/20/2001	10/16/2002	6/19/2006	11/18/2002	9/5/2001	8/31/2002	7/12/2004	8/18/2009	11/13/2010	1/3/2002	6007 /8/0T	+002/01/01	6002/CT/8	6/29/2003	6/22/2003	8/23/2005	6/15/2008	5/18/2008	11/21/2003	9/3/2000	8/30/2003	┢	+	ľ	+	╉	_	- 1
	Property Dmg Report	Property Dmg Report	Property Dmg Report	Property Dmg Report	Property Dmg Report	Property Dmg Report	Property Dmg Report	B Injury Accident	B Injury Accident	Property Dmg Report	Property Dmg Report	C Injury Accident	Property Dmg Report	Property Dmg Report	Property Dmg Report	Property Dmg Report	Property Dmg Report	Property Dmg Report	Property Dmg Report	Property Umg Report		Clearner Accident	Property Ome Report	Property Dring Report	Property Jing Report	B INJURY ACCIDENC	Property Dmg Report	Property Umg Report	Property Dmg Report	Property Dmg Kepon	Property Dmg Report	A Injury Accident	Property Umg Keport	Property Unig Report	Property Ding Report	Property Ding Report	Cilijaly Accident	Claims Accident	Droppety Dmg Report





Appendix B.5

Wild Animal Crashes on US-95 in District 2

Between January 1, 2003 and December 31, 2012

42	41	40	6 E	38	37	36	35	34	33	32	31	30	29	28	27	26	25	24	23	22	21	20	19	18	17	16	15	14	13	12	11	10	9	8	7	6	5	4	ω	2	1	Accident #	
218.590	218.400	218.300	218.100	217.000	216.700	216.500	215.900	215.800	215.700	215.000	214.009	213.200	210.100	210.049	209.600	208.600	208.100	204.800	204.500	203.012	202.300	199.019	195.003	194.129	193.500	192.200	192.018	192.000	191.000	190.900	190.500	189.300	188.426	187.500	187.000	186.500	185.600	185.000	183.601	183.500	182.700	Milepost	
Pickup/Van/Panel/SUV	Pickup/Van/Panel/SUV	SUV/Crossover	SUV/Crossover	Pickup/Van/Panel/SUV	Pickup/Van/Panel/SUV	SUV/Crossover	SUV/Crossover	Pickup/Van/Panel/SUV	Truck - 2 Axle/6 Tires	Car	Pickup/Van/Panel/SUV	Pickup/Van/Panel/SUV	SUV/Crossover	Pickup	Car	Pickup/Van/Panel/SUV	SUV/Crossover	Pickup/Van/Panel/SUV	SUV/Crossover	Car	Pickup/Van/Panel/SUV	Pickup	Pickup/Van/Panel/SUV	Pickup/Van/Panel/SUV	Car	Pickup/Van/Panel/SUV	Car	Car	Tractor - 1 Trailer	Car	Car	Car	Car	Pickup	Pickup/Van/Panel/SUV	Tractor - 1 Trailer	Car	Car	Car	Van - 1 to 8 seats	Pickup/Van/Panel/SUV	Vehicle Type	
Going Straight	Going Straight	Negotiating Curve	Going Straight	Going Straight	Going Straight	Turning Left	Going Straight	Going Straight	Going Straight	Going Straight	Going Straight	Negotiating Curve	Going Straight	Avoiding Obstacle	Negotiating Curve	Going Straight	Going Straight	Negotiating Curve	Going Straight	Going Straight	Going Straight	Going Straight	Going Straight	Negotiating Curve	Going Straight	Negotiating Curve	Going Straight	Going Straight	Going Straight	Going Straight	Going Straight	Negotiating Curve	Going Straight	Going Straight	Going Straight	Going Straight	Going Straight	Going Straight	Going Straight	Negotiating Curve	Going Straight	Driver Action	
Animal - Wild	Animal - Wild	Animal - Wild	Animal - Wild	Animal - Wild	Animal - Wild	Animal - Wild	Animal - Wild	Animal - Wild	Animal - Wild	Animal - Wild	Animal - Wild	Animal - Wild	Animal - Wild	Animal - Wild	Animal - Wild	Animal - Wild	Animal - Wild	Animal - Wild	Animal - Wild	Animal - Wild	Animal - Wild	Animal - Wild	Animal - Wild	Animal - Wild	Animal - Wild	Animal - Wild	Animal - Wild	Animal - Wild	Animal - Wild	Animal - Wild	Animal - Wild	Animal - Wild	Animal - Wild	Animal - Wild	Animal - Wild	Animal - Wild	Animal - Wild	Animal - Wild	Animal - Wild	Animal - Wild	Animal - Wild	Event	Most Harmful
Nonjunction	Nonjunction	Nonjunction	Nonjunction	Nonjunction	Nonjunction	Nonjunction	Nonjunction	Nonjunction	Nonjunction	Nonjunction	Nonjunction	Nonjunction	Nonjunction	Nonjunction	Nonjunction	Nonjunction	Nonjunction	Nonjunction	Nonjunction	Nonjunction	Nonjunction	Nonjunction	_	Intersection	Nonjunction	Nonjunction	Nonjunction	Nonjunction	Nonjunction	Nonjunction	Nonjunction	Nonjunction	Nonjunction	Nonjunction	Nonjunction		Nonjunction	Nonjunction	Nonjunction	Nonjunction	Nonjunction	Junction	Event Relation to
Clear	Cloudy	Clear	Clear	Clear	Cloudy	Clear	Clear	Cloudy	Rain	Cloudy	Clear	Clear	Clear	Clear	Clear	Clear	Cloudy	Cloudy	Cloudy	Clear	Clear	Clear	Clear	Cloudy	Clear	Clear	Clear	Clear	Cloudy	Clear	Clear	Clear	Clear	Clear	Cloudy	Cloudy	Claudy	Clear	Cloudy	Cloudy	Cloudy	Weather	
Dry	Dry	Dry	Dry	Dry	Dry	Dry	Dry	Dry	Wet	Wet	Dry	Dry	Dry	Dry	Dry	Dry	Dry	Dry	Wet	Dry	Dry	Dry	Dry	Dry	Dry	Dry	DIV	Dry	Dry	Dry	Dry	Dry	Wet	Dry	Ice	Dry	Wet	Dry	Dry	Dry	Dry	Surface	
Day	Dark, No Street Lights	Day	Dark, No Street Lights	Day	Dark, No Street Lights	Day	Day	Day	Dawn or Dusk	Dark, No Street Lights	Day	Day	Dawn or Dusk	Dark, No Street Lights	Day	Dark, No Street Lights	Dark, No Street Lights	Dawn or Dusk	Day	Dark, No Street Lights	Dark, No Street Lights	Dark, No Street Lights	Day	Dark, Street Lights On	Dawn or Dusk	Dawn or Dusk	Day	Dark, No Street Lights	Dark, No Street Lights	Dawn or Dusk	Dark, No Street Lights	Dark, No Street Lights	Dawn or Dusk	Dark, No Street Lights	Dark, No Street Lights	Day	Dark, No Street Lights	Dark, No Street Lights	Dark, No Street Lights	Day	Dark, No Street Lights	Light	
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8/30/2003	9/16/2005	7/13/2012	7/27/2012	6/12/2007	5/1/2003	7/21/2012	8/17/2012	2/3/2003	4/26/2012	9/2/2004	10/2/2007	8/8/2004	10/6/2012	1/4/2012	6/25/2011	3/13/2004	4/23/2012	9/21/2008	1/17/2011	11/23/2012	7/20/2008	8/13/2012	8/26/2005	2/1/2010	4/26/2009	8/14/2004	5/24/2010	10/21/2008	11/16/2005	5/18/2012	12/24/2010	12/29/2006	11/28/2010	12/16/2011	12/23/2008	1/14/2012	12/23/2007	2/18/2008	2/10/2008	7/16/2011	2/16/2009	តី	
Property Dmg Report	Property Dmg Report	Property Dmg Report	Property Dmg Report	Property Dmg Report	Property Dmg Report	Property Dmg Report	Property Dmg Report	Property Dmg Report	Property Dmg Report	Property Dmg Report	Property Dmg Report	Property Dmg Report	Property Dmg Report	Property Dmg Report	Property Dmg Report	Property Dmg Report	Property Dmg Report	Property Dmg Report	Property Dmg Report	Property Dmg Report	Property Dmg Report	Property Dmg Report	Property Dmg Report	Property Dmg Report	Property Dmg Report	Property Dmg Report	Property Dmg Report	Property Dmg Report	Property Dmg Report	Property Dmg Report	Property Dmg Report	Property Dmg Report	Property Dmg Report	Property Dmg Report	Property Dmg Report	Property Dmg Report	Property Dmg Report	Property Dmg Report	Property Dmg Report	Property Dmg Report	Property Dmg Report	Severity	• •

Wild Animal Crashes on US-95 in District 2 between 1/1/2003 and 12/32/2012

Total Crashes: 476 Total Fatalities: 0 Total Injuries: 58

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271.000	270.300	269.758	269.732	269.732	269.700	268.800	268.600	268.500	267.500	266.400	260.180	260.040	259.000	257.912	256.600	256.300	254.400	250.100	248.000	247.034	240.700	239.250	239.000	238.943	237.800	236,400	236.082	235.900	234.700	234.438	234.100	234.000	233.700	232.000	224.000	000 112	222. JUU	2001 200	221 962	221.924	221.900	221.800	221.400	221.005	220.700	219.600	219 500	219 000
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Negotiating Curve	Going Straight	Going Straight	Going Straight	Going Straight	Negotiating Curve	Going Straight	Going Straight	Going Straight	Going Straight	Going Straight	Going Straight	Going Straight	Going Straight	Going Straight	Going Straight	Negotiating Curve	Going Straight	Going Straight	Going Straight	Going Straight	Going Straight	Going Straight	Going Straight	Going Straight	Going Straight	Going Straight	Going Straight	Going Straight	Going Straight	Going Straight	Going Straight	Going Straight	Going Straight	Going Straight	Going Straight	Going Straight	Going Straight	Going Straight	Going Straight	Negotiating Curve	Going Straight	Going Straight	Going Straight			INe		Going Straight
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Nonjunction	Nonjunction	Nonjunction	Nonjunction	Nonjunction	Nonjunction	Nonjunction	Nonjunction	Nonjunction	Nonjunction	Nonjunction	Nonjunction	Nonjunction	Nonjunction	Nonjunction	Nonjunction	Nonjunction	Nonjunction	Nonjunction	Nonjunction	Nonjunction	Nonjunction	Nonjunction	Nonjunction	Nonjunction	Nonjunction	Nonjunction	Nonjunction	Nonjunction	Nonjunction	Nonjunction	Nonjunction	Nonjunction	Nonjunction								Nonjunction	Nonjunction			Nonjunction			Nonjunction
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0/07/27/C	9/18/2004	/ 12/2007	4/19/2007	3/24/2007	8007/81//	2/10/2022	3/3/2012	CO07 15 101	71/2/01/1/	+002/0C/#	5007/5/6	5007 /8/0	6002/2/1	7/9/2010	7/15/2011	9/27/2005	8/12/2011	10/3/2012	7/18/2007	7/12/2012	7/16/2011	9/10/2003	7/22/2007	10/20/2010	9/3/2007	9/6/2004	10/31/2010	9/1/2012	8/27/2007	5/14/2008	9/7/2012	7/18/2004	11/15/2012	5/12/2005	8/14/2008	10/23/2011	7/24/2004	5/1/2010	8/7/2012	6/1/2009	5/8/2009	7/5/2012	12/23/2010	6/16/2008	12/26/2012	8/21/2010	6/2/2005	10/26/2003
	Property Umg Report	Property Dmg Report	Property Dmg Report	Property Dmg Report	Property Dmg Report	Property Ding Report	Property Umg Report		Property Dmg Report	Property Ding Report	Property Drig Report	Property Dill nepur	Property Dmg Report	Property Umg Report	Property Dmg Report	Property Umg Report	Property Umg Report	Property Dmg Report	Property Dmg Report	Property Dmg Report	Property Dmg Report	Property Dmg Report	Property Dmg Report	Property Dmg Report	Property Dmg Report	Property Dmg Report	Property Dmg Report	Property Dmg Report	Property Dmg Report	Property Dmg Report	Property Dmg Report	Property Dmg Report	Property Dmg Report	Property Dmg Report	Property Dmg Report	Property Dmg Report	Property Dmg Report	Property Dmg Report	Property Dmg Report	Property Dmg Report	Property Dmg Report	Property Dmg Report	Property Dmg Report	Property Dmg Report	Property Dmg Report	Property Dmg Report	Property Dmg Report	Property Dmg Report

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Car	Car	Car	Pickup	Car	Pickup/Van/Panel/SUV	Pickup/Van/Panel/SUV	Pickup/Van/Panel/SUV	Pickup/Van/Panel/SUV	SUV/Crossover	Pickup/Van/Panel/SUV	Pickup/Van/Panel/SUV	Car	Pickup/Van/Panel/SUV	Pickup/Van/Panel/SUV	Car	Car	Pickup/Van/Panel/SUV	Car	Car	Pickup/Van/Panel/SUV	Pickup/Van/Panel/SUV	Pickup	Car	Car	Car	Pickup/Van/Panel/SUV	Car	Pickup/Van/Panel/SUV	Car	Car	Car	Tractor - 1 Trailer	Pickup/Van/Panel/SUV	Pickun/Van/Panel/SUIV	Car		Pickup/Van/Panel/SUV	Car	Car	Car	Pickup/Van/Panel/SUV	Car	Pickup/Van/Panel/SUV	Car	SUV/Crossover	Car	Car
Going Straight	Going Straight	Going Straight	Going Straight	Going Straight	Going Straight	Going Straight	Going Straight	Going Straight	Going Straight	Going Straight	Going Straight	Going Straight	Going Straight	Going Straight	Going Straight	Negotiating Curve	Going Straight	Going Straight	Going Straight	Going Straight	Avoiding Obstacle	Going Straight	Going Straight	Going Straight	Going Straight	Going Straight	Going Straight	Going Straight	Negotiating Curve	Going Straight	Going Straight	Going Straight	Negotiating Curve	Going Straight	Going Straight	Going Straight	Going Straight	Going Straight	Going Straight	Going Straight	Going Straight	Going Straight	Going Straight	Going Straight	Going Straight		Negotiating Curve
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Nonjunction	Ł	Nonjunction		Nonjunction	Nonjunction	Nonjunction	Nonjunction	Nonjunction	Nonjunction	Nonjunction	Nonjunction	Nonjunction	Nonjunction	Nonjunction	Nonjunction	Nonjunction	Nonjunction		Nonjunction	Nonjunction	Nonjunction	Nonjunction	_ 1	Nonjunction		Nonjunction		Nonjunction	Nonjunction	Nonjunction	Nonjunction				Noniunction						Nonjunction	Nonjunction		Nonjunction			Nonjunction (
Clear	Clear	Clear	Cloudy	Clear	Clear	Clear	Clear	Clear	Clear	Clear	Clear	Clear	Cloudy	Clear	Clear	Cloudy	Fog	Clear	Clear	Cloudy	Cloudy	Rain	Clear	Clear	Cloudy	Clear	Clear	Cloudy	Clear	Clear	Cloudy	Clear	Clear	Clear	Cloudy	Close	Clear	Clear	Clear	Clear	Clear	Cloudy	Clear	Rain	Cloudy	Cloudy	Cloudy
Dry	Dry	Dry	VQ VQ	Dry	Dry	Dry	Dη	Dry	Dry	Dry	P7	DIV	Dry	Dry	Dry	Dry	Wet	Dη	Dη	Dry	Dry	Wet	Dry	Dry	Dry	Dry	Dry	Dry	Dry	Dry	Dry	Dry	Dry	Dry .					VIO	UN	Dry	Wet	Dry	Wet	Dry	Dry	Dry
Dark, No Street Lights	Day	Dark, No Street Lights		Dark, No Street Lights	Day	Dark, No Street Lights	Day	Dawn or Dusk	Dark, No Street Lights	Dawn or Dusk	Dark, No Street Lights	Dark, No Street Lights	Day	Dark, No Street Lights	Dawn or Dusk	Dark, No Street Lights	Day	Dark, No Street Lights	Dawn or Dusk	Dav	Dark, NO Street Lights	Dark, ND Street Lights	Day	Dark, No Street Lights	Dawn or Dusk	Dark, No Street Lights	Dark, No Street Lights																				
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195.003	194.129	193.500	192.200	192.018	192.000	191.000	190.900	190.500	189.300	188.426	187.500	187.000	186.500	185.600	185.000	183.601	183.500	182.700	318.100	318.000	317.500	317.458	317.400	315.000	315.000	314.985	314.800	314.800	314.500	314,160	314.100	314.000	313,800	313 400	313.200	313.100	313.100	313.018	312.800	312.500	311.500	311.000	310.800	310.500	309,900	309.700	309.433
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365.900	365.800	365.650	365.500	365.500	365.500	365.000	364.180	363.800	363.780	363.700	363.687	363.500	362.600	362.500	362.500	362.100	362.000	361.724	361.480	361.200	361.200	361.001	361.000	360.994	360.981	360.800	360.611	360.400	360.300	360.020	360.001	359.770	359.700	359.500	359.500	359.500	359.200	358.981	358.976	358.900	358.800	358.724	358.600	358.500	358.197	358.000	357.600	357.500
Tractor - 2 Trailers	Pickup/Van/Panel/SUV	Pickup/Van/Panel/SUV	Pickup/Van/Panel/SUV	Car	Pickup/Van/Panel/SUV	Car	Car	Car	Car	Van - 1 to 8 seats	Car	Pickup/Van/Panel/SUV	Car	Pickup/Van/Panel/SUV	Car	Car	Pickup/Van/Panel/SUV	Pickup/Van/Panel/SUV	Car	Pickup/Van/Panel/SUV	Car	SUV/Crossover	Car	Pickup/Van/Panel/SUV	Car	Car	Pickup	Car	Car	Pickup/Van/Panel/SUV	Car	Pickup/Van/Panel/SUV	Pickup	Matorcycle	Pickup/Van/Panel/SUV	Car	Pickup/Van/Panel/SUV	Car	Pickup/Van/Panel/SUV	Car	Pickup/Van/Panel/SUV	Car	Pickup	SUV/Crossover	Pickup/Van/Panel/SUV	Pickup/Van/Panel/SUV	Car	Pickup/Van/Panel/SUV
Negotiating Curve	Going Straight	Going Straight	Going Straight	Going Straight	Going Straight	Going Straight	Going Straight	Going Straight	Going Straight	Going Straight	Going Straight	Going Straight	Going Straight	Going Straight	Going Straight	Going Straight	Going Straight	Negotiating Curve	Going Straight	Going Straight	Going Straight	Going Straight	Going Straight	Going Straight	Going Straight	Going Straight	Negotiating Curve	Going Straight	Negotiating Curve	Going Straight	Going Straight	Going Straight	Going Straight	Going Straight	Going Straight	Going Straight	Going Straight	Going Straight	Going Straight									Going Straight
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Dry	Dry	Dry	Dry	Dry	Wet	Dry	Dry	Wet	Wet	Dry	Dry	Wet	Dry	Dry	Dry	Wet	Wet	Dry	Dry	Wet	Dry	Dry	Dry	Dry	Dry	Dry	Dry	Dry	Dry	Dry	Snow	Dry	Dry	Dry	Dry	Dı	Dγ	Dry	Dry	Dη	Dry	Dry	Dry	Wet	Dry	Wet	Dry	Dry
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Going Straight	Going Straight	Going Straight	Going Straight	Negotiating Curve	Going Straight	Going Straight		Negotiating Curve	Going Straight Negotiating Curve	Going Straight Going Straight Negotiating Curve	Going Straight Going Straight Going Straight Negotiating Curve	Going Straight Going Straight Going Straight Going Straight Negotiating Curve	Going Straight Going Straight Going Straight Going Straight Going Straight Negotiating Curve	Negotiating Curve Going Straight Going Straight Going Straight Going Straight Going Straight Negotiating Curve	Going Straight Negotiating Curve Going Straight Going Straight Going Straight Going Straight Going Straight Going Straight	Going Straight Negotiating Curve Negotiating Curve Going Straight Going Straight Going Straight Going Straight Going Straight Going Straight	Going Straight Going Straight Negotiating Curve Going Straight Going Straight Going Straight Going Straight Going Straight Going Straight	Going Straight Going Straight Going Straight 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375.942	374.500	374.000	372.733	372.037	370.300	368.100	357.800	356.100	356.100	350.700	350.600	350.346	343.100	339.800	338.200	338.100	338.100	338.004	337.180	315.024	310,300	308.800	303,700	302.995	281.400	278.520	275.100	268.100	208.900	188.000	373.001	368.003	353.990	328.400	319.600	318.500	315.000	309.000	283.004	226.800	183.600
Car	Car	Matorcycle	Car	Pickup/Van/Panel/SUV	Pickup/Van/Panel/SUV	Pickup/Van/Panel/SUV	Pickup/Van/Panel/SUV	SUV/Crossover	Pickup/Van/Panel/SUV	Car	Car	Car	Car	Car	Pickup/Van/Panel/SUV	Car	Car	Car	Pickup/Van/Panel/SUV	Саг	Car	Pickup/Van/Panel/SUV	Pickup/Van/Panel/SUV	Car	Car	SUV/Crossover	Pickup/Van/Panel/SUV	Pickup/Van/Panel/SUV	Car	Car	Car	Car	Pickup	Pickup/Van/Panel/SUV	Car	Car	Pickup/Van/Panel/SUV	Pickup/Van/Panel/SUV	Pickup/Van/Panel/SUV	Car	Car
Avoiding Obstacle	Going Straight	Going Straight	Going Straight	Negotiating Curve	Going Straight	Negotiating Curve	Going Straight	Going Straight	Going Straight	Going Straight	Going Straight	Going Straight	Going Straight	Going Straight	Going Straight	Going Straight	Negotiating Curve	Going Straight	Negotiating Curve	Negotiating Curve	Going Straight	Going Straight	Going Straight	Negotiating Curve	Negotiating Curve	Going Straight	Going Straight	Negotiating Curve	Negotiating Curve	Going Straight	Going Straight	Negotiating Curve	Going Straight	Going Straight	Going Straight	Going Straight	Negotiating Curve	Going Straight	Going Straight	Going Straight	Going Straight
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Nonjunction	Nonjunction	Nonjunction	Nonjunction	Nonjunction	Nonjunction	Nonjunction	Nonjunction	Nonjunction	Nonjunction	Nonjunction	Nonjunction	Nonjunction	Nonjunction	Nonjunction	Nonjunction	Nonjunction	Nonjunction	Nonjunction	Intersection	Nonjunction	Nonjunction	Nonjunction	Nonjunction	Nonjunction	Nonjunction	Nonjunction	Nonjunction	Nonjunction	Nonjunction	Nonjunction	Nonjunction	Nonjunction	Nonjunction								
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Dry	Wet	Dry	Dry	Dry	Dry	Dry	ΡŊ	Dry	Wet	PZ	Dry	Wet	Dry	Wet	Dry	Dry	Dry	Wet	Wet	Dry	Dry	Dry	Dry	Dry	Dry	Dry	Dry	Dry	Dry	Dry	Dry	Dry	Dry								
Dark, No Street Lights	Dark, No Street Lights	Day	Dark, No Street Lights	Day	Dark, No Street Lights	Day	Dark, No Street Lights	Dark, No Street Lights	Dark, No Street Lights	Day	Dark, No Street Lights	Dark, Street Lights On	Dark, No Street Lights	Dawn or Dusk	Dark, No Street Lights	Dark, No Street Lights	Day	Dark, No Street Lights	Dark, Street Lights On	Dark, No Street Lights	Dark, No Street Lights	Dark, No Street Lights	Dark, Street Lights Off	Day	Day	Dark, No Street Lights															
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3 Monday	2 Friday	1 Friday	2 Tuesday	1 Wednesday	2 Friday	1 Wednesday	Thursday	Tuesday	Monday	Friday	Sunday	Tuesday	Sunday	Saturday	2 Thursday	1 Sunday	1 Monday	Tuesday	2 Wednesday	1 Wednesday	Thursday	Tuesday	Wednesday	Wednesday	Sunday	Friday	Tuesday	Friday	Friday	Monday	Tuesday	Sunday	Friday	Saturday	Friday	Monday	Thursday	Sunday	Saturday	Monday	Monday
8/18/2008	2/10/2012	7/25/2008	11/10/2009	9/15/2010	9/10/2010	12/15/2010	8/23/2007	9/27/2011	4/5/2010	10/20/2006	2/17/2008	12/14/2010	7/25/2004	10/1/2005	10/25/2007	11/4/2012	10/25/2010	11/20/2012	1/7/2009	10/24/2007	10/12/2006	11/4/2008	1/23/2008	10/29/2003	11/1/2009	9/23/2011	6/19/2007	12/25/2009	9/15/2006	1/23/2006	10/12/2010	7/8/2007	9/7/2012	8/7/2010	6/18/2004	11/17/2008	5/15/2008	10/3/2004	5/10/2008	7/4/2011	1/16/2012
C Injury Accident	C Injury Accident	C Injury Accident	C Injury Accident	C Injury Accident	C Injury Accident	C Injury Accident	C Injury Accident	C Injury Accident	C Injury Accident	C Injury Accident	C Injury Accident	C Injury Accident	C Injury Accident	C Injury Accident	C Injury Accident	C Injury Accident	C Injury Accident	C Injury Accident	C Injury Accident	C Injury Accident	C Injury Accident	C Injury Accident	C Injury Accident	C Injury Accident	C Injury Accident	C Injury Accident	C Injury Accident	C Injury Accident	C Injury Accident	C Injury Accident	B Injury Accident	B Injury Accident	B Injury Accident	B Injury Accident	B Injury Accident	B Injury Accident	B Injury Accident	B Injury Accident	B Injury Accident	B Injury Accident	B Injury Accident

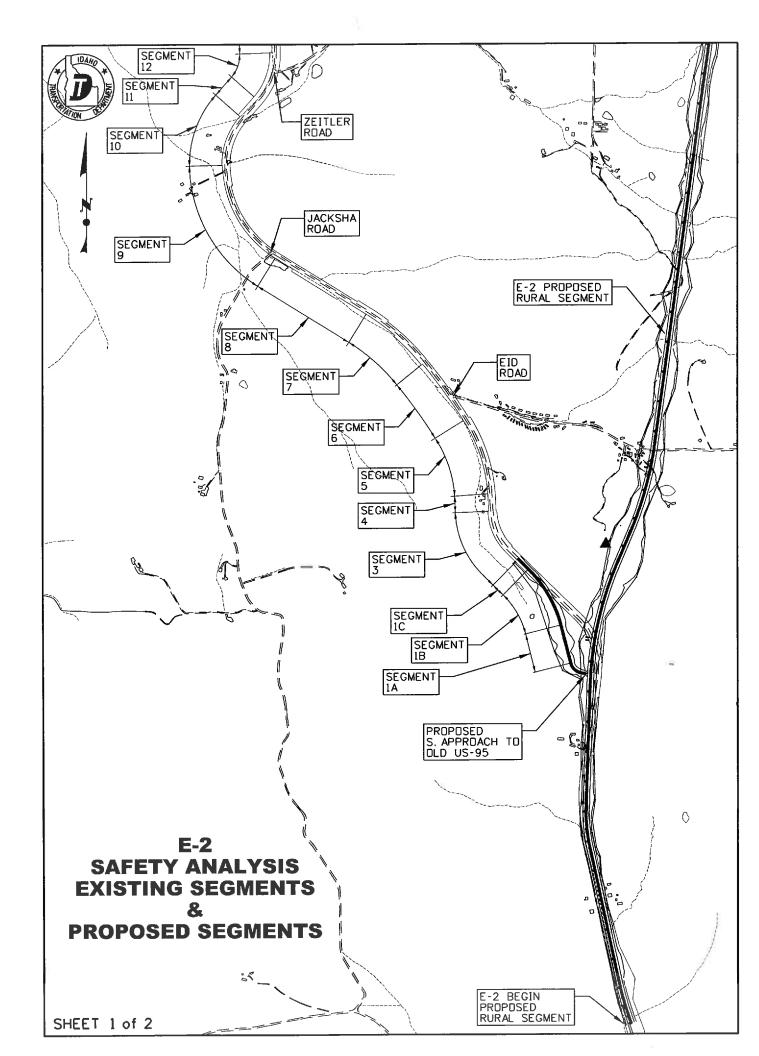
Accident Injury Classification for Wild Animal Crashes Along US-95 in District 2 Having Multiple Injuries Between 1/1/03 and 12/31/12

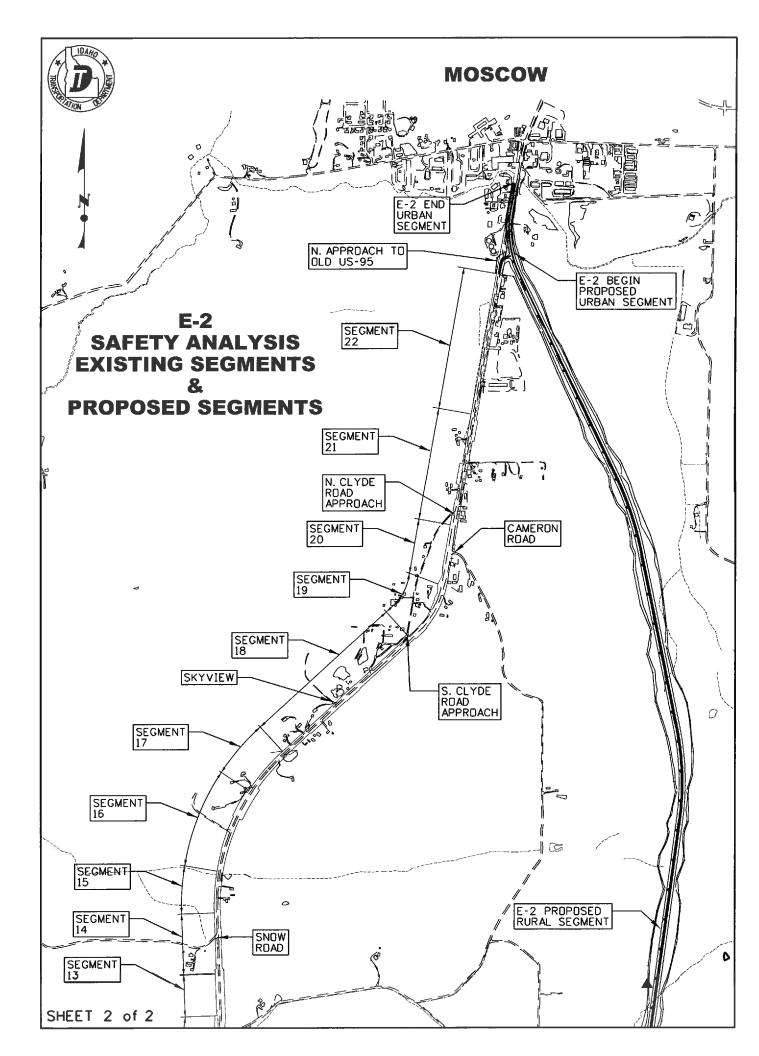
- Accident 433 1 Type A Incapacitating Injury, 1 Type B Non Incapacitating Injury
- Accident 438 4 Type B Non Incapacitating Injuries
- Accident 443 2 Type B Non Incapacitating Injuries

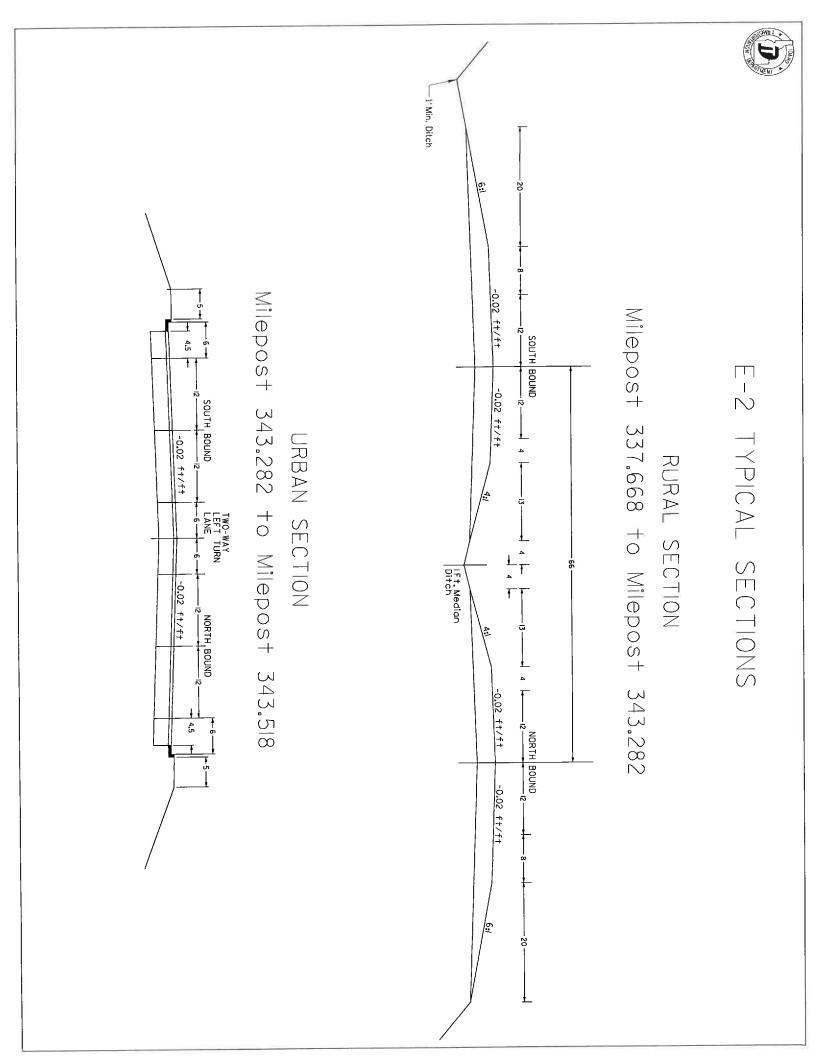
*From Data provided by ITD Headquarters Office of Highway Safety on Webcars.

Appendix C.1

Segment Breakdown, Proposed Typical Sections, and Predictions For Alternative E2







Predicted 2017 ADT for Proposed US-95 and County Roads for Safety Analysis

<u>Rural US-95</u> – **5920** From ITD Traffic Volume Report <u>Suburban US-95</u> – **7465** From ITD Traffic Volume Report <u>Eid Road</u> – **65** – From Current North Latah Highway District Plan <u>Jacksha Road</u> – **50** – Volume is predicted to be less than Eid Road Volume <u>Zeitler Road</u> – **50** – Volume is predicted to be less than Eid Road Volume <u>Snow Road</u> – **25** – Volume is predicted to be very light due to few residences <u>Skyview Drive</u> – **25** – The private drive currently services only 1 home <u>North Clyde Road</u> – **50** – Volume predicted to be greater than other rural roads based on proximity to Moscow and the number of homes <u>South Clyde Road</u> – **50** – Volume of traffic predicted to be light and less than Eid Road Volume

Predicted 2017 ADT on Existing US-95 Loop If Alternative E2 Was Constructed

Segments 1a - 19 - 300 (For Predictions See Below)

27 Approaches generate 10 trips per day = 270 Eid Road = 65, Jacksa Road = 50, Zeitler Road = 50, Snow Road = 25, Skyline Drive = 25, Clyde Road South = 50 / Total = 265

Predict an Even Directional Split – (270+265)/2 = 267.5

Predict about 32.5 commuters that drive through on Old US-95 per day – 267.5 + 32.5 = 300

Segments 20-22 - 1545 from Differences in ITD Traffic Volume Report (7465 – 5920 = 1545)

Predicted Traffic Growth Rates

Proposed US-95 Rural – 1.63% From ITD Traffic Volume Report
Proposed US-95 Suburban – 1.58% From ITD Traffic Volume Report
Segment 22 on Existing US-95 Loop – 0.91% Growth Rate of Moscow from Latah County Comprehensive
Plan because the segment is within the area of impact of Moscow
Segments 20,21 on Existing US-95 Loop – 0.53% Growth Rate of Unincorporated Latah County from
Latah County Comprehensive Plan because the segments are near the area of impact of Moscow
Segments 1a-19 – 0.19% Growth Rate within the Thorncreek to Moscow Corridor based on Census
Bureau Data in the Thorncreek to Moscow – This is a new subdivision so growth is predicted to be
greater than other areas within the corridor
All County Roads Except Skyline Drive – 0.19% Growth Rate within the Thorncreek to Moscow Corridor based on Census

Assumed E2 ADTs

E2 Ru	ral ADT	E2 Subu	rban ADT	Г	E2 Segmen	t 21,20 ADT		E2 Segme	nt 22 ADT
Year	ADT	Үеаг	ADT		Year	ADT		Year	ADT
2017	5920	2017	7465		2017	1545		2017	1545
2018	6016	2018	7583		2018	1553		2018	1559
2019	6114	2019	7703		2019	1561		2019	1573
2020	6214	2020	7825		2020	1570		2020	1588
2021	6315	2021	7949	\sim	2021	1578		2021	1602
2022	6417	2022	8075		2022	1586		2022	1617
2022	6522	2023	8203		2023	1594		2023	1631
2023	6628	2024	8333		2024	1603		2024	1646
2024	6736	2025	8465		2025	1611		2025	1661
2025	6845	2026	8599		2026	1620		2026	1677
2020	6957	2027	8735		2027	1628		2027	1692
2027	7070	2027	8873		2028	1637		2028	1707
	7185	2028	9014		2028	1646		2029	1723
2029	7302	2029	9014 9157		2020	1654		2030	1738
2030		2030	9302		2030	1663		2030	1754
2031	7421					1672		2031	1770
2032	7541	2032	9449		2032	1672		2032	1786
2033	7664	2033	9598		2033			2033	1788
2034	7789	2034	9750		2034	1689			
2035	7915	2035	9905		2035	1698		2035	1819
2036	8044	2036	10062		2036	1707		2036	1836
2037	8175	2037	10221		2037	1716		2037	1853
2038	8308	2038	10383		2038	1725		2038	1869
2039	8443	2039	10547		2039	1734		2039	1886
2040	8581	2040	10714		2040	1743		2040	1904
2041	8720	2041	10884		2041	1753		2041	1921
2042	8862	2042	11056		2042	1762		2042	1939
2043	9006	2043	11231		2043	1771		2043	1956
2044	9153	2044	11409		2044	1780		2044	1974
2045	9302	2045	11590		2045	1790		2045	1992
2046	9453	2046	11773		2046	1799		2046	2010
2047	9607	2047	11960		2047	1809		2047	2029
2048	9763	2048	12149		2048	1818		2048	2047
2049	9922	2049	12342		2049	1828		2049	2066
2050	10083	2050	12537		2050	1837		2050	2085
2051	10247	2051	12736		2051	1847		2051	2104
2052	10414	2052	12937	1	2052	1857		2052	2123
2052	10583	2053	13142		2053	1867		2053	2142
2055	10755	2054	13350		2054	1876		2054	2162
2054	10930	2055	13562		2055	1886		2055	2181
2055	11108	2055	13776	1	2055	1896		2056	2201
2056	11289	2050	13994		2057	1906		2057	2221
2057	11209	2057	13334		2058	1916		2058	2242
2058	11475	2058	14210		2059	1926		2059	2262
2059	11859	2039	14441		2055	1927		2060	2283
		2060	14870		2060	1947		2061	2303
2061	12042	2061	14902		2061	1947		2062	2324
2062	12238				2062	1957		2063	2346
2063	12437	2063	15378		2063	1967		2063	2340
2064	12639	2064	15621			1978		2064	2389
2065	12845	2065	15869		2065 2066	1988		2065	2389
2066	13054	2066	16120	JL	2000	1999	ł.	2.000	2410

Assumed E2 Old US-95 Highway ADTs

Old US-	-95 ADT	Eid R	d. ADT	Jacksha	Rd. ADT	Zeitler l	Rd. ADT
Year	ADT	Year	ADT	Year	ADT	Year	ADT
2017	300	2017	65	2017	50	2017	50
2018	301	2018	65	2018	50	2018	50
2019	301	2019	65	2019	50	2019	50
2020	302	2020	65	2020	50	2020	50
2021	302	2021	65	2021	50	2021	50
2022	303	2022	66	2022	50	2022	50
2022	303	2023	66	2023	51	2023	51
2024	304	2024	66	2024	51	2024	51
2024	305	2025	66	2025	51	2025	51
2025	305	2026	66	2026	51	2026	51
2020	306	2027	66	2027	51	2027	51
2027	306	2028	66	2028	51	2028	51
2028	307	2028	67	2029	51	2029	51
2029	308	2020	67	2030	51	2030	51
2030	308	2030	67	2031	51	2031	51
	309	2031	67	2032	51	2032	51
2032	309	2032	67	2032	52	2033	52
2033 2034	309 310	2033	67	2033	52	2034	52
		2034	67	2035	52	2035	52
2035	310	2035	67	2035	52	2036	52
2036	311	2038	68	2030	52	2037	52
2037	312	2037	68	2037	52	2038	52
2038	312	2038	68	2038	52	2038	52
2039	313	2039	68	2039	52	2035	52
2040	313		68	2040	52	2040	52
2041	314	2041 2042	68	2041	52	2041	52
2042	315					2042	53
2043	315	2043	68	2043	53	2043	53
2044	316	2044	68	2044	53		
2045	316	2045	69	2045	53	2045	53 53
2046	317	2046	69	2046	53	2046	53
2047	318	2047	69	2047	53	2047	53
2048	318	2048	69	2048	53	2048 2049	53
2049	319	2049	69	2049	53	2049	53
2050	319	2050	69	2050	53 53	2050	53
2051	320	2051	69 69	2051	53	2051	53
2052	321	2052	69 70	2052	54	2052	54
2053	321	2053	70 70	2053 2054	54 54	2055	54 54
2054	322	2054			54 54	2054	54 54
2055	323	2055	70 70	2055	54 54	2055	54 54
2056	323	2056	70 70	2056	54 54	2056	54 54
2057	324	2057	70	2057	54 54	2057	54 54
2058	324	2058	70 70	2058 2059	54	2058	54
2059	325	2059		2059	54	2059	54 54
2060	326	2060	71 71	2060	54 54	2060	54
2061	326	2061		2061	54 54	2061	54
2062	327	2062	71	2062	54 55	2062	55
2063	327	2063	71	2063	55	2065	55
2064	328	2064	71	2064	55	2064	55
2065	329	2065 2066	71 71	2065	55	2065	55
2066	329	2066	/1	2000	55	2000	

Snow F	Rd. ADT	Skyviev	v Dr. ADT	I I	Clyde I	Rd. ADT		Cameror	Rd. ADT
Year	ADT	Year	ADT	11	Year	ADT	1	Year	ADT
2017	25	2017	25	1 1	2017	50	1	2017	100
2018	25	2018	25		2018	50		2018	100
2019	25	2019	25		2019	50		2019	100
2020	25	2020	26		2020	50		2020	101
2020	25	2021	26		2021	50		2021	101
2021	25	2022	26		2022	50		2022	101
2022	25	2022	26		2023	51		2023	101
2023	25	2023	20		2023	51		2023	101
2024 2025	25	2024	27		2024	51		2024	101
2025	25	2025	27		2025	51		2025	102
					2020	51		2027	102
2027	25	2027	27		2027	51		2027	102
2028	26	2028	28			1			102
2029	26	2029	28		2029	51		2029	
2030	26	2030	28	11	2030	51		2030	103
2031	26	2031	28		2031	51		2031	103
2032	26	2032	29		2032	51		2032	103
2033	26	2033	29		2033	52		2033	103
2034	26	2034	29		2034	52		2034	103
2035	26	2035	29		2035	52		2035	103
2036	26	2036	30		2036	52		2036	104
2037	26	2037	30	11	2037	52		2037	104
2038	26	2038	30		2038	52		2038	104
2039	26	2039	31		2039	52		2039	104
2040	26	2040	31		2040	52		2040	104
2041	26	2041	31		2041	52		2041	105
2042	26	2042	31	11	2042	52		2042	105
2043	26	2043	32		2043	53		2043	105
2044	26	2044	32		2044	53		2044	105
2045	26	2045	32		2045	53		2045	105
2046	26	2046	33		2046	53		2046	106
2047	26	2047	33		2047	53		2047	106
2048	27	2048	33		2048	53		2048	106
2049	27	2049	33		2049	53		2049	106
2050	27	2050	34		2050	53		2050	106
2050	27	2050	34		2051	53		2051	107
2051	27	2051	34		2052	53		2052	107
2052	27	2052	35		2052	54		2052	107
2053	27	2053	35		2055	54		2055	107
2054	27	2055	35		2055	54		2055	107
2055	27	2055	36		2055	54		2055	108
2056	27	2056	36		2057	54		2050	108
2057	27 27	2058	36		2057	54		2057	108
2058	27 27	2058	37		2058	54		2058	108
					2059	54		2039	108
2060	27	2060	37				10		109
2061	27	2061	37		2061	54		2061	
2062	27	2062	38		2062	54		2062	109
2063	27	2063	38		2063	55		2063	109
2064	27	2064	38		2064	55		2064	109
2065	27	2065	39		2065	55		2065	110
2066	27	2066	39	ιL	2066	55		2066	110

J 16, 2011	7			2037				2017				2010	Year					ADT
	2037		001540	001539	2017		001540	001539	2010		001540	001539	l	Seg	Segment To	Segment From		ADT Volume Projection Report
			001540 001540	001539			001540	001539			001540	001539 001539	5	Segment		_	Route L	me P
	Weighted	339.620	337.668	337.180	Weighted	339.620	337.668	337.180	Weighted	339.620	337.668	337.180 337.668	From	Milepost	1540	1539	US095	rojec
	hted	342.930	339.620	337.180 337.668	hted	342.930	339.620	337.668	hted	342.930	339.620	337.668	То	post	Milepost To	Milepost From		tion I
	8,175	8,437	7,821	7,809	5,920	6,113	5,657	5,654	5,130	5,300	4,900	4,900	AADT		ost To	From		Repo
	1,318	1,323	1,323	1,264	843	8	8	8	677	9	9	9	CAADT		342.930	337.180		T
	18 925	23 954	23 886	64 885	43 679	847 700	847 650	809 650	77 593	680 611	680 567	650 567	DHV		_	-		
1	5 11.30	4 11.3	5 11.3	5 11.3	9 11.40	0 11.4) 11.4	0 11.4	11.50	11.5	7 11.5	11.5	% NHD		End	Start	_	
	0 104	.3 105	.3 105	.3 100	0 68	.4 68	.4 68	.4 65	0 55	.5 55	.5 55	5 53	CDHV		End Projection 2037	Start Projection 2017	Traffic Data	
a	7.91	7.912	7.933	7.934	8.01	8.014	8.044	8.044	8.07	8.071	8.106	8.106	CDHV %		n 2037	n 2017	a 2010	
		60/40%	60/40%	60/40%		60/40%	60/40%	60/40%		60/40%	60/40%	60/40%	DIR					
		eid rd	END NEW ALIGNMENT	THORN CREEK RD		EID RD	END NEW ALIGNMENT	THORN CREEK RD		EID RD	END NEW ALIGNMENT	THORN CREEK RD	From Description					
Page 1 of 1			EID RD	END NEW ALIGNMENT			EID RD	END NEW ALIGNMENT			EID RD	END NEW ALIGNMENT	To Description					

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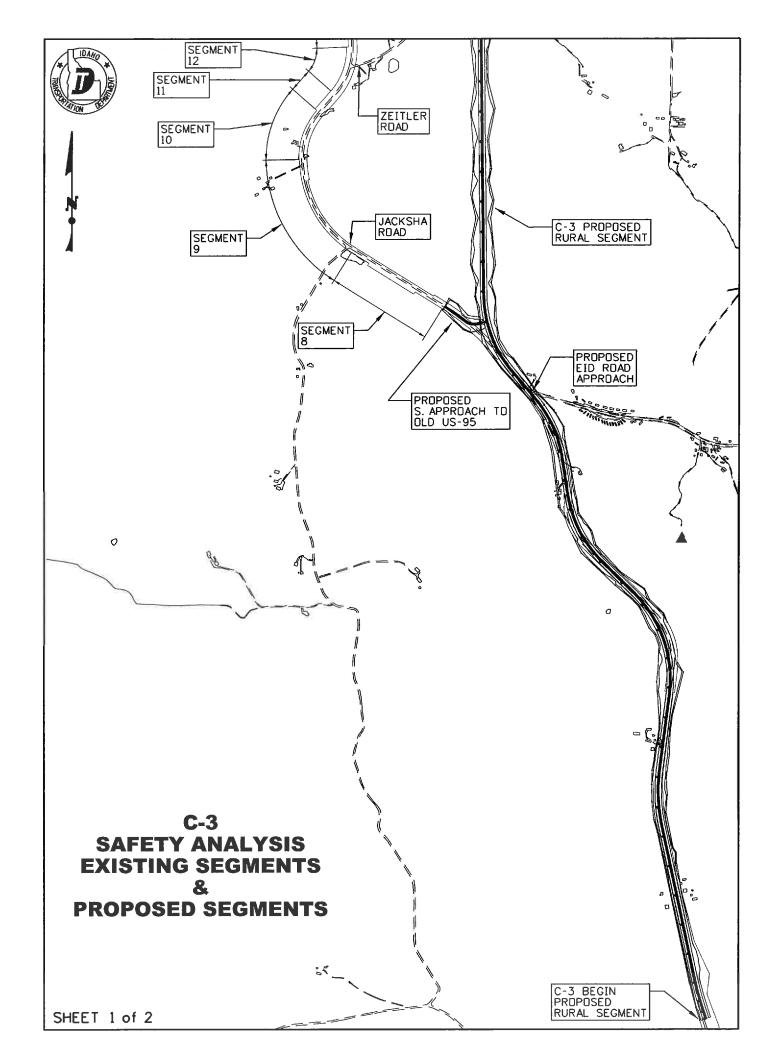
ADT Volume Projection Report	olume'	e Proj	ection	Repo	it								
		Route US095	S095					Traf	Traffic Data 2010	2010			
	Segment From	t From 1540	540	Milepost From		342.93		Start Projection 2017	ojection	2017			
	Segme	Segment To 1540	540	Milepo	Milepost To 344.11	344.11		End Pr	End Projection 2037	2037			
	Seg	Segment	Milepost	ost					2				
Year	From	То	From	Το	AADT	CAADT	DHV	о С	CDHV %		DIR	From Description	To Description
2010	001540	001540	342.933	344.116	6,500.	520	742	11.4	42 7		60/40%	60/40% CLYDE RD	PALOUSE RIVER DR
	2010	Wei	Weighted averages	ages	6,500	520	742	11.4	42	7.99			
2017		001540	342.933	344.116	7,465	647	848	11.3	51	51 7.947	60/40%	60/40% CLYDE RD	PALOUSE RIVER DR
	2017	Wei	Weighted averages	ages	7,465	647	848	11.3	51	7.95			
2037		001540	342.933	344.116	10,221	1,011	1,148	11.2	80 7.865		60/40%	60/40% CLYDE RD	PALOUSE RIVER DR
	2037	We	Weighted averages	ages	10,221	10,221 1,011 1,148 11.2	1,148	11.2	80 7.87	7.87			

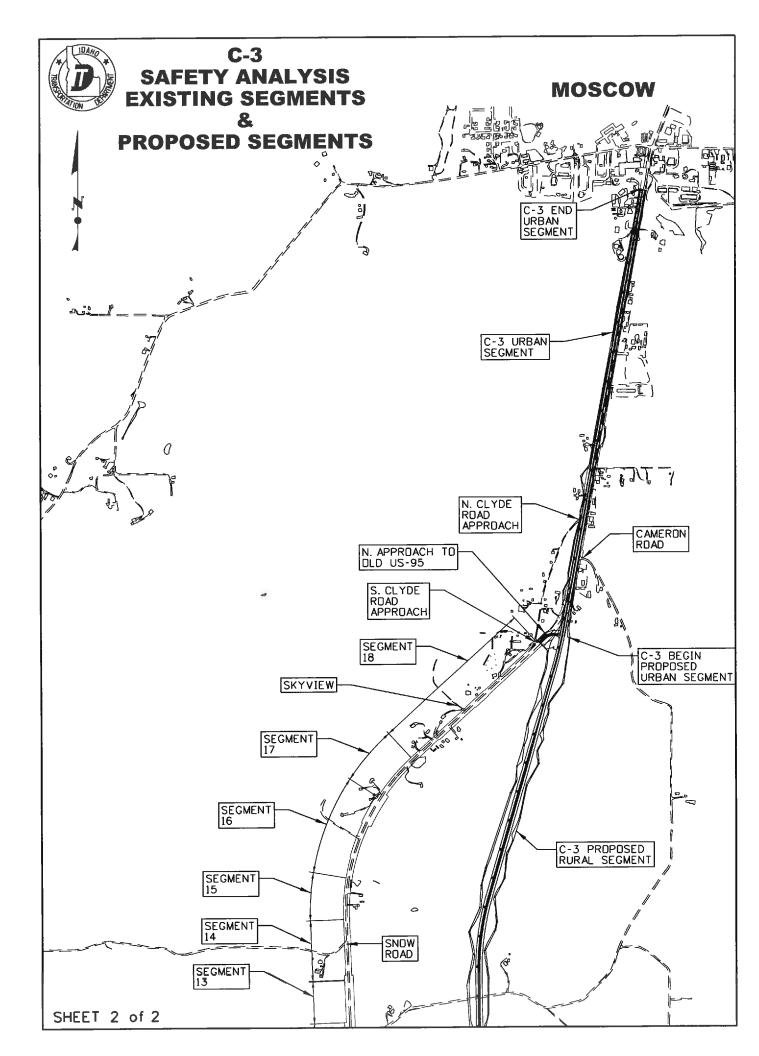
June 16, 2011

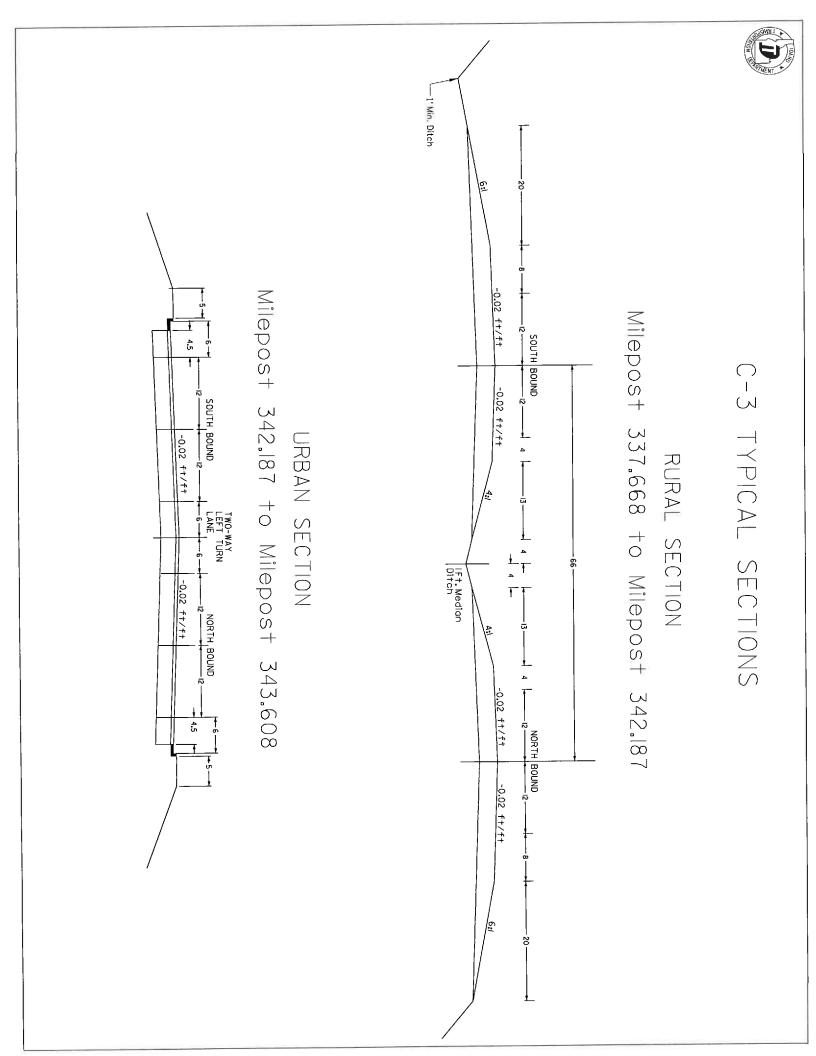
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Appendix C.2

Segment Breakdown, Proposed Typical Sections, and Predictions For Alternative C3







Predicted 2017 ADT for Proposed US-95 and County Roads for Safety Analysis

<u>Rural US-95</u> – **5920** From ITD Traffic Volume Report <u>Suburban US-95</u> – **7465** From ITD Traffic Volume Report <u>Eid Road</u> – **65** – From Current North Latah Highway District Plan <u>Jacksha Road</u> – **50** – Volume is predicted to be less than Eid Road Volume <u>Zeitler Road</u> – **50** – Volume is predicted to be less than Eid Road Volume <u>Snow Road</u> – **25** – Volume is predicted to be very light due to few residences <u>Skyview Drive</u> – **25** – The private drive currently services only 1 home <u>North Clyde Road</u> – **50** – Volume predicted to be greater than other rural roads based on proximity to Moscow and the number of homes <u>South Clyde Road</u> – **50** – Volume of traffic predicted to be light and less than Eid Road Volume

Predicted 2017 ADT on Existing US-95 Loop If Alternative C3 Was Constructed

Segments 8 – 18 – 220 (For Predictions See Below)

17 Approaches generate 10 trips per day = 170 Jacksa Road = 50, Zeitler Road = 50, Snow Road = 25, Skyline Drive = 25, Clyde Road South = 50 / Total = 200

Predict an Even Directional Split – (170+200)/2 = 187.5

Predict about 32.5 commuters that drive through on Old US-95 per day – 187.5 + 32.5 = 220

Predicted Traffic Growth Rates

Proposed US-95 Rural – 1.63% From ITD Traffic Volume Report

Proposed US-95 Suburban - 1.58% From ITD Traffic Volume Report

<u>Segments 8-18</u> – **0.19%** Growth Rate within the Thorncreek to Moscow Corridor based on Census Bureau Data in the Thorncreek to Moscow Community Impact Technical Report

<u>Skyline Drive</u> – **0.91%** Growth Rate of Moscow – This is a new subdivision so growth is predicted to be greater than other areas within the corridor

All County Roads Except Skyline Drive – **0.19%** Growth Rate within the Thorncreek to Moscow Corridor based on Census Bureau Data in the Thorncreek to Moscow Community Impact Technical Report

Assumed C3 ADTs

ſ	E2 Ru		E2 Subur	ban ADT
	Year	ADT	Year	ADT
ł	2017	5920	2017	7465
	2017	6016	2017	7583
	2018	6114	2018	7703
	2019	6214	2019	7825
	2020	6315	2020	7949
	2021	6417	2021	8075
			2022	8203
	2023	6522 6628	2025	8333
	2024	6736	2024	8355
	2025 2026	6756	2025	8599
	2026	6957	2028	8735
		7070	2027	8873
	2028			
	2029	7185	2029	9014
	2030	7302	2030	9157
	2031	7421	2031	9302
	2032	7541	2032	9449
	2033	7664	2033 2034	9598 9750
	2034	7789		
1	2035	7915	2035	9905
	2036	8044	2036	10062
	2037	8175	2037	10221
	2038	8308	2038	10383
	2039 2040	8443 8581	2039 2040	10547 10714
			2040	10714
	2041 2042	8720 8862	2041 2042	10884
	2042	9006	2042	11030
	2043	9008 9153	2043	11231
	2044	9302	2044	11405
	2045	9453	2045	11773
	2040	9607	2048	11960
	2048	9763	2048	12149
	2040	9922	2049	12342
	2050	10083	2050	12537
	2051	10247	2051	12736
	2052	10414	2052	12937
	2053	10583	2053	13142
	2054	10755	2054	13350
	2055	10930	2055	13562
	2056	11108	2056	13776
	2057	11289	2057	13994
	2058	11473	2058	14216
	2059	11659	2059	14441
	2060	11849	2060	14670
	2061	12042	2061	14902
	2062	12238	2062	15138
	2063	12437	2063	15378
	2064	12639	2064	15621
	2065	12845	2065	15869
	2066	13054	2066	16120

Assumed C3 Old US-95 Highway ADTs

Old US	-95 ADT	Jacksha	Rd. ADT	Zeitler F	Rd. ADT	Snow F	Rd. ADT
Year	ADT	Year	ADT	Year	ADT	Year	ADT
2017	220	2017	50	2017	50	2017	25
2018	220	2018	50	2018	50	2018	25
2019	221	2019	50	2019	50	2019	25
2020	221	2020	50	2020	50	2020	25
2021	222	2021	50	2021	50	2021	25
2022	222	2022	50	2022	50	2022	25
2023	223	2023	51	2023	51	2023	25
2023	223	2024	51	2024	51	2024	25
2025	223	2025	51	2025	51	2025	25
2026	224	2026	51	2026	51	2026	25
2027	224	2027	51	2027	51	2027	25
2028	225	2028	51	2028	51	2028	26
2029	225	2029	51	2029	51	2029	26
2029	225	2025	51	2030	51	2030	26
2030	226	2030	51	2030	51	2031	26
2031	226	2031	51	2032	51	2032	26
2032	220	2032	52	2033	52	2033	26
2033	227	2033	52	2033	52	2035	26
2034	227	2034	52	2035	52	2035	26
2035	228	2033	52	2035	52	2035	26
2036	228	2038	52	2030	52	2030	26
	229	2037	52	2037	52	2038	26
2038 2039	229	2038	52	2038	52	2038	26
2039	229	2039	52	2039	52	2035	26
		2040	52	2040	52	2040	26
2041	230	2041	52	2041	52	2041	26
2042	231	2042	53	2042	53	2042	26
2043 2044	231 232	2043	53	2043	53	2043	26
2044	232	2044	53	2044	53	2044	26
2045	232	2045	53	2045	53	2045	26
2046	233	2040	53	2040	53	2040	26
2047	233	2047	53	2047	53	2048	27
2048	233	2048	53	2048	53	2048	27
2049	234	2049	53	2050	53	2050	27
2050	234	2050	53	2050	53	2051	27
2051	235	2051	53	2051	53	2052	27
2052	235	2052	54	2052	54	2053	27
2054	236	2053	54	2053	54	2054	27
2055	230	2054	54	2055	54	2055	27
2056	237	2055	54	2056	54	2056	27
2050	237	2050	54	2057	54	2057	27
2057	237	2057	54	2058	54	2058	27
2058	238	2058	54	2059	54	2059	27
2059	238	2053	54	2059	54	2055	27
2060	239	2000	54	2000	54	2061	27
2061	239	2061	54	2062	54	2062	27
2062	240	2062	55	2063	55	2062	27
2063	240	2063	55	2064	55	2064	27
2064	241	2065	55	2065	55	2065	27
2065	241 242	2065	55	2065	55	2065	27
2000	242	2000		2000			

Skyview	Dr. ADT	Clyd	e Rd. ADT	Eid Ro	. ADT	Cameron	Rd. ADT
Year	ADT	Year	ADT	Year	ADT	Year	ADT
2017	25	2017	50	2017	65	2017	100
2018	25	2018	50	2018	65	2018	100
2019	25	2019	50	2019	65	2019	100
2020	26	2020	50	2020	65	2020	101
2021	26	2021	50	2021	65	2021	101
2022	26	2022	50	2022	66	2022	101
2023	26	2023	51	2023	66	2023	101
2024	27	2024	51	2024	66	2024	101
2025	27	2025	51	2025	66	2025	102
2026	27	2026	51	2026	66	2026	102
2027	27	2027	51	2027	66	2027	102
2028	28	2028	51	2028	66	2028	102
2029	28	2029	51	2029	67	2029	102
2030	28	2030	51	2030	67	2030	103
2031	28	2031	51	2031	67	2031	103
2032	29	2032	51	2032	67	2032	103
2033	29	2033	52	2033	67	2033	103
2035	29	2034	52	2034	67	2034	103
2035	29	2035	52	2035	67	2035	103
2035	30	2035	52	2036	67	2036	104
2030	30	2030	52	2037	68	2037	104
2037	30	2038	52	2038	68	2038	104
2038	30 31	2038	52	2030	68	2039	104
2035	31	2040	52	2040	68	2040	104
2040	31	2040	52	2041	68	2041	105
2041	31	2041	52	2041	68	2042	105
2042	32	2042	53	2042	68	2043	105
2043	32	2043	53	2044	68	2044	105
2044	32	2044	53	2045	69	2045	105
2045	33	2045	53	2046	69	2046	106
2040	33	2040	53	2047	69	2047	106
2047	33	2048	53	2048	69	2048	106
2048	33	2049	53	2049	69	2049	106
2050	34	2050	53	2050	69	2050	106
2050	34	2051	53	2051	69	2051	107
2052	34	2052	53	2052	69	2052	107
2052	35	2053	54	2053	70	2053	107
2054	35	2054	54	2054	70	2054	107
2054	35	2055	54	2055	70	2055	108
2056	36	2056	54	2056	70	2056	108
2057	36	2057	54	2057	70	2057	108
2058	36	2057	54	2058	70	2058	108
2059	37	2059	54	2059	70	2059	108
2060	37	2060	54	2060	71	2060	109
2061	37	2061	54	2061	71	2061	109
2062	38	2062	54	2062	71	2062	109
2062	38	2063	55	2063	71	2063	109
2064	38	2064	55	2064	71	2064	109
2065	39	2065	55	2065	71	2065	110
2066	39	2066	55	2066	71	2066	110

r.

ADT	ADT Volume Projection Report	ne P	rojec	tion F	Repo	rt							
	71	Route L	US095					Traf	Traffic Data	2010			
	Segment From	_	1539	Milepost From	From	337.180		Start Projection	ojection	2017			
	Segment To	-	1540	Milepost To	st To	342.930		End Pr	End Projection	2037			
	Segment	ment	Milepost	oost									
Year	From	To	From	То	AADT	CAADT	DHV	DHV %	CDHV	CDHV %	DIR	From Description	To Description
2010	001539	001539	337.180 337.668	337,668	4,900	650	567	11.5	53	8.106	60/40%	THORN CREEK RD	END NEW ALIGNMENT
	001540	001540	337.668	339.620	4,900	680	567	11.5	55	8.106	60/40%	END NEW ALIGNMENT	EID RD
			339.620	342.930	5,300	089	611	11.5	55	8.071	60/40%	EID RD	
	2010		Weighted	hted	5,130	677	593	11.50	55	8.07			
2017	001539	001539	337.180	337.180 337.668	5,654	608	650	11.4	65	8.044	60/40%	THORN CREEK RD	END NEW ALIGNMENT
	001540	001540	337.668	339,620	5,657	847	650	11.4	89	8.044	60/40%	END NEW ALIGNMENT	EID RD
			339.620	342.930	6,113	847	700	11.4	89	8.014	60/40%	EID RD	
	2017		Weighted	hted	5,920	843	679	11.40	89	8.01			
2037	001539	001539	337.180	337,180 337,668	7,809	1,264	885	11.3	100	7.934	60/40%	THORN CREEK RD	END NEW ALIGNMENT
	001540	001540	337.668	339.620	7,821	1,323	988	11.3	105	7.933	60/40%	END NEW ALIGNMENT	EID RD
			339,620	342.930	8,437	1,323	954	11.3	105	7.912	60/40%	EID RD	
	2037		Weighted	hted	8,175	1,318	925	11.30	104	7.91			
J 16, 2011													

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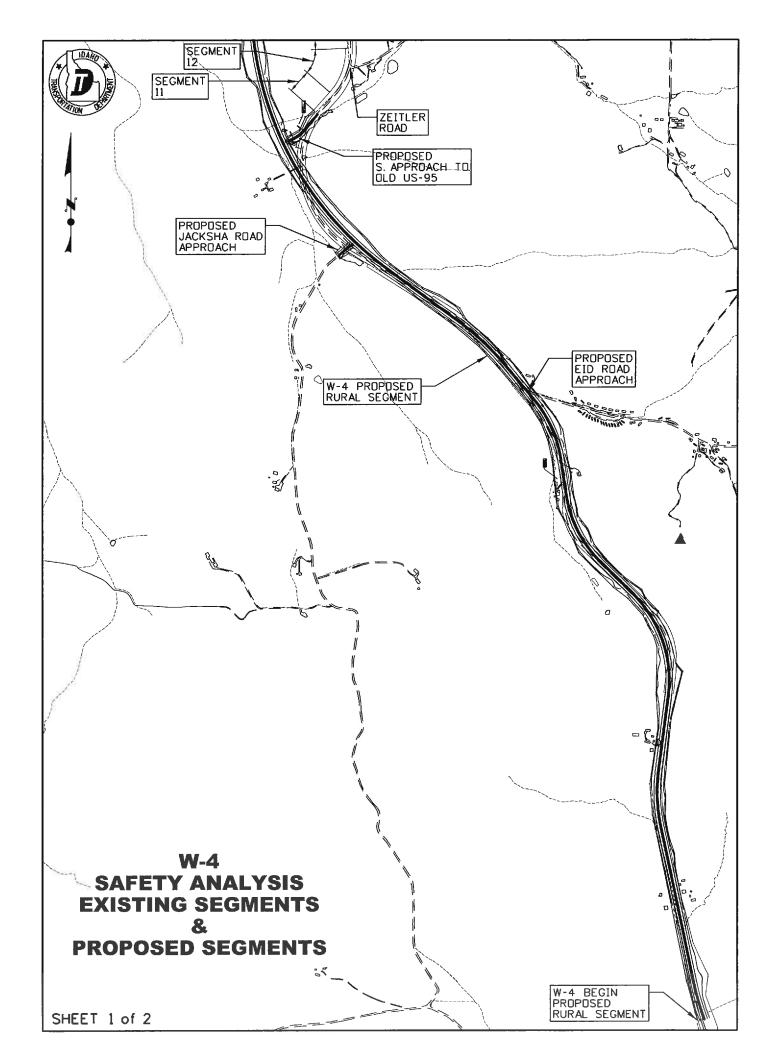
	2037 001540 342.933	2017 Weighted averages	2017 001540 342.933	2010 Weighted averages	2010 001540 001540 342.933	Segment M Year From To From	Segment To 1540	Segment From 1540	Route US095	AD1 Volume Projection Report
	33 344,116	verages	33 344.116	verages	33 344.116	Milepost m To	Milepost To	Milepost From		on Repo
10,221	10,221	7,465	7,465	6,500	6,500.	AADT	ist To			F
10,221 1,011 1,148 11.2	1,011	647	647	520	520	CAADT	344.11	342.93		
1,148	1,148	848	848	742	742	DHV				
11.2	11.2	848 11.3	11.3	742 11.4	11.4	DHV	End P	Start Projection 2017	Tra	
80	80	51	51	42	42	CDHV	End Projection 2037	rojectio	Traffic Data 2010	
80 7.87	80 7.865	51 7.95	51 7.947	42 7.99	7.992	HV CDHV	n 2037	n 2017	a 2010	
	60/40%		60/40%		60/40%	DIR				
	60/40% CLYDE RD		60/40% CLYDE RD		CLYDE RD	From Description				
	PALOUSE RIVER DR		PALOUSE RIVER DR		PALOUSE RIVER DR	To Description				

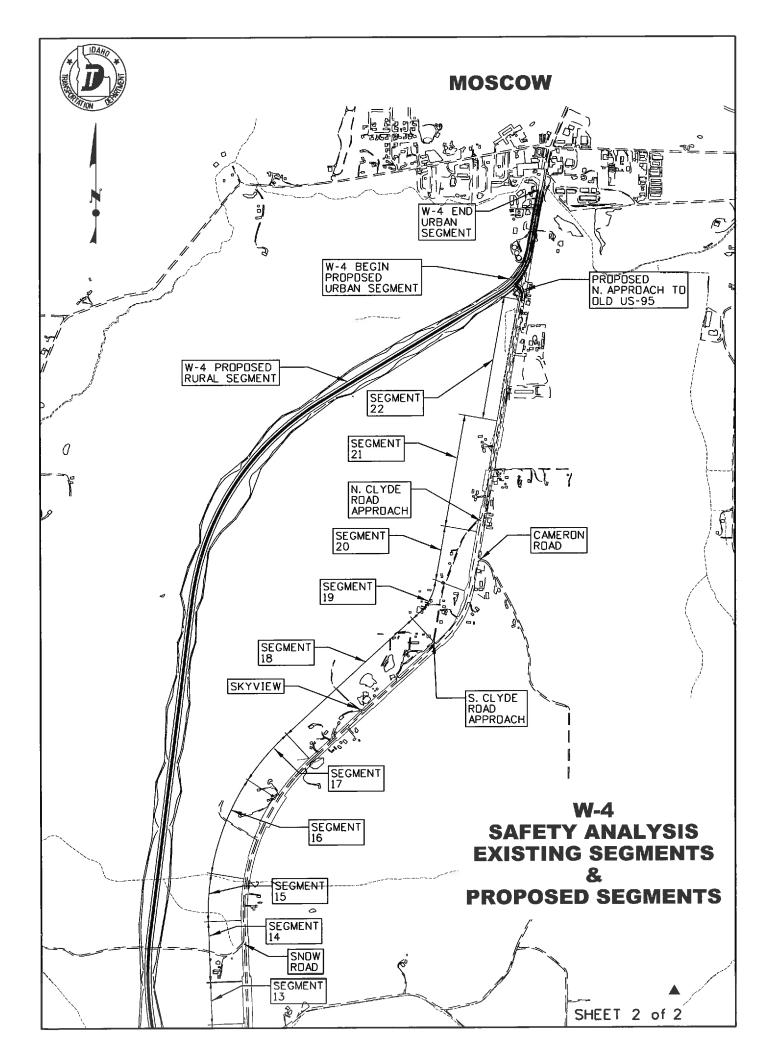
June 16, 2011

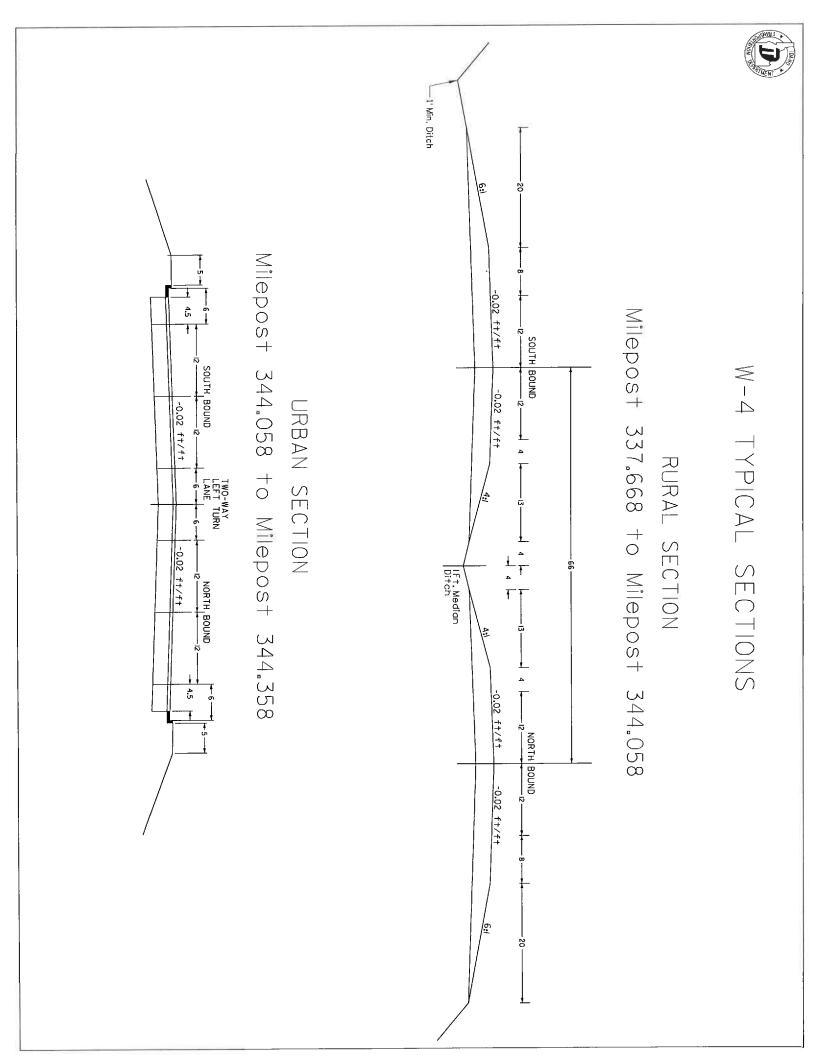
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Appendix C.3

Segment Breakdown, Proposed Typical Sections, and Predictions For Alternative W4







Predicted 2017 ADT for Proposed US-95 and County Roads for Safety Analysis

<u>Rural US-95</u> – **5920** From ITD Traffic Volume Report <u>Suburban US-95</u> – **7465** From ITD Traffic Volume Report <u>Eid Road</u> – **65** – From Current North Latah Highway District Plan <u>Jacksha Road</u> – **50** – Volume is predicted to be less than Eid Road Volume <u>Zeitler Road</u> – **50** – Volume is predicted to be less than Eid Road Volume <u>Snow Road</u> – **25** – Volume is predicted to be very light due to few residences <u>Skyview Drive</u> – **25** – The private drive currently services only 1 home <u>North Clyde Road</u> – **50** – Volume predicted to be less than Eid Road Volume <u>Cameron Road</u> – **100** – Volume predicted to be greater than other rural roads based on proximity to Moscow and the number of homes South Clyde Road – **50** – Volume of traffic predicted to be light and less than Eid Road Volume

Predicted 2017 ADT on Existing US-95 Loop If Alternative W4 Was Constructed

Segments 11-19-200 (For Predictions See Below)

18 Approaches generate 10 trips per day = 180 Zeitler Road = 50, Snow Road = 25, Skyline Drive = 25, Clyde Road South = 50 / Total = 150

Predict an Even Directional Split -(180+150)/2 = 165

Predict about 35 commuters that drive through on Old US-95 per day – 165 + 35 = 200

Segments 20-22 - 1545 from Differences in ITD Traffic Volume Report (7465 – 5920 = 1545)

Predicted Traffic Growth Rates

<u>Proposed US-95 Rural</u> – 1.63% From ITD Traffic Volume Report
<u>Proposed US-95 Suburban</u> – 1.58% From ITD Traffic Volume Report
<u>Segment 22 on Existing US-95 Loop</u> – 0.91% Growth Rate of Moscow from Latah County Comprehensive
Plan because the segment is within the area of impact of Moscow
<u>Segments 20,21 on Existing US-95 Loop</u> – 0.53% Growth Rate of Unincorporated Latah County from
Latah County Comprehensive Plan because the segments are near the area of impact of Moscow
<u>Segments 11-19</u> – 0.19% Growth Rate within the Thorncreek to Moscow Corridor based on Census
Bureau Data in the Thorncreek to Moscow – This is a new subdivision so growth is predicted to be
greater than other areas within the corridor
All County Roads Except Skyline Drive – 0.19% Growth Rate within the Thorncreek to Moscow Corridor based on Census

Assumed W4 ADTs

W4 Ru	ral ADT	W4 Subu	rban ADT	W4 Segm	ent 22 ADT	W4 Segmer	1t 21,20 ADT
Year	ADT	Year	ADT	Year	ADT	Year	ADT
2017	5920	2017	7465	2017	1545	2017	1545
2018	6016	2018	7583	2018	1559	2018	1553
2019	6114	2019	7703	2019	1573	2019	1561
2020	6214	2020	7825	2020	1588	2020	1570
2021	6315	2021	7949	2021	1602	2021	1578
2022	6417	2022	8075	2022	1617	2022	1586
2023	6522	2023	8203	2023	1631	2023	1594
2024	6628	2024	8333	2024	1646	2024	1603
2025	6736	2025	8465	2025	1661	2025	1611
2026	6845	2026	8599	2026	1677	2026	1620
2020	6957	2027	8735	2027	1692	2027	1628
2027	7070	2028	8873	2028	1707	2028	1637
2028	7185	2028	9014	2029	1723	2029	1646
2029	7302	2025	9157	2020	1738	2030	1654
	7421	2030	9302	2030	1754	2031	1663
2031	7541	2031	9502 9449	2031	1734	2031	1672
2032				2032	1786	2032	1680
2033	7664	2033	9598 9750	2033	1803	2033	1689
2034	7789	2034				2034	1698
2035	7915	2035	9905	2035	1819		
2036	8044	2036	10062	2036	1836	2036	1707
2037	8175	2037	10221	2037	1853	2037	1716
2038	8308	2038	10383	2038	1869	2038	1725
2039	8443	2039	10547	2039	1886	2039	1734
2040	8581	2040	10714	2040	1904	2040	1743
2041	8720	2041	10884	2041	1921	2041	1753
2042	8862	2042	11056	2042	1939	2042	1762
2043	9006	2043	11231	2043	1956	2043	1771
2044	9153	2044	11409	2044	1974	2044	1780
2045	9302	2045	11590	2045	1992	2045	1790
2046	9453	2046	11773	2046	2010	2046	1799
2047	9607	2047	11960	2047	2029	2047	1809
2048	9763	2048	12149	2048	2047	2048	1818
2049	9922	2049	12342	2049	2066	2049	1828
2050	10083	2050	12537	2050	2085	2050	1837
2051	10247	2051	12736	2051	2104	2051	1847
2052	10414	2052	12937	2052	2123	2052	1857
2053	10583	2053	13142	2053	2142	2053	1867
2054	10755	2054	13350	2054	2162	2054	1876
2055	10930	2055	13562	2055	2181	2055	1886
2056	11108	2056	13776	2056	2201	2056	1896
2057	11289	2057	13994	2057	2221	2057	1906
2058	11473	2058	14216	2058	2242	2058	1916
2059	11659	2059	14441	2059	2262	2059	1926
2060	11849	2060	14670	2060	2283	2060	1937
2061	12042	2061	14902	2061	2303	2061	1947
2062	12238	2062	15138	2062	2324	2062	1957
2063	12437	2063	15378	2063	2346	2063	1967
2064	12639	2064	15621	2064	2367	2064	1978
2065	12845	2065	15869	2065	2389	2065	1988
2066	13054	2066	16120	2066	2410	2066	1999

Assumed W4 Old US-95 ADTs

Old US	-95 ADT	Jacksha	Rd. ADT		Zeitler	Rd. ADT		Snow F	d. ADT
Year	ADT	Year	ADT		Year	ADT		Year	ADT
2017	200	2017	50		2017	50		2017	25
2018	200	2018	50		2018	50		2018	25
2019	201	2019	50		2019	50		2019	25
2020	201	2020	50		2020	50		2020	25
2021	202	2021	50		2021	50		2021	25
2022	202	2022	50		2022	50		2022	25
2023	202	2023	51		2023	51		2023	25
2024	203	2024	51		2024	51		2024	25
2025	203	2025	51		2025	51		2025	25
2026	203	2026	51		2026	51		2026	25
2020	203	2027	51		2027	51		2027	25
2028	204	2028	51		2028	51		2028	26
2028	204	2029	51		2029	51		2029	26
2029	205	2020	51		2030	51		2030	26
2030	205	2030	51		2030	51		2031	26
2031	203	2031	51		2031	51		2031	26
2032	206	2032	52		2032	52		2032	26
2033	208	2033	52		2033	52		2035	26
2034	207	2034	52		2034	52		2034	26
	1 1	2035	52		2035	52		2035	26
2036	207				2036	52		2030	26
2037	208	2037	52			52 52		2037	26
2038	208	2038	52		2038			2038	
2039	209	2039	52		2039	52		2039	26
2040	209	2040	52		2040	52			26
2041	209	2041	52		2041	52		2041	26
2042	210	2042	52		2042	52		2042	26
2043	210	2043	53		2043	53		2043	26
2044	211	2044	53		2044	53		2044	26
2045	211	2045	53		2045	53		2045	26
2046	211	2046	53		2046	53		2046	26
2047	212	2047	53		2047	53		2047 2048	26
2048	212	2048	53		2048	53		2048 2049	27
2049	213	2049	53		2049	53 52		2049	27 27
2050	213	2050	53		2050	53	•	2050	27
2051	213	2051	53		2051	53			27
2052	214	2052	53		2052	53	İ	2052	
2053	214	2053	54		2053 2054	54 54		2053 2054	27 27
2054	215	2054	54						
2055	215	2055	54		2055	54		2055	27 7
2056	215	2056	54		2056	54		2056	27
2057	216	2057	54		2057	54		2057	27
2058	216	2058	54		2058	54		2058	27
2059	217	2059	54		2059	54		2059	27
2060	217	2060	54		2060	54 54		2060	27
2061	217	2061	54		2061	54		2061	27
2062	218	2062	54		2062	54		2062	27
2063	218	2063	55		2063	55		2063	27
2064	219	2064	55	1	2064	55		2064	27
2065	219	2065	55		2065	55		2065	27
2066	220	2066	55		2066	55		2066	27

Skyview	Dr. ADT	Civde I	Rd. ADT		Eid Ro	I. ADT	Cameron	Rd. ADT
Үеаг	ADT	Year	ADT	[]	Year	ADT	Year	ADT
2017	25	2017	50		2017	65	2017	100
2018	25	2018	50		2018	65	2018	100
2019	25	2019	50		2019	65	2019	100
2020	26	2020	50		2020	65	2020	101
2021	26	2021	50		2021	65	2021	101
2022	26	2022	50		2022	66	2022	101
2023	26	2023	51		2023	66	2023	101
2024	27	2024	51		2024	66	2024	101
2025	27	2025	51		2025	66	2025	102
2026	27	2026	51		2026	66	2026	102
2027	27	2027	51		2027	66	2027	102
2028	28	2028	51		2028	66	2028	102
2029	28	2029	51		2029	67	2029	102
2030	28	2030	51		2030	67	2030	103
2031	28	2031	51		2031	67	2031	103
2032	29	2032	51		2032	67	2032	103
2033	29	2033	52		2033	67	2033	103
2034	29	2034	52		2034	67	2034	103
2035	29	2035	52		2035	67	2035	103
2036	30	2036	52		2036	67	2036	104
2037	30	2037	52		2037	68	2037	104
2038	30	2038	52		2038	68	2038	104
2039	31	2039	52		2039	68	2039	104
2040	31	2040	52		2040	68	2040	104
2041	31	2041	52		2041	68	2041	105
2042	31	2042	52		2042	68	2042	105
2043	32	2043	53		2043	68	2043	105
2044	32	2044	53		2044	68	2044	105
2045	32	2045	53		2045	69	2045	105
2046	33	2046	53		2046	69	2046	106
2047	33	2047	53		2047	69	2047	106
2048	33	2048	53		2048	69	2048	106
2049	33	2049	53		2049	69	2049	106
2050	34	2050	53		2050	69	2050	106
2051	34	2051	53		2051	69	2051	107
2052	34	2052	53		2052	69	2052	107
2053	35	2053	54		2053	70	2053	107
2054	35	2054	54		2054	70	2054	107
2055	35	2055	54		2055	70	2055	108
2056	36	2056	54		2056	70	2056	108
2057	36	2057	54		2057	70	2057	108
2058	36	2058	54		2058	70	2058	108
2059	37	2059	54		2059	70	2059	108
2060	37	2060	54		2060	71	2060	109
2061	37	2061	54 54		2061	71	2061	109 109
2062	38	2062	54		2062	71 71	2062	109
2063	38	2063	55		2063	71 71	2063	109 109
2064	38	2064 2065	55		2064 2065	71 71	2064 2065	109 110
2065 2066	39 39	2065	55 55		2065	71	2065	110
2000	59	2000	<u>در</u>		2000	/1	2000	110

J 16, 2011				2037				2017				2010	Year					ADT
5	2037		001540	001539	2017		001540	001539	2010		001540	001539	From	Segment	Segment To	Segment From	п	ADT Volume Projection Report
			001540	001539			001540	001539			001540	001539	5	nent			Route	ne P
	Weig	339.620	337.668	337.180	Weig	339.620	337.668	337.180	Weig	339.620	337.668	337.180	From	Mile	1540	1539	US095	rojec
	Weighted	342.930	339.620	337.180 337.668	Weighted	342.930	339.620	337.180 337.668	Weighted	342.930	339.620	337.180 337.668	ъ	Milepost	Milep	Milepost From		tion
	8,175	8,437	7,821	7,809	5,920	6,113	5,657	5,654	5,130	5,300	4,900	4,900	AADT		Milepost To	t From		Repo
	1.3		1	1,					0				CAADT		342.930	337.180		ň
	1,318	1,323	1,323	1,264	843 (847	847	809	677 (680	680	650	DHV		ö	ö		
	925	954	886	885	679	700	650	650	593	611	567	567						
	11.30	11.3	11.3	11.3	11.40	11.4	11.4	11.4	11.50	11.5	11.5	11.5	DHV % (End Projection 2037	Start Projection	Traf	
	104	105	105	100	89	68	68	65	55	55	55	53	CDHV		ojectior	ojectior	Traffic Data	
in the second	7.91	7.912	7.933	7.934	8.01	8.014	8.044	8.044	8.07	8.071	8.106	8.106	CDHV %		1 2037		1 2010	
5		60/40%	60/40%	60/40%		60/40%	60/40%	60/40%		60/40%	60/40%	60/40%	DIR					
:		EID RD	END NEW ALIGNMENT	THORN CREEK RD		EID RD	END NEW ALIGNMENT	THORN CREEK RD		EID RD	END NEW ALIGNMENT	THORN CREEK RD	From Description					
Page 1 of 1			EID RD	END NEW ALIGNMENT			EID RD	END NEW ALIGNMENT			EID RD	END NEW ALIGNMENT	To Description					Ĩ

DT V 2010	Olume Pr Route Segment From Segment To From To 001540 001540	ume Project Route US095 syment From 1540 Segment To 1540 Segment From To Fro	ADT Volume Projection Report Route US095 Segment From 1540 Milepost Fro Segment To 1540 Milepost Year From To From To Av 2010 001540 342.933 344.116	ion Report Milepost From 342.93 Milepost To 344.11 Milepost <u>To AADT CAAD</u> 2.933 344.116 6,500. 5	eport lepost From 342.93 Milepost To 344.11 <u>A.116 6,500</u> <u>6</u>	342.93 344.11 CAADT 520	DHV 742	Tra Start P End P <u>11.4</u>	affic Da Projecti Project	<u> </u>	Traffic Data 2010 rt Projection 2017 d Projection 2037 d CDHV CDHV .4 42 7.992		81
00154	00154	5 0	0 342.933 344 Weighted averages	344.116 rages	6,500. 6,500		742 742	11.4		7.992 7.99	60/40%	CLYDE RD	
		001540	342.933	344.116	7,465	647	848	11.3	5	51 7.947		60/40% CLYDE RD	
	2017	×	Weighted averages	rages	7,465	647	848	11.3	51	7.95			
		001540	342.933	344.116	10,221	1,011	1,148	11.2	80	80 7.865	60/40%	60/40% CLYDE RD	ō
1	2037	5	Weighted averages	brages	10,221	10,221 1,011 1,148 11.2	1,148	11.2	80	80 7.87			

June 16, 2011

Appendix D

Economic Cost of Crashes

- Economic Cost of Predicted Crashes on Alternative E2
- Economic Cost of Predicted Crashes on Alternative C3
- Economic Cost of Predicted Crashes on Alternative E4
- Economic Cost of Predicted Crashes No Action Alternative
 - Economic Cost Relating to Wild Animal Crashes
- Percentages and Factors Used to Determine Economic Costs of Alternatives and Wild Animal Crashes

Economic Cost of Predicted Crashes on Alternative E2

Total Predicted Crashes on Alternative E2:	179.5
Total Predicted Fatal and Injury Crashes on Alternative E2:	89

	Estimated	Economic Cos	t of Crashes on	Alternative E2 Between	2017 and 2036	
Crash Type	Crashes	Percentage of Crashes	Multiple Car Multiplier	Fatalities or Injuries	2012 FHWA Cost of Crashes	Total Cost
Fatality	1.93	1.1%	1.09	2.11	\$6,295,406	\$13,252,566.19
Type A	11.88	6.6%	1.24	14.73	\$313,516	\$4,619,050.04
Туре В	29.39	16.4%	1.33	39.09	\$87,814	\$3,432,278.16
Type C	45.81	25.5%	1.56	71.46	\$58,209	\$4,159,670.32
Property Damage	90.5	50.4%	1.00	0.00	\$6,739	\$609,879.50
Total:	179.5	100.0%				\$26,073,444.21

Total Predicted Crashes on Alternative E2 and US-95 Loop:	213.9
Total Predicted Fatal and Injury Crashes on Alternative E2 and US-95 Loop:	100.7

Estimate	d Economic	Cost of Crashe	es on Alternati	ve E2 and the US-95 Loop	Between 2017 and	2036
Crash Type	Crashes	Percentage of Crashes	Multiple Car Multiplier	Fatalities or Injuries	2012 FHWA Cost of Crashes	Total Cost
Fatality	2.19	1.0%	1.09	2.38	\$6,295,406	\$14,994,757.48
Type A	13.44	6.3%	1.24	16.67	\$313,516	\$5,226,273.47
Type B	33.25	15.5%	1.33	44.22	\$87,814	\$3,883,487.76
Type C	51.83	24.2%	1.56	80.86	\$58,209	\$4,706,503.39
Property Damage	113.2	52.9%	1.00	0.00	\$6,739	\$762,854.80
Total:	213.9	100.0%				\$29,573,876.90

Economic Cost of Predicted Crashes on Alternative E2

Total Predicted Crashes on Alternative E2:	7.6
Total Predicted Fatal and Injury Crashes on Alternative E2:	3.8

	Est	timated Econo	omic Cost of Cra	shes on Alternative E2 Fe	or 2017	
Crash Type	Predicted Crashes	Percentage of Crashes	Multiple Car Multiplier	Predicted Fatalities or Injuries	2012 FHWA Cost of Crashes	Total Cost
Fatality	0.08	1.1%	1.09	0.09	\$6,295,406	\$565,839.90
Type A	0.51	6.7%	1.24	0.63	\$313,516	\$197,217.87
Type B	1.25	16.5%	1.33	1.67	\$87,814	\$146,546.71
Type C	1.96	25.7%	1.56	3.05	\$58,209	\$177,603.90
Property Damage	3.8	50.0%	1.00	0.00	\$6,739	\$25,608.20
Total:	7.6	100.0%				\$1,112,816.58

Total Predicted Crashes on Alternative E2 and US-95 Loop:	9.2
Total Predicted Fatal and Injury Crashes on Alternative E2 and US-95 Loop:	4.4

	Estimated Economic Cost of Crashes on Alternative E2 and the US-95 Loop For 2017								
Crash Type	Predicted Crashes	Percentage of Crashes	Multiple Car Multiplier	Predicted Fatalities or Injuries	2012 FHWA Cost of Crashes	Total Cost			
Fatality	0.10	1.0%	1.09	0.10	\$6,295,406	\$655,183.05			
Type A	0.59	6.4%	1.24	0.73	\$313,516	\$228,357.53			
Type B	1.45	15.8%	1.33	1.93	\$87,814	\$169,685.66			
Type C	2.26	24.6%	1.56	3.53	\$58,209	\$205,646.62			
Property Damage	4.8	52.2%	1.00	0.00	\$6,739	\$32,347.20			
Total:	9.2	100.0%				\$1,291,220.06			

Economic Cost of Predicted Crashes on Alternative C3

Total Predicted Crashes on Alternative C3:	253.8
Total Predicted Fatal and Injury Crashes on Alternative C3:	107.7

	Estimated	Economic Cos	t of Crashes or	Alternative C3 Between	2017 and 2036	
Crash Type	Crashes	Percentage of Crashes	Multiple Car Multiplier	Fatalities or Injuries	2012 FHWA Cost of Crashes	Total Cost
Fatality	2.3694	0.9%	1.09	2.58	\$6,295,406	\$16,258,805.12
Type A	14.70105	5.7%	1.24	18.23	\$313,516	\$5,715,177.85
Type B	36.34875	14.2%	1.33	48.34	\$87,814	\$4,245,265.75
Type C	56.6502	22.1%	1.56	88.37	\$58,209	\$5,144,180.33
Property Damage	146.1	57.0%	1.00	0.00	\$6,739	\$984,567.90
Total:	256.1694	100.0%				\$32,347,996.94

Total Predicted Crashes on Alternative C3 and US-95 Loop:	260.2
Total Predicted Fatal and Injury Crashes on Alternative C3 and US-95 Loop:	110

Estimate	Estimated Economic Cost of Crashes on Alternative C3 and the US-95 Loop Between 2017 and 2036								
Crash Type	Crashes	Percentage of Crashes	Multiple Car Multiplier	Fatalities or Injuries	2012 FHWA Cost of Crashes	Total Cost			
Fatality	2.42	0.9%	1.09	2.64	\$6,295,406	\$16,606,021.95			
Туре А	15.015	5.7%	1.24	18.62	\$313,516	\$5,837,229.00			
Type B	37.125	14.1%	1.33	49.38	\$87,814	\$4,335,926.02			
Туре С	57.86	22.0%	1.56	90.26	\$58,209	\$5,254,037.47			
Property Damage	150.2	57.2%	1.00	0.00	\$6,739	\$1,012,197.80			
Total:	262.62	100.0%				\$33,045,412.24			

Economic Cost of Predicted Crashes on Alternative C3

Total Predicted Crashes on Alternative C3:	10.8
Total Predicted Fatal and Injury Crashes on Alternative C3:	4.6

	Estimated Economic Cost of Crashes on Alternative C3 For 2017								
Crash Type	Predicted Crashes	Percentage of Crashes	Multiple Car Multiplier	Predicted Fatalities or Injuries	2012 FHWA Cost of Crashes	Total Cost			
Fatality	0.1012	0.9%	1.09	0.11	\$6,295,406	\$694,433.65			
Type A	0.6279	5.8%	1.24	0.78	\$313,516	\$244,102.30			
Type B	1.5525	14.2%	1.33	2.06	\$87,814	\$181,320.54			
Туре С	2.4196	22.2%	1.56	3.77	\$58,209	\$219,714.29			
Property Damage	6.2	56.9%	1.00	0.00	\$6,739	\$41,781.80			
Total:	10.9012	100.0%				\$1,381,352.59			

Total Predicted Crashes on Alternative C3 and US-95 Loop:	11.1
Total Predicted Fatal and Injury Crashes on Alternative C3 and US-95 Loop:	4.7

Estimated Economic Cost of Crashes on Alternative C3 and the US-95 Loop For 2017								
Crash Type	Predicted Crashes	Percentage of Crashes	Multiple Car Multiplier	Predicted Fatalities or Injuries	2012 FHWA Cost of Crashes	Total Cost		
Fatality	0.1034	0.9%	1.09	0.11	\$6,295,406	\$709,530.03		
Type A	0.64155	5.7%	1.24	0.80	\$313,516	\$249,408.88		
Type B	1.58625	14.2%	1.33	2.11	\$87,814	\$185,262.29		
Type C	2.4722	22.1%	1.56	3.86	\$58,209	\$224,490.69		
Property Damage	6.4	57.1%	1.00	0.00	\$6,739	\$43,129.60		
Total:	11.2034	100.0%				\$1,411,821.49		

Economic Cost of Predicted Crashes on Alternative W4

Total Predicted Crashes on Alternative W4:	219.3
Total Predicted Fatal and Injury Crashes on Alternative W4:	107.7

Estimated Economic Cost of Crashes on Alternative W4 Between 2017 and 2036							
Crash Type	Crashes	Percentage of Crashes	Multiple Car Multiplier	Fatalities or Injuries	2012 FHWA Cost of Crashes	Total Cost	
Fatality	2.3694	1.1%	1.09	2.58	\$6,295,406	\$16,258,805.12	
Type A	14.70105	6.6%	1.24	18.23	\$313,516	\$5,715,177.85	
Type B	36.34875	16.4%	1.33	48.34	\$87,814	\$4,245,265.75	
Type C	56.6502	25.6%	1.56	88.37	\$58,209	\$5,144,180.33	
Property Damage	111.6	50.3%	1.00	0.00	\$6,739	\$752,072.40	
Total:	221.6694	100.0%				\$32,115,501.44	

Total Predicted Crashes on Alternative W4 and US-95 Loop:246.2Total Predicted Fatal and Injury Crashes on Alternative W4 and US-95 Loop:116.9

Estimate	Estimated Economic Cost of Crashes on Alternative W4 and the US-95 Loop Between 2017 and 2036								
Crash Type	Crashes	Percentage of Crashes	Multiple Car Multiplier	Fatalities or Injuries	2012 FHWA Cost of Crashes	Total Cost			
Fatality	2.5718	1.0%	1.09	2.80	\$6,295,406	\$17,647,672.41			
Type A	15.95685	6.4%	1.24	19.79	\$313,516	\$6,203,382.45			
Type B	39.45375	15.9%	1.33	52.47	\$87,814	\$4,607,906.83			
Type C	61.4894	24.7%	1.56	95.92	\$58,209	\$5,583,608.92			
Property Damage	129.3	52.0%	1.00	0.00	\$6,739	\$871,352.70			
Total:	248.7718	100.0%				\$34,913,923.31			

Economic Cost of Predicted Crashes on Alternative W4

Total Predicted Crashes on Alternative W4:	9.3
Total Predicted Fatal and Injury Crashes on Alternative W4:	4.6

	Esti	mated Econor	mic Cost of Cra	shes on Alternative W4 F	or 2017	
Crash Type	Predicted Crashes	Percentage of Crashes	Multiple Car Multiplier	Predicted Fatalities or Injuries	2012 FHWA Cost of Crashes	Total Cost
Fatality	0.1012	1.1%	1.09	0.11	\$6,295,406	\$694,433.65
Type A	0.6279	6.7%	1.24	0.78	\$313,516	\$244,102.30
Туре В	1.5525	16.5%	1.33	2.06	\$87,814	\$181,320.54
Type C	2.4196	25.7%	1.56	3.77	\$58,209	\$219,714.29
Property Damage	4.7	50.0%	1.00	0.00	\$6,739	\$31,673.30
Total:	9.4012	100.0%				\$1,371,244.09

Total Predicted Crashes on Alternative W4 and US-95 Loop:	10.5
Total Predicted Fatal and Injury Crashes on Alternative W4 and US-95 Loop:	

E	stimated Eco	onomic Cost o	f Crashes on Al	ternative W4 and the US-	95 Loop For 2017	
Crash Type	Predicted Crashes	Percentage of Crashes	Multiple Car Multiplier	Predicted Fatalities or Injuries	2012 FHWA Cost of Crashes	Total Cost
Fatality	0.1122	1.1%	1.09	0.12	\$6,295,406	\$769,915.56
Type A	0.69615	6.6%	1.24	0.86	\$313,516	\$270,635.16
Туре В	1.72125	16.2%	1.33	2.29	\$87,814	\$201,029.30
Type C	2.6826	25.3%	1.56	4.18	\$58,209	\$243,596.28
Property Damage	5.4	50.9%	1.00	0.00	\$6,739	\$36,390.60
Total:	10.6122					\$1,521,566.91

Economic Cost of Wild Animal Crashes

Extra Wild Animal Crashes (1 Per Year for 20 Years):

20

Crash Type	Crashes	Percentage of Crashes	Multiple Car Multiplier	Fatalities or Injuries	2012 FHWA Cost of Crashes	Total Cost
Crash Type	Number	Percentage	Multiplier	Fatalities and Injuries	Cost of Crashes	Total Cost
Fatality	0	0.00%	1.00	0.00	\$6,295,406	\$0.00
Type A	0.12	0.60%	1.00	0.12	\$313,516	\$37,621.92
Type B	0.46	2.30%	1.45	0.67	\$87,814	\$58,571.94
Type C	1.31	6.55%	1.26	1.65	\$58,209	\$96,079.78
Property Damage	18.11	90.55%	1.00		\$6,739	\$122,043.29
Total:	20	100.0%				\$314,316.92

Extra Wild Animal Crashes: 2

223

How Many Wild	l Animal Cras	shes Would Be		Nake Economic Cost of Cra ative C3	ashes for Alternative	E2 Equal to
Crash Type	Crashes	Percentage of Crashes	Multiple Car Multiplier	Fatalities or Injuries	2012 FHWA Cost of Crashes	Total Cost
Fatality	0	0.00%	1.00	0.00	\$6,295,406	\$0.00
Type A	1.338	0.60%	1.00	1.34	\$313,516	\$419,484.41
Type B	5.129	2.30%	1.45	7.44	\$87,814	\$653,077.11
Type C	14.6065	6.55%	1.26	18.40	\$58,209	\$1,071,289.50
Property Damage	201.9265	90.55%	1.00		\$6,739	\$1,360,782.68
Total:	223	100.0%				\$3,504,633.70

*The economic cost of crashes estimated is based on crash costs from the Office of Highway Safety Publication titled Idaho Highway Crashes 2012. The percentage of fatal and injury crashes and property damage only crashes, and the factor increasing the number of fatalities or injuries is based on wild animal crash data along US-95 in District 2 between 1/1/03 and 12/31/12.

Percentages and Factors Used to Determine Economic Costs of Alternatives and Wild Animal Crashes

	Number of Crashes of	Percentage of Crash
	Different Severity From	Type Compared to Total
	ITD Database	Fatal and Injury* Crashes
Fatal Crashes	169	2.17%
A Crashes	1042	13.35%
B Crashes	2577	33.02%
C Crashes	4017	51.47%
Total F+I Crashes	7805	100.00%

*The percentage of crash type compared to total Fatal and Injury Crashes is used to help estimated the percentage of Fatal and Injury Crashes on the different alternatives

	Total Number of Injuries or Fatalities in Idaho in 2012*	Crash Multiplication Factor**
Total Fatalities (2012)	184	1.09
Total A Injuries (2012)	1287	1.24
Total B Injuries (2012)	3428	1.33
Total C Injuries (2012)	6273	1.56
Total F+I (2012)	11172	

*This Number is from Table 3 of Section 1 of Idaho Traffic Crashes 2012 by the Idaho Transportation Department Office of Highway Safety

**The Crash Multiplication Factor is the total number of the injuries or fatalities of a given crash type divided by the total number of crashes listed with the corresponding severity level. The Crash Multiplication Factor is used to account for numerous injuries in a given crash event and is used to help estimate the estimated economic cost of crashes for different accident types.

	Crashes Resulting in a Fatal or Injury Accident Caused By Animal Crashes Between 1/03 and 12/31	Injuries or Fatalities for Wild Animal Crashes Between 1/03 and 12/31	Crash Multiplication Factor**
Total Fatalities (2012)	0	0	0.00
Total A Injuries (2012)	3	3	1.00
Total B Injuries (2012)	11	16	1.45
Total C Injuries (2012)	31	39	1.26
Total F+I (2012)	45	58	

*This Number is from Table 3 of Section 1 of Idaho Traffic Crashes 2012 by the Idaho Transportation Department Office of Highway Safety

**The Crash Multiplication Factor is the total number of the injuries or fatalities of a given crash type divided by the Total number of crashes listed with the corresponding severity level. The Crash Multiplication Factor is used to account for numerous injuries in a given crash event and is used to help estimate the estimated economic cost of crashes for different accident types.

Economic Cost of No Action Alternative

Economi	c Cost of No
Action A	Alternative
Year	Cost Per Year
2013	\$5,613,549
2014	\$5,705,050
2015	\$5,798,042
2016	\$5,892,550
2017	\$5,988,599
2018	\$6,086,213
2019	\$6,185,418
2020	\$6,286,241
2021	\$6,388,706
2022	\$6,492,842
2023	\$6,598,676
2024	\$6,706,234
2025	\$6,815,546
2026	\$6,926,639
2027	\$7,039,543
2028	\$7,154,288
2029	\$7,270,903
2030	\$7,389,418
2031	\$7,509,866
2032	\$7,632,277
2033	\$7,756,683
2034	\$7,883,117
2035	\$8,011,611
2036	\$8,142,201
Total 2017	\$140,265,019
to 2036:	, ,

^{*}This table is based on a 10 year crash average from Thorncreek Road to Moscow between 2003 and 2012, the 2012 economic cost of crashes, and a growth factor of 1.63%.

Appendix E Crash Predictions

- Total Predicted Crashes Between 2017 and 2036 and Total Predicted Fatal and Injury Crashes Between 2017 and 2036
- Summary of Crash Predictions for Alternatives E2, C3, and W4
 - E2 Total Crash Summary
 - E2 Fatal and Injury Cash Summary
 - C3 Total Crash Summary
 - C3 Fatal and Injury Cash Summary
 - W4 Total Crash Summary
 - W4 Fatal and Injury Cash Summary
 - Crash Prediction for No Action Alternative

(2017 – 2036 Crash Data is Bound Separately)

246	14.3	14.0	13.8	13.6	13.4	13.2	13.0	12.8	12.6	12.4	12.2	12.0	11.8	11.6	11.4	11.2	11.1	10.9	10.7	10.5	W4
260.	15.1	14.9	14.6	14.4	14.2	13.9	13.7	13.5	13.3	13.1	12.8	12.6	12.4	12.2	12.0	11.9	11.7	11.5	11.3	11.1	ß
213	12.3	12.1	11.9	11.8	11.6	11.4	11.2	11.1	10.9	10.7	10.6	10.4	10,3	10.1	10.0	9.8	9.7	9.5	9.4	9.2	Ð
Total	2036	2035	2034	2033	2032	2031	2030	2029	2028	2027	2026	2025	2024	2023	2022	2021	2020	2019	2018	2017	
								ry by Year	ash Summa	op Total Cr	8 US-95 Lot	and Existin	Alignment	Proposed							
:																					
219	12.8	12.6	12.4	12.2	12.0	11.8	11.6	11.4	11.2	11.0	10.8	10.6	10.5	10.3	10.1	9.9	9.8	9.6	9.4	9.3	W4
253.8	14.8	14.5	14.3	14.1	13.8	13.6	13.4	13.2	12.9	12.7	12.5	12.3	12.1	11.9	11.7	11.5	11.3	11.2	11.0	10.8	G
179.	10.5	10.3	10.1	10.0	<u>9.</u> 8	9.6	9.5	9.3	9.2	9.0	8.9	8.7	8.6	8.4	8.3	8. -1	8.0	7.9	7.7	7.6	ß
Total	2036	2035	2034	2033	2032	2031	2030	2029	2028	2027	2026	2025	2024	2023	2022	2021	2020	2019	2018	2017	
					÷				y by Year	sh Summar	t Total Cras	d Allgnmen	Propose								

Total Predicted Crashes Between 2017 and 2037

Total Predicted Fatal and Injury Crashes Between 2017 and 2037

Proposed Alignment Total Fatal and Injury Crash Summary by Year 2018 2019 2020 2021 2022 2023 2024 2025 2026 2027 2028 2030 2031 2032 2033 2034 2035 3.9 3.9 4.0 4.1 4.2 4.3 4.3 4.5 4.5 4.6 4.7 4.8 4.9 5.0 5.1 5.2 5.2 5.3 5.4 5.5 5.6 5.7 5.8 5.8 5.9 6.0 6.1 4.7 4.8 4.9 5.0 5.1 5.2 5.2 5.3 5.4 5.5 5.6 5.7 5.8 5.8 5.9 6.0 6.1 4.7 4.8 4.9 5.0 5.1 5.2 5.2 5.3 5.4 5.5 5.6 5.7 5.8 5.8 5.9 6.0 6.1 4.7 4.8 4.9 5.0 5.1 5.2 5.2 5.3 5.4	100.7	5.8	5.7	5.6	5.5	5.4	5.4	5.3	5.2	5.1	5.1	5.0	4.9	4.8	4.8	4.7	4.6	4.6	4 5	4.4	44	3
Proposed Alignment Total Fatal and Injury Crash Summary by Year 2017 2018 2019 2020 2021 2022 2023 2024 2025 2026 2027 2028 2030 2031 2032 2033 2034 2035 2036 3.8 3.9 3.9 4.0 4.1 4.2 4.3 4.3 4.5 4.5 4.6 4.7 4.8 4.9 5.0 5.1 5.2 5.2 5.3 5.4 5.5 5.6 5.7 5.8 5.8 5.9 6.0 6.1 6.2 4.6 4.7 4.8 4.9 5.0 5.1 5.2 5.2 5.3 5.4 5.5 5.6 5.7 5.8 5.8 5.9 6.0 6.1 6.2 4.6 4.7 4.8 4.9 5.0 5.1 5.2 5.2 5.3 5.4 5.5 5.6 5.7 5.8 5.8 5.9 6.0 6.1 6.2																						

116.9	6.7	6.6	6.5	6.4	6.3	6.2	6.1	6.0	6.0	5.9	5.8	5.7	5.6	5,5	5.4	5.4	5.3	5.2	5.1	<u>5</u>	W4
110.0	6.3	6.2	6.1	6.0	6.0	5.9	5.8	5.7	5.6	5.5	5.4	5,4	5.3	5.2	5.1	5.0	5.0	4.9	4.8	4.7	ឩ
100.7	5.8	5.7	5.6	5.5	5.4	5.4	5.3	5.2	5.1	5.1	5.0	4.9	4.8	4.8	4.7	4.6	4.6	4.5	4.4	4.4	62
Total	2036	2035	2034	2033	2032	2031	2030	2029	2028	2027	2026	2025	2024	2023	2022	2021	2020	2019	2018	2017	
	* *						AA I COL	Amount in A	in the second	TOUR POLOT	tons door	co-co Suno	WHIT ONLY EA	HIGHN DAS	Stau						
							bur Veas	Cummany	tinner Frach	Estal and b	I ann Tatal	tion I to DE	ane and Evi	a finance							

	11.8 1	11.6																
				11.2	11.1	10.9	10.7	10.6	10.4	10.3	10.1	10.0	9.8	9.7	9.5	9.4	9.2	Total (Crashes/year)
	1.8	1.8	1.8	1.8	1.7	1.7	1.7	1.7	1.7	1.7	1.7	1.7	1.7	1.7	1.7	1.6	1.6	Total
		0.127	0.126	0,125	0.125	0.124	0.124	0.123	0.122	0.122	0.121	0.120	0.120	0.119	0.119	0.118	0.117	North Clyde
	0.086 0	0.086	0.085	0.085	0.084	0.084	0.084	0.083	0.083	0.082	0.082	0.081	0.081	0.081	0.080	0.080	0.079	Cameron
	0.033 0	0.033	0.033	0.033	0.033	0.033	0.033	0.033	0.033	0.033	0.033	0.033	0.032	0.032	0.032	0.032	0.032	South Clyde
		0.012	0.012	0.012	0.012	0.012	0.012	0.012	0.012	0.011	0.011	0.011	0.011	0.011	0.011	0.011	0.011	Skyline
		0.024	0.024	0.024	0.024	0.024	0.023	0.023	0.023	0.023	0.023	0.023	0.023	0.023	0.023	0.023	0.023	Snow
		0.038	0.038	0.037	0.037	0.037	0.037	0.037	0.037	0.037	0.037	0.037	0.037	0.037	0.036	0.036	0.036	Zeitler
0.040		0.040	0.040	0.040	0.040	0.040	0.039	0.039	0.039	0.039	0.039	0.039	0.039	0.039	0.039	0.039	0.038	Jacksha
		0.048	0.048	0.048	0.048	0.048	0.048	0.048	0.047	0.047	0.047	0.047	0.047	0.047	0.047	0.047	0.046	Ð
	2033 2	2032	2031	2030	2029	2028	2027	2026	2025	2024	2023	2022	2021	2020	2019	2018	2017	Intersection
0.557	0.547 0	0.542	0.536	0.531	0.526	0.521	0.517	0.512	0.507	0.502	0.497	0.493	0.488	0.484	0.479	0.475	0.470	22
0.285	0.282 0	0.281	0.279	0.278	0.276	0.275	0.273	0.272	0.270	0.269	0.267	0.266	0.264	0.263	0.261	0.260	0.259	21
0.101	0.100 0	0.100	0.099	0.099	0.098	0.098	0.097	0.097	0.096	0.096	0.096	0.095	0.095	0.094	0.094	0.093	0.093	20
0.017 0.017 0.017	0.017 0	0.017	0.017	0.017	0.017	0.017	0.017	0.017	0.017	0.017	0.017	0.017	0.017	0.017	0.017	0.017	0.017	19
0.061 0.061 0.061	0.060 0	0.060	0.060	0.060	0.060	0.060	0.060	0.060	0.060	0.059	0.059	0.059	0.059	0.059	0.059	0.059	0.059	18
0.019		0.019	0.019	0.018	0.018	0.018	0.018	0.018	0.018	0.018	0.018	0.018	0.018	0.018	0.018	0.018	0.018	17
0.030		0.030	0.030	0.030	0.030	0.030	0.030	0.030	0.030	0.030	0.030	0.030	0.030	0.029	0.029	0.029	0.029	16
0.014		0.014	0.014	0.014	0.014	0.014	0.014	0.014	0.014	0.014	0.014	0.014	0.013	0.013	0.013	0.013	0.013	15
0.024		0.024	0.024	0.024	0.024	0.024	0.024	0.024	0.024	0.024	0.024	0.024	0.024	0.024	0.024	0.024	0.024	14
0.011		0.011	0.011	0.011	0.011	0.011	0.010	0.010	0.010	0.010	0.010	0.010	0.010	0.010	0.010	0.010	0.010	13
0.024		0.024	0.024	0.024	0.024	0.024	0.024	0.024	0.024	0.024	0.024	0.024	0.024	0.024	0.024	0.024	0.024	12
0.009		0.008	0.008	0.008	0.008	0.008	0.008	0.008	0.008	0.008	0.008	0.008	0.008	0.008	0.008	0.008	0.008	11
0.022		0.022	0.022	0.022	0.022	0.022	0.022	0.022	0.022	0.022	0.022	0.022	0.022	0.022	0.022	0.021	0.021	10
		0.035	0.035	0.035	0.035	0.035	0.035	0.035	0.035	0.035	0.035	0.035	0.035	0.035	0.035	0.035	0.034	9
		0.033	0.033	0.033	0.033	0.032	0.032	0.032	0.032	0.032	0.032	0.032	0.032	0.032	0.032	0.032	0.032	68
		0.026	0.026	0.026	0.026	0.026	0.026	0.026	0.026	0.026	0.026	0.026	0.026	0.026	0.026	0.026	0.026	7
		0.021	0.021	0.021	0.021	0.020	0.020	0.020	0.020	0.020	0.020	0.020	0.020	0.020	0.020	0.020	0.020	6
		0.029	0.029	0.029	0.029	0.029	0.029	0.029	0.029	0.028	0.028	0.028	0.028	0.028	0.028	0.028	0.028	C1
		0.005	0.005	0.005	0.005	0.005	0.005	0.005	0.005	0.005	0.005	0.005	0.005	0.005	0.005	0.005	0.005	4
0.017		0.017	0.017	0.017	0.017	0.017	0.017	0.017	0.017	0.017	0.017	0.017	0.017	0.017	0.017	0.017	0.017	ω
0.005		0.005	0.005	0.005	0.005	0.005	0.005	0.005	0.005	0.005	0.005	0.005	0.005	0.005	0.005	0.005	0.005	1c
0.027		0.027	0.027	0.027	0.027	0.027	0.026	0.026	0.026	0.026	0.026	0.026	0.026	0.026	0.026	0.026	0.026	1b
		0.013	0.013	0.013	0.013	0.013	0.013	0.013	0.013	0.013	0.012	0.012	0.012	0.012	0.012	0.012	0.012	1a
2034 2035	2033	2032	2031	2030	2029	2028	2027	2026	2025	2024	2023	2022	2021	2020	2019	2018	2017	Segment
																		Existing US-95
10.1 10.3	10.0	9,8	9.6	9.5	9.3	9.2	9.0	8.9	8.7	8.6	8.4	8.3	8.1	8.0	7.9	7.7	7.6	Total
0.502 0.511	0.493 (0.484	0.475	0.466	0.458	0.450	0.442	0.434	0.426	0.419	0.411	0.404	0.397	0.390	0.383	0.376	0.370	North Old US-95
		0.315	0.309	0.303	0.297	0.291	0.285	0.280	0.274	0.269	0.264	0.258	0.253	0.248	0.243	0.239	0.234	South Old US-95
2034 2035		2032	2031	2030	2029	2028	2027	2026	2025	2024	2023	2022	2021	2020	2019	2018	2017	Intersection
1.240 1.260	1.220 1	1.200	1.181	1.162	1.143	1.125	1.107	1.089	1.072	1.055	1.038	1.022	1.005	0.989	0.974	0.958	0.943	Suburban
		7.808	7.677	7.548	7.421	7.297	7.174	7.054	6.935	6.819	6.704	6.592	6.481	6.373	6.266	6.160	6.057	Rural Divided
2034 2035	2033	2032	2031	2030	2029	2028	2027	2026	2025	2024	2023	2022	2021	2020	2019	2018	2017	Segment

E-2 Total Crash Summary

	Total (Crashes/year)	Total	South Clyde	Skyline	Snow	Zeitler	Jacksha	Intersection	18	17	16	15	14	13	12	11	10	9	80	Segment	Existing US-95	Total	North Old US-95	North Clyde	Cameron	Eid	South Old US-95	Intersection	Suburban	Rural Divided	Segment	New Alignment
	11.1	0.3	0.025	0.009	0.018	0.028	0.030	2017	0.039	0.014	0.023	0.010	0.018	0.008	0.018	0.006	0.017	0.027	0.025	2017		10.8	0.179	0.241	0.158	0.217	0.339	2017	4.790	4.880	2017	
	11.3	0.3	0.025	0.009	0.018	0.028	0.030	2018	0.039	0.014	0.023	0.010	0.018	0.008	0.018	0.006	0.017	0.027	0.025	2018		11.0	0.182	0.245	0.160	0.222	0.345	2018	4.865	4.963	2018	
	11.5	0.3	0.025	0.009	0.018	0.029	0.030	2019	0.039	0.014	0.023	0.010	0.018	0.008	0.018	0.006	0.017	0.027	0.025	2019		11.2	0.185	0.249	0.163	0.226	0.352	2019	4.941	5.048	2019	
	11.7	0.3	0.025	0.009	0.018	0.029	0.030	2020	0.039	0.014	0.023	0.010	0.018	0.008	0.018	0.006	0.017	0.027	0.025	2020		11.3	0.188	0.252	0.165	0.231	0.359	2020	5.019	5.134	2020	
	11.9	0.3	0.025	0.009	0.018	0.029	0.030	2021	0.039	0.014	0.023	0.010	0.018	0.008	0.018	0.006	0.017	0.027	0.025	2021		11.5	0.191	0.256	0.168	0.235	0.367	2021	5.098	5.222	2021	
	12.0	0.3	0.025	0.009	0.018	0.029	0.031	2022	0.039	0.014	0.023	0.010	0.018	0.008	0.018	0.006	0.017	0.027	0.025	2022		11.7	0.194	0.260	0.171	0.240	0.374	2022	5.178	5.311	2022	
	12.2	0.3	0.026	0.009	0.018	0.029	0.031	2023	0.039	0.014	0.023	0.010	0.018	0.008	0.018	0.006	0.017	0.027	0.025	2023		11.9	0.197	0.265	0.173	0.245	0.382	2023	5.260	5.402	2023	
	12.4	0.3	0.026	0.009	0.018	0.029	0.031	2024	0.039	0.014	0.023	0.010	0.019	0.008	0.018	0.006	0.017	0.027	0.025	2024		12.1	0.200	0.269	0.176	0.250	0.389	2024	5.343	5.494	2024	
	12.6	0.3	0.026	0.009	0.018	0.029	0.031	2025	0.040	0.014	0.023	0.010	0.019	0.008	0.018	0.006	0.017	0.027	0.025	2025		12.3	0.203	0.273	0.179	0.255	0.397	2025	5.427	5.588	2025	
_	12.8	0.3	0.026	0.009	0.018	0.029	0.031	2026	0.040	0.014	0.023	0.011	0.019	0.008	0.019	0.006	0.017	0.027	0.025	2026		12.5	0.206	0.277	0.181	0.260	0.405	2026	5.513	5.683	2026	
	13.1	0.3	0.026	0.009	0.018	0.029	0.031	2027	0.040	0.014	0.023	0.011	0.019	0.008	0.019	0.006	0.017	0.027	0.025	2027		12.7	0.210	0.282	0.184	0.265	0.413	2027	5.601	5.780	2027	
7	13.3	0.3	0.026	0.009	0.018	0.029	0.031	2028	0.040	0.014	0.023	0.011	0.019	0.008	0.019	0.006	0.017	0.027	0.025	2028		12.9		0.286	0.187	0.271	0.421	2028	5.690	5.879	2028	
Total crashes between 2017 and 2036	13.5	0.3	0.026	0.009	0.018	0.029	0.031	2029	0.040	0.014	0.023	0.011	0.019	0.008	0.019	0.007	0.017	0.027	0.025	2029		13.2	0.216	0.291	0.190	0.276	0.430	2029	5.780	5.979	2029	
shes b	13.7	0.3	0.026	0.009	0.018	0.029	0.031	2030	0.040	0.014	0.023	0.011	0.019	0.008	0.019	0.007	0.017	0.027	0.025	2030		13.4	0.220	0.295	0.193	0.282	0.439	2030	5.872	6.081	2030	
etween	13.9	0.3	0.026	0.009	0.019	0.029	0.031	2031	0.040	0.014	0.023	0.011	0.019	0.008	0.019	0.007	0.017	0.027	0.025	2031		13.6	0.224	0.300	0.196	0.287	0.447	2031	5.965	6.185	2031	
2017 a	14.2	0.3	0.026	0.009	0.019	0.029	0.031	2032	0.040	0.014	0.023	0.011	0.019	0.008	0.019	0.007	0.017	0.027	0.025	2032		13.8	0.227	0.305	0.199	0.293	0.456	2032	6.061	6.291	2032	
nd 203	14.4	0.3	0.026	0.009	0.019	0.030	0.031	2033	0.040	0.014	0.023	0.011	0.019	0.008	0.019	0.007	0.017	0.027	0.025	2033		14.1 14.1	0.231	0.310	0.202	0.299	0.465	2033	6.157	6.398	2033	
6	14.6	0.3	0.026	0.010	0.019	0.030	0.031	2034	0.040	0.014	0.023	0.011	0.019	0.008	0.019	0.007	0.017	0.027	0.025	2034		14.3	0.235	0.315	0.205	0.305	0.475	2034	6.256	6.507	2034	
	14.9	0.3	0.026	0.010	0.019	0.030	0.031	2035	0.040	0.014	0.023	0.011	0.019	0.008	0.019	0.007	0.017	0.028	0.025	2035		14.5	0.238	0.320	0.209	0.311	0.484	2035	6.356	6.619	2035	
260.2	15.1	0.3	0.026	0.010	0.019	0.030	0.032	2036	0.040	0.014	0.023	0.011	0.019	0.008	0.019	0.007	0.017	0.028	0.025	2036		14.8	0.242	0.325	0.212	0.317	0.494	2036	6.458	6.732	2036	

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	liotal (Crasnes/year)		Total			South Chul	Shuling	Zeitler	Intersection	22	21	20	19	18	17	16	15	14	13	12	11	Segment	Existing US-95	Total	North Old US-95	South Old US-95	Jacksha	Eid	Intersection	Suburban	Rural Divided	Segment	New Alignment
	/year)		1	•	đ				Š																-95	-95			ă		ä		
	10.5	5	4 -	0.079				0.026	2017	0.470	0.259	0.093	0.015	0.051	0.016	0.026	0.012	0.021	0.009	0.021	0.007	2017		9.3	0.370	0.213	0.318	0.339	2017	1.123	6.910	2017	
	10.7	ī	4 TO	0.000				0.026	2018	0.475	0.260	0.093	0.015	0.051	0.016	0.026	0.012	0.021	0.009	0.021	0.007	2018		9,4	0.376	0.217	0.325	0.345	2018	1.141	7.028	2018	
	10.9	Ī	4.3		0.023			0.026	2019	0.479	0.261	0.094	0.015	0.051	0.016	0.026	0.012	0.021	0.009	0.021	0.007	2019		9.6	0.383	0.221	0.331	0.352	2019	1.159	7.148	2019	
	11.1		4 1 1	0.001	0.024	0.000	0.011	0.027	2020	0.484	0.263	0.094	0.015	0.051	0.016	0.026	0.012	0.021	0.009	0.021	0.007	2020		9.8	0.390	0.226	0.338	0.359	2020	1.178	7.270	2020	
	11.2	Ī	4.5	0.00	0.024	0.000	0.017	0.027	2021	0.488	0.264	0.095	0.015	0.052	0.016	0.026	0.012	0.021	0.009	0.021	0.007	2021		9.9	0.397	0.230	0.345	0.367	2021	1.196	7.394	2021	
	11.4							0.027					0.015				0.012		0.009	0.021	0.007	2022		10.1	0,404	0.235	0.352	0.374	2022			2022	
	4 11.6											5 0.096						1 0.021	9 0.009	1 0.021	7 0.007	2 2023		10.3	4 0.411	5 0.240	2 0.359	4 0.382	2 2023			2 2023	
			ł				-																		ľ	10 0.244	59 0.366						
	11.8							0.027 0		0.502 0			0.015 0				0.012 0	0.021 0	0.009 0	0.021 0	0.007 0	2024 2		10.5	0.419 0	-		0.389 0	2024 2			2024 2	
	12.0	1	4.3														0.012	0.021	0.009	0.021	0.007	2025		10.6	0.426	0.249	0.373	0.397	2025		7.912	2025	
	12.2	1	1 20		0.024			0.027 D D17	2026	0.512	0.272	0.097	0.015	0.052	0.016	0.026	0.012	0.021	0.009	0.021	0.007	2026		10.8	0.434	0.254	0.381	0.405	2026	1.296	8.047	2026	
	12.4	ī	, <u>C</u> . I∠4	0.004	0.024			0.027	2027	0.517	0.273	0.097	0.015	0.052	0.016	0.026	0.012	0.021	0.009	0.021	0.007	2027		11.0	0.442	0.259	0.388	0.413	2027	1.317	8,184	2027	
	12.6	ī	0.124	0.004	0.024	0.003	0.017	0.027	2028	0.521	0.275	0.098	0.015	0.052	0.016	0.026	0.012	0.021	0.009	0.021	0.007	2028		11.2	0.450	0.265	0.396	0.421	2028	1.338	8.324	2028	
Tota	12.8	-	0,120	0.004	0.024		0.017	0.027	2029	0.526	0.276	0.098	0.015	0.052	0.016	0.026	0.012	0.021	0.009	0.021	0.007	2029		11.4	0.458	0.270	0.404	0.430	2029	1.360	8.466	2029	
Total crashes between 2017 and 2036	13.0	5	C21.0	0.000	0.024		0.017	0.027	2030	0.531	0.278	0.099	0.015	0.052	0.016	0.026	0.012	0.021	0.009	0.021	0.007	2030		11.6	0.466	0.275	0.412	0.439	2030	1.382	8.611	2030	
betwee	13.2	-	0.120		0.024		0.017	0.027	2031	0.536	0.279	0.099	0.015	0.053	0,016	0.026	0.012	0.021	0.009	0.021	0.007	2031		11.8	0.475	0.281	0.420	0.447	2031	1.404	8.758	2031	
1 2017 ar	13.4	-	0.127		0.024			0.027	2032	0.542	0.281	0,100	0.015	0.053	0.016	0.026	0.012	0.021	0.009	0.021	0.007	2032		12.0	0.484	0.287	0.429	0.456	2032	1.427	8.907	2032	
1d 2036	13.6							0.027											0.009		0.007	2033		12.2	0.493	0.29	0.438	0.46	2033	1.45	9.055	2033	
	5 13.8							7 0.027													_	3 2034		12,4			8 0.446		3 2034			3 2034	
	8 14.0							27 0.028																4 12.6					4 2035			4 2035	
24																														-			
246.2	14.3	- +	129 1129	100/	024	0.008		028	2036	563	287	102	0.015	053	016	026	012	021	600	0.021	0.007	2036		12.8	0.521	0.310	0.464	0.494	2036	1.522	9.531	2036	

Total (Crashes/year)	Total	North Clyde	Cameron	South Clyde	Skyline	Snow	Zeitler	Jacksha		Intersection	22	21	20	19	18	17	16	15	14	13	12	11	10	. 9	00	7	л Л	- C1	4	ω	1c	1b	1a	Segment	Existing US-95	Total	North Old US-95	South Old US-95	Intersection	Suburban	Rural Divided	Segment	New Alignment
4.4	0.6	0.049	0.033	0.013	0.005	0.010	0.015	0.016	0.019	2017	0.151	0.083	0.030	0.005	0.019	0.006	0.009	0.004	0.008	0.003	0.008	0.003	0.007	0.011	0.010	0.008	0.006	0.009	0.001	0.005	0.001	0.008	0.004	2017		ა. 8	0.142	0.078	2017	0.286	3.316	2017	
4.4	0.6	0.049	0.033	0.013	0.005	0.010	0.015	0.016	0.019	2018	0.152	0.083	0.030	0.005	0.019	0.006	0.009	0.004	0.008	0.003	0.008	0.003	0.007	0.011	0.010	0.008	0.006	0.009	0.001	0.005	0.001	0.008	0.004	2018		3.9	0.145	0.079	2018	0.290	3.368	2018	
4.5	0.6	0.049	0.033	0.013	0.005	0.010	0.015	0.016	0.019	2019	0.154	0.084	0.030	0.005	0.019	0.006	0.009	0.004	0.008	0.003	0.008	0.003	0.007	0.011	0.010	0.008	0.006	0.009	0.001	0.005	0.001	0.008	0.004	2019		3.9	0.147	0.080	2019	0.294	3,420	2019	
4.6	0.6	0.049	0.033	0.013	0.005	0.010	0.015	0.016	0.019	2020	0.155	0.084	0.030	0.005	0.019	0.006	0.009	0.004	0.008	0.003	0.008	0.003	0.007	0.011	0.010	0.008	0.006	0.009	0.001	0.005	0.001	0.008	0.004	2020		4.0	0.149	0.082	2020	0.299	3.473	2020	
4,6	0.6	0.050	0.034	0.013	0.005	0.010	0.015	0.016	0.019	2021	0.157	0.085	0.030	0.005	0.019	0.006	0.009	0.004	0.008	0.003	0.008	0.003	0.007	0.011	0.010	0.008	0.006	0.009	0.001	0.005	0.001	0.008	0.004	2021		4:1 1	0.152	0.083	2021	0.303	3.528	2021	
4.7	0.6	0.050	0.034	0.014	0.005	0.010	0.015	0.016	0.020	2022	0.158	0.085	0.031	0.005	0.019	0.006	0.009	0.004	0.008	0.003	0.008	0.003	0.007	0.011	0.010	0.008	0.007	0.009	0.002	0.005	0.002	0.008	0.004	2022		4.1	0.154	0.085	2022	0.308	3.583	2022	
4.8	0.6	0.050	0.034	0.014	0.005	0.010	0.015	0.016	0.020	2023	0,160	0.086	0.031	0.005	0.019	0.006	0.010	0.004	0.008	0.003	0.008	0.003	0.007	0.011	0.010	0.008	0.007	0.009	0.002	0.005	0.002	0.008	0.004	2023		4.2	0.157	0.087	2023	0.313	3.638	2023	
4.8	0.6	0.050	0.034	0.014	0.005	0.010	0.015	0.016	0.020	2024	0.161	0.086	0.031	0.005	0.019	0.006	0.010	0.004	0.008	0.003	0.008	0.003	0.007	0.011	0.010	0.008	0.007	0.009	0.002	0.005	0.002	0.008	0.004	2024		4.3	0.160	0.088	2024	0.317	3.695	2024	
4.9	0.6	0.051	0.034	0.014	0.005	0.010	0.015	0.016	0.020	2025	0.163	0.087	0.031	0.005	0.019	0.006	0.010	0.004	0.008	0.003	0.008	0.003	0.007	0.011	0.010	0.008	0.007	0.009	0.002	0.005	0.002	0.008	0.004	2025		4,3	0.162	0.090	2025	0.322	3.753	2025	
5.0	0.6	0.051	0.034	0.014	0.005	0.010	0.015	0.016	0.020	2026	0.164	0.087	0.031	0.005	0.019	0.006	0.010	0.004	0.008	0.003	0.008	0.003	0.007	0.011	0.010	0.008	0.007	0.009	0.002	0.005	0.002	0.008	0.004	2026		4.4	0.165	0.091	2026	0.327	3.811	2026	
5.1	0.6	0.051	0.035	0.014	0.005	0.010	0.015	0.016	0.020	2027	0.166	0.088	0.031	0.005	0.019	0.006	0.010	0.004	0.008	0.003	0.008	0.003	0.007	0.011	0.010	0.008	0.007	0.009	0.002	0.006	0.002	0.008	0.004	2027		4.5	0.168	0.093	2027	0.332	3.870	2027	2
5.1	0.6	0.052	0.035	0.014	0.005	0.010	0.015	0.016	0.020	2028	0.167	0.088	0.031	0.005	0.019	0.006	0.010	0.004	0.008	0.003	0.008	0.003	0.007	0.011	0.010	0.008	0.007	0.009	0.002	0.006	0.002	0.009	0.004	2028		4.5	0.171	0.095	2028	0.337	3.931	2028	
5.2	0.6	0.052	0.035	0.014	0.005	0.010	0.015	0.016	0.020	2029	0.169	0.089	0.032	0.005	0.019	0.006	0.010	0.004	0.008	0.003	0.008	0.003	0.007	0.011	0.010	0.008	0.007	0.009	0.002	0.006	0.002	0.009	0.004	2029		4.6	0.174	0.097	2029	0.343	3.992	2029	
5.3	0.6	0.052	0.035	0.014	0.005	0.010	0.016	0.016	0.020	2030	0.171	0.089	0.032	0.005	0.019	0.006	0.010	0.004	0.008	0.003	0.008	0.003	0.007	0.011	0.010	0.008	0.007	0.009	0.002	0.006	0.002	0.009	0.004	2030		4.7	0.177	0.098	2030	0.348	4.054	2030	
5.4	0.6	0.052	0.035	0.014	0.005	0.010	0.016	0.017	0.020	2031	0.172	0.090	0.032	0.006	0.019	0.006	0.010	0.004	0.008	0.003	0.008	0.003	0.007	0.011	0.010	0.008	0.007	0.009	0.002	0.006	0.002	0.009	0.004	2031		4.8	0.180	0.100	2031	0.353	4.117	2031	
5.4	0.6	0.053	0.036	0.014	0.005	0.010	0.016	0.017	0.020	2032	0.174	0.090	0.032	0.006	0.019	0.006	0.010	0.004	0.008	0.003	0.008	0.003	0.007	0.011	0.011	0.008	0.007	0.009	0.002	0.006	0.002	600 0	0.004	20.32		4.8	0.183	0.102	2032	0.359	4.181	2032	
5.5	0.6	0.053	0.036	0.014	0.005	0.010	0.016	0.017	0.020	2033	0.176	0.091	0.032	0.006	0.019	0.006	0.010	0.004	0.008	0.003	0.008	0.003	0.007	0.011	0.011	0.008	0.007	0.009	0.002	0.006	0.002	0.009	0.004	2033		4.9	0.186	0.104	2033	0.364	4.247	2033	
5.6	0.6	0.053	0.036	0.014	0.005	0.010	0.016	0.017	0.020	2034	0.177	0.091	0.032	0.006	0.019	0.006	0.010	0.004	0.008	0.003	0.008	0.003	0.007	0.011	0.011	0.008	0.007	0,009	0.002	0.006	0.002	600.0	0.004	2034								2034	
5.7	0.6	0.053	0.036	0.014	0.005	0.010	0.016	0.017	0.020	2035	0.179	0.092	0.033	0.006	0.019								0.007	0.011										20.35		5.1						2035	
5.8	0.6	0.054	0.036	0.014	0.005	0.010	0.016	0.017	0.020	2036	0.181	0.092	0.033	0.006	0.020	0.006	0.010	0.004	0.008	0.003	0,008	0.003	0.007	0.011	0.011	0.009	0.007			0.006	0.002			3500		5.1						2036	

Total fatal and injury crashes between 2017 and 2036

100.7

E-2 Fatal and Injury Crash Summary

ი ა	6.2	6.1	6.0	6.0	9.9 2.9	0.0	0.7	0.0 0	0.0	0.4	ט 4	0.0 .0	Ú.N	9	ļ	0.0	1			
	!						l I				2	n 3	2	n	כ	л Э	49	48	47	Total (Crashes/vear)
0.1	0.1	0.1	0.1	0.1	0.1	0.1	0.1	0.1	0.1	0.1	0.1	0.1	0.1	0.1	0.1	0.1	0.1	0.1	0.1	Total
0.011	0.011	0.011	0.011	0.011	0.011	0.011	0.011	0.011	0.011	0.011	0.011	0.011	0.011	0.011	0.011	0.011	0.010	0.010	0.010	Clyde South
0.004	0.004	0.004	0.004	0.004	0.004	0.004	0.004	0.004	0.004	0.004	0.004	0.004	0.004	0.004	0.004	0.004	0.004	0.004	0.004	Skyline
0.008	0.008	0.008	0.008	0.008	0.008	0.008	0.008	0.008	0.008	0.008	0.008	0.008	0.008	0.008	0.008	0.007	0.007	0.007	0.007	Snow
0.012	0.012	0.012	0.012	0.012	0.012	0.012	0.012	0.012	0.012	0.012	0.012	0.012	0.012	0.012	0.012	0.012		0.012	0.012	Zeitler
0.013	0.013	0.013	0.013	0.013	0.013	0.013	0.013	0.013	0.013	0.013	0.013	0.013	0.013	0.013	0.013	0.013	0.013	0.013	0.013	Jacksha
2036	2035	2034	2033	2032	2031	2030	2029	2028	2027	2026	2025	2024	2023	2022	2021	2020	2019	2018	2017	Intersection
0.013	0.013	0.013	0.013	0.013	0.013	0.013	0.013	0.013	0.013	0.013	0.013	0.013	0.013	0.013	0.013	0.013	0.013	0.013	0.013	18
0.005	0.005	0.005	0.005	0.005	0.005	0.005	0.005	0.005	0.005	0.005	0.005	0.005	0.005	0.005	0.004	0.004	0.004	0.004	0.004	17
0.008	0.008	0.007	0.007	0.007	0.007	0.007	0.007	0.007	0.007	0.007	0.007	0.007	0.007	0.007	0.007	0.007	0.007	0.007	0.007	16
0.003	0.003	0.003	0.003	0.003	0.003	0.003	0.003	0.003	0.003	0.003	0.003	0.003	0.003	0.003	0.003	0.003	0.003	0.003	0.003	15
0.006	0.006	0.006	0.006	0.006	0.006	0.006	0.006	0.006	0.006	0.006	0.006	0.006	0.006	0.006	0.006	0.006	0.006	0.006	0.006	14
0.003	0.003	0.003	0.003	0.003	0.003	0.003	0.003	0.003	0.003	0.003	0.003	0.003	0.003	0.003	0.003	0.003	0.003	0.003	0.003	13
0.006	0.006	0.006	0.006	0.006	0.006	0.006	0.006	0.006	0.006	0.006	0.006	0.006	0.006	0.006	0.006	0.006	0.006	0.006	0.006	12
0.002	0.002	0.002	0.002	0.002	0.002	0.002	0.002	0.002	0.002	0.002	0.002	0.002	0.002	0.002	0.002	0.002	0.002	0.002	0.002	11
0.006	0.005	0.005	0.005	0.005	0.005	0.005	0.005	0.005	0.005	0.005	0.005	0.005	0.005	0.005	0.005	0.005	0.005	0.005	0.005	10
0.009	600'0	0.009	0.009	0.009	0.009	0.009	0.009	0.009	0.009	0.009	0.009	0.009	0.009	0.009	0.009	0.009	0.009	0.009	0.009	9
0.008	0.008	0.008	0.008	0.008	0.008	0.008	0.008	0.008	0.008	0.008	0.008	0.008	0.008	0.008	0.008	0.008	0.008	0.008	0.008	60
2036	2035	2034	2033	2032	2031	2030	2029	2028	2027	2026	2025	2024	2023	2022	2021	2020	2019	2018	2017	Segment
																				Existing US-95
5	9					:				,										
	2 1	8.0	פּא	л 20	20	5.7	5.6	טי טי	5.4	5 .3	5.2	5.2	5.1	5.0	4.9	4.9	4.8	4.7	4.6	Total
	0 1 7 7	0 1 2 5	0 1 2 3	0.121	0.119	0.117	0.115	0.114	0.112	0.110	0.108	0.106	0.105	0.103	0.101	0.100	0.098	0.097	0.095	North Old US-95
	0.160	0.158	0.155	0.153	0.150	0.148	0.146	0.143	0.141	0.139	0.137	0.135	0.132	0.130	0.128	0.126	0.124	0.122	0.121	Clyde North
	0.097	0.095	0.094	0.092	0.091	680'0	0.088	0.087	0.085	0.084	0.083	0.082	0.080	0.079	0.078	0.077	0.075	0.074	0.073	Cameron
	0.099	0.097	0.096	0.094	0.092	0.091	0.089	0.087	0.086	0.084	0.083	0.081	0.080	0.078	0.077	0.075	0.074	0.073	0.071	Eid
	0.206	0.202	0.198	0.194	0.191	0.187	0.184	0.181	0.177	0.174	0.171	0.168	0.165	0.162	0.159	0.156	0.153	0.150	0.148	South Old US-95
ļ	2035	2034	2033	2032	2031	2030	2029	2028	2027	2026	2025	2024	2023	2022	2021	2020	2019	2018	2017	Intersection
	1.900	1.872	1.844	1.817	1.790	1.763	1.738	1.712	1.687	1.662	1.638	1.614	1.591	1.568	1.545	1.523	1.501	1.479	1.458	Suburban
	3.529	3.475	3.422	3.369	3.317	3.266	3.216	3.167	3.118	3.071	3.023	2.977	2.931	2.886	2.842	2.799	2.756	2.713	2.672	Rural Divided
2036	2035	2034	2033	2032	2031	2030	2029	2028	2027	2026	2025	2024	2023	2022	2021	2020	2019	2018	2017	Segment

C-3 Total Fatal and Injury Crash Summary

Total fatal and injury crashes between 2017 and 2036 110.0

	Total (Crashes/year) 5.1	lotal 0.4	de		e				ion		21 0.083								13 0.003			Segment 2017	Existing US-95	10tal 4.5	05-95				Intersection 2017	Suburban 0.341	Rural Divided 3.783	Segment 2017
	5.1	0.4	0.049	0.033	0.010	0.003	0.007	0.011	2018	0.152	0.083	0.030	0.005	0.016	0.005	0.008	0.004	0.007	0.003	0.007	0.002	2018		4./	0.145	0.071	0.140	0.150	2018	0.346	3.842	2018
	5.2	0.4	0.049	0.033	0.010	0.003	0.007	0,011	2019	0.154	0.084	0.030	0.005	0.016	0.005	0.008	0.004	0.007	0.003	0.007	0.002	2019		4.8	0.147	0.072	0.143	0.153	2019	0.351	3.902	2019
	5.3	0.4	0.049	0.033	0.010	0.003	0.007	0.011	2020	0.155	0.084	0.030	0.005	0.017	0.005	0.008	0.004	0.007	0.003	0.007	0.002	2020		4.8	0.149	0.073	0.145	0.156	2020	0.356	3.963	2020
	5.4	0.4	0.050	0.034	0.010	0.003	0.007	0.011	2021	0.157	0.085	0.030	0.005	0.017	0.005	0.008	0.004	0.007	0.003	0.007	0.002	2021		4.9	0.152	0.075	0.148	0.159	2021	0.362	4.024	2021
	5.4	0.4	0.050	0.034	0.010	0.003	0.007	0.011	2022	0.158	0.085	0.031	0.005	0.017	0.005	0.008	0.004	0.007	0.003	0.007	0.002	2022		5.0	0.154	0.076		0.162	2022			2022
	5.5	0.4	0.050	0.034	0.010	0.003	0.007	0.011	2023	0.160	0.086	0.031	0.005	0.017	0.005	0.008	0.004	0.007	0.003	0.007	0.002	2023		5.1	0.157	0.078		0.165	2023	0.373	4.151	2023
	5.6	0.5	0.050	0.034	0.010	0.003	0.007	0.011	2024	0.161	0.086	0.031	0.005	0.017	0.005	0.008	0.004	0.007	0.003	0.007	0.002	2024		5.2	0.160	0.079		0.168	2024			2024
	5.7	0.5	0.051	0.034	0.010	0.003	0.007	0.011	2025	0.163	0.087	0.031	0.005	0.017	0.005	0.008	0.004	0.007	0.003	0.007	0.002	2025		5.2	0.162	0.080	0.159	0.171				2025
	5.8	0.5	0.051	0.034	0.010	0.004	0.007	0.011	2026	0.164	0.087	0.031	0.005	0.017	0.005	0.008	0.004	0.007	0.003	0.007	0.002	2026		5.3	0.165	0.082	0.162	0.174	2026			2026
	5.9	0.5	0.051	0.035	0.010	0.004	0.007	0.011	2027	0.166	0.088	0.031	0.005	0.017	0.005	0.008	0.004	0.007	0.003	0.007	0.002	2027		5.4	0.168	0.083	0.165	0.177			1	2027
Total	6.0	0.5	0.052	0.035	0.010	0.004	0.007	0.011	2028	0.167	0.088	0.031	0.005	0.017	0.005	0.008	0.004	0.007	0.003	0.007	0.00Z	2028		5.5	0.171	0.085	0.168	0.181	2028	0.402	4.484	2028
fatal and	6.0	0.5	0.052	0.035	0.010	0.004	0.007	0.011	2029	0.169	0.089	0.032	0.005	0.017	0.005	0.008	0.004	0.007	0.003	0.007	0.002	2029		5.6	0.174	0.087	0.171	0.184	2029			2029
injury c	6.1	0.5	0.052	0.035	0.010	0.004	0.007	0.011	2030	0.171	0.089	0.032	0.005	0.017	0.005	0.008	0.004	0.007	0.003	0.007	0.002	2030		5.7	0.177	0.088			2030			2030
rashes	6.2	0.5	0.052	0.035	0.010	0.004	0.007	0.011	2031	0.172	0.090	0.032	0.005	0.017	0.005	0.008	0.004	0.007	0.003	0.007	0.002	2031		5.8	0.180	0.090			2031			2031
fatal and injury crashes between 2017 and 2036	6.3	0.5	0.053	0.036	0.010	0.004	0.007	0.011	2032	0.174	0.090	0.032	0.005	0.017	0.005	0.008	0.004	0.007	0.003	0.007	0.002	2032		5.8	0.183	0.091	0.181	0.194	2032	0.427	4.770	2032
12017 ar	6.4	0.5	0.053	0.036	0.010	0.004	0.007	0.011	2033	0.176	0.091	0.032	0.005	0.017	0.005	0.008	0.004	0.007	0.003	0.007	0.002	2033		5.9	0.186	0.093	0.184	0.198	2033	0.433	4.845	2033
1d 2036	6.5	0.5	0.053	0.036	0.010	0.004	0.007	0.011	2034	0.177	0.091	0.032	0.005	0.017	0.005	0.008	0.004	0.007	0.003	0.007	0.002	2034		6.0	0.189	0.095	0.188	0.202	2034	0.440	4.920	2034
	6.6	0.5	0.053	0.036	0.010	0.004	0.007	0.011	2035	0.179	0.092	0.033	0.005	0.017	0.005	0.008	0.004	0.007	0.003	0.007	0.00Z	2035		6.1	0.193	0.097	0.191	0.206	2035	0.447	4.997	2035
116.9	6.7	0.5	0.054	0.036	0.010	0.004	0.007	0.011	2036	0.181	0.092	0.033	0.005	0.017	0.005	0.009	0.004	0.007	0.003	0.007	0.002	2036		6.2	0.196	0.098	0.195	0.209	2036	0.454	5.075	2036

W-4 Total Fatal and Injury Crash Summary

Crash Prediction on the No Action Alternative

Crash Pre	diction on N	lo Action Al	ternative
Year	Crashes	F and I	PDO
2017	27.4	11.0	16.5
2018	27.9	11.1	16.7
2019	28.3	11.3	17.0
2020	28.8	11.5	17.3
2021	29.3	11.7	17.6
2022	29.7	11.9	17.9
2023	30.2	12.1	18.2
2024	30.7	12.3	18.5
2025	31.2	12.5	18.8
2026	31.7	12.7	19.1
2027	32.2	12.9	19.4
2028	32.8	13.1	19.7
2029	33.3	13.3	20.0
2030	33.8	13.5	20.3
2031	34.4	13.7	20.7
2032	35.0	14.0	21.0
2033	35.5	14.2	21.3
2034	36.1	14.4	21.7
2035	36.7	14.6	22.0
2036	37.3	14.9	22.4
Total:	642.5	256.5	386.0

*This table is based on a 10 year crash average from Thorncreek Road to Moscow between 2003 and 2012 and a growth factor of 1.63%.